Hydrogeological Assessment Report 1386 & 1394 Greely Lane, Ottawa, Ontario

January 14, 2026

Prepared for:
Cassidy EW Construction Consultant Ltd.

Cambium Reference: 17281-002

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1.0 Introduction

Cambium Inc. (Cambium) was retained by Cassidy EW Construction Consultant Ltd. (the Client) to complete a hydrogeological assessment and terrain analysis for the proposed redevelopment of the land located at 1386 and 1394 Greely Lane, Ottawa, Ontario (the Site).

The purpose of the field work and testing was to obtain information on the general subsurface and groundwater conditions at the Site by means of groundwater monitoring well measurements, as well as field and laboratory tests. This report addresses the hydrogeological aspects of the subsurface conditions at the Site. Cambium has also completed a Geotechnical Investigation (Cambium, 2023a) and a Phase Two Environmental Site Assessment (Cambium, 2023b) prior to the hydrogeological assessment and relevant details of these investigations have been incorporated into this report. Detailed information from the Geotechnical Investigation and the Phase Two Environmental Site Assessment were provided under separate cover.

This report provides the results of the hydrogeological assessment and should be read in conjunction with the “Standard Limitations” in Section 12.0, which forms an integral part of this document. The reader’s attention is specifically drawn to this information, as it is essential for the proper use and interpretation of this report. The data, interpretations, and recommendations contained in this report pertain to a specific project as described in the report and are not applicable to any other project or site location. If the project is modified in concept, location, or elevation, or if the project is not initiated within eighteen months of the date of the report, Cambium should be given an opportunity to confirm that the recommendations in this report are still valid.

1.1 Site Description

The Site is an irregularly shaped 0.47 ha (1.15 acres) property that is developed for commercial use. It contains a single-storey commercial car wash building, two temporary sea-can storage units, and an additional single storey metal storage building adjacent to the commercial building. A driveway connects to the adjacent Greely Lane at two locations on the north side of the site. The remainder of the property is landscaped, with the southern portion of

the Site predominantly occupied by a septic bed raised at a higher elevation than the grade. The Site is bound by Greely Lane to the east, Parkway Road to the south, and commercial/light industrial use to the north and west.

Based on discussions with the Client and preliminary site sketches provided to Cambium, it understood that the proposed plan is to construct one 1,110 m^2 (12,000 ft^2) building for light industrial use which will be divided in three 370 m^2 (4,000 ft^2) units with two loading bays, two washrooms, and an estimated five employees for each unit. The building will be constructed slab-on-grade with perimeter foundations that will extend to below the local frost penetration depths. The development will include at grade parking and driveways to access delivery doors at the backs of each building.

The proposed finished floor elevations (FFE) have not yet been determined; however, it is anticipated that the grades of the Site will not differ significantly from the current grades of the property, exclusive of the raised septic bed on the southern property. The grade there will be lowered as a result of removal of the septic bed.

The regional location of the Site is identified on Figure 1, the property and surrounding areas are outlined on Figure 2, and a Site plan is included in Appendix A.

1.2 City of Ottawa Formal Review Comments

The City of Ottawa provided review comments sent via email on August 20, 2025, and on December 17, 2025 pertaining to this hydrogeological assessment. A response letter entitled *Response to City of Ottawa Review Questions Re. Hydrogeological Assessment Report – 1386 & 1394 Greely Lane, Ottawa, Ontario* was prepared by Cambium on November 25, 2025 outlining each City of Ottawa comment and Cambiums' responses. It has since been confirmed that the City of Ottawa requires the response comments addressed in an updated Hydrogeological Assessment Report rather than a response letter. As such, this report has been updated to address the City of Ottawa review comments from December 17, 2025. Cambium's previous response letter and City of Ottawa review comments are included in Appendix A.

2.0 Physical Setting

2.1 Topography and Drainage

Based on regional topographic maps the Site area is relatively flat with a gentle slope to the east-southeast towards the North Castor River. The Site has a raised septic bed located in the southern portion of the property with a topographic high of approximately 100 meters above sea level (masl).

The Site is located within the Castor River quaternary watershed, and the North Castor River is located approximately 250 m south-southeast of the Site. North Castor River subsequently flows eastward into South Nation River, which is a tributary to Ottawa River.

Regionally, surface elevation decreases to the east toward Ottawa River. It is assumed that local drainage will follow the local surficial topography and flow towards the south-southeast ultimately discharging into the North Castor River. Based on the location of the nearest water bodies and topographic relief, the inferred that the regional groundwater flow direction is easterly.

2.2 Physiography

The Site is located in the physiographic region known as the Russell and Prescott Sand Plains (Chapman & Putnam, 1984). The Russell and Prescott Sand Plains region covers and area of approximately 1,490 km^2 extending from Ottawa to Hawkesbury. The Sand Plains are a relatively flat region with a clay valley located to the south, which was formed as a delta by the Ottawa River and tributaries of the Champlain Sea. The sand deposits have a thickness of 5 m to 10 m in the northern region of the plains and thin towards the clay plains of the south. The sand plains consist of coarser grained sands to the north grading into fine sand to silt in the south. The region is underlain by stratified red and grey clays (Appendix A).

2.3 Overburden Geology

According to Miscellaneous Release – Data 128 from the Ontario Geological Survey (2010) the predominant overburden of the Site consists of coarse-textured glaciomarine deposits (sand, gravel, minor silt and clay) (Appendix A).

2.4 Bedrock Geology

According to Miscellaneous Release – Data 219 from the Ontario Geological Survey (2007), the bedrock in the area of the Site consists of the Beekmantown Group. The Beekmantown Group consists of two formations: the March and Oxford Formations. The bedrock of the Site consists of the Oxford Formation and is described as dolostone, minor shale and sandstone (Appendix A).

2.5 Vulnerable and Regulated Areas

The Site is situated within the South Nation Source Protection Area, under jurisdiction of the South Nation Conservation Authority, as per the Source Water Protection Information Atlas (SPIA) from the Ministry of the Environment, Conservation and Parks (MECP) (2024a). The Site is within the following areas:

- Intake Protection Zone 3 (IPZ-3) with a vulnerability score of 7
- Significant Ground Water Recharge Area (SGRA) with a vulnerability score of N/A
- Highly Vulnerable Aquifer (HVA) with a vulnerability score of 6

IPZs are areas surrounding water courses and lakes which have surface water intakes for water supply. There is potential that contaminants spilled within IPZs may reach intakes more quickly than the ability to take appropriate action to shut down the intake should a spill occur. IPZ-3s are defined as event-based areas only. They are areas that can contribute contaminants under an extreme event (e.g., high winds or heavy rain) at a concentration that would result in deterioration of untreated source water. Best management practices should be used to minimize the potential for the release of chemicals to the environment during future operations at the Site.

SGRAs are landscape surfaces which allow a high volume of water to infiltrate into the ground. A recharge area is classified as significant if the recharge rate for a particular area is greater than the average watershed recharge rate by 15% or more and the area has a hydrological connection to a surface water body or to an aquifer that is a source of groundwater for a drinking water system (Ministry of the Environment, Conservation and Parks, 2021). SGRAs

are delineated using models which consider topography, surficial soil, land cover and climate. The SGRA in the vicinity of the Site does not have a vulnerability score associated with it. Efforts should be made to maintain the Site pre-development water balance as much as practicable following redevelopment. Water balance information is presented in Section 7.0.

HVAs are aquifers that are more sensitive to contamination as a result of the proximity to surface (shallow aquifers). By default, all HVA's have a vulnerability score of 6. Best management practices should be used to minimize the potential for the release of chemicals to the subsurface environment during future operations at the Site.

A review of the Natural Heritage System database from the Ministry of Natural Resources and Forestry (2024) indicates the Site is not located within any Areas of Natural and Scientific Interest.

The Site does not fall under a regulated area, as per the South Nation Conservation Authority or O.Reg. 41/24.

The source protection, natural heritage, and conservation area mapping is attached in Appendix A.

3.0 Subsurface Investigation

Cambium staff completed a borehole investigation at the Site on March 7th to 8th, 2023, to assess subsurface conditions. A total of nine boreholes, designated as BH101-23 through BH109-23, were advanced at the Site to depths ranging from approximately 3.7 to 6.7 meters below ground surface (mbgs). Test pit locations are shown in Figure 4 and test pit logs are included in Appendix B.

3.1 Borehole Logs

Subsurface conditions generally consist of surficial deposits of pavements or topsoil overlying a relatively thin deposit of fill overlying native deposits of clays and silts.

A summary of general lithological details obtained from the investigation is presented below.

Topsoil

Topsoil was encountered from the surface of all boreholes with the exception BH101-23 and BH108-23. The thickness of the topsoil ranges from 0.10 to 0.91 m.

Asphaltic Concrete

Asphaltic concrete was encountered from the surface of BH101-23 and BH108-23 that were advanced in the existing paved areas. The thickness of the asphalt measures 0.08 and 0.05 m in BH101-23 and BH108-23, respectively.

Base Material

Pavement base material was encountered underlying the asphaltic concrete. The base material is composed of brown gravelly sand with some silt. The thickness of the material measures 380 and 560 mm in BH101-23 and BH108-23, respectively.

Fill Material

Fill material other than the pavement structure was encountered at all borehole locations. The fill material varies slightly in composition between borehole locations but is predominantly composed of silty sandy. The material ranges from trace gravel to gravelly, and trace clay was

noted in BH105-23 and BH107-23. Roots were noted within the fill material in BH102-23. The fill material varies in colour between brown and grey depending on location.

The thickness of the fill material ranges from 0.1 to 1.4 m and extends to depth ranging from 0.3 to 1.5 mbgs.

Clayey Silt

Native deposits of grey, sandy, clayey silt were encountered underlying the fill material at all borehole locations at depths ranging from 0.3 to 1.5 mbgs. A notable decrease in clay content was observed in BH103-23 and BH104-23 at a depth of 2.3 mbgs as the material transitions to the non-cohesive underlying deposits.

Boreholes BH108-23 and BH109-23 terminated within the clayey silt deposits at depths of 1.5 mbgs. The deposit was fully penetrated at all other borehole locations. The thickness of the deposits at these locations ranges from 0.9 to 2.3 m, and the deposits extend to depths ranging from 2.3 to 3.2 mbgs.

Silty Sand

A native deposit of grey silty sand was observed in BH101-23 underlying the clayey silt deposit at a depth of 2.6 mbgs. The deposit measures 0.5 m in thickness and extends to a depth of 3.1 mbgs. A seam similar in composition was noted in BH104-23 at a depth of 3.1 mbgs. The seam measured 0.10 m.

Silt

Native deposits of silt were encountered underlying the clayey silt and silty sand in boreholes BH101-23 through BH107-23. The deposit is grey in colour and contains some sand to sandy and trace clay.

The silt deposits were encountered at depths ranging from 2.3 to 3.2 mbgs. Where encountered, all boreholes terminated within the silt at depths ranging from 3.7 to 6.7 mbgs.

Groundwater

Groundwater was observed at all borehole locations during drilling. Unstabilized groundwater level measurements were recorded upon completion of drilling and monitoring wells were installed in three locations (BH105-23, BH106-23, and BH107-23) to enable further characterization. A subsequent monitoring event was completed as part of Phase II ESA work, as well as during hydraulic testing detailed later in this report (Section 4.3). As demonstrated in Table 1, there is significant variability in groundwater levels, which is expected within shallow unconfined aquifers. A figure illustrating the approximate groundwater flow direction based on water levels measured April 19, 2024 is provided in Figure 3.

Table 1 Summary of Measured Water Levels

Borehole ID	Water Level (mbgs)			Water Level (masl)		
	Post-drilling	March 15, 2023*	April 19, 2024	Post-drilling	March 15, 2023*	April 19, 2024
BH101-23	1.1	-	-	97.9	-	-
BH102-23	1.5	-	-	97.2	-	-
BH103-23	0.9	-	-	97.8	-	-
BH104-23	0.6	-	-	98.2	-	-
BH105-23	2.0	1.30	0.62	96.9	98.91	98.29
BH106-23	1.5	0.89	0.30	97.1	98.64	98.34
BH107-23	1.8	1.14	0.36	96.3	98.12	97.76
BH108-23	0.8	-	-	98.3	-	-
BH109-23	1.1	-	-	97.5	-	-

** water level measured prior to well development*

Further well construction details for the three monitoring wells are provided in Table 2.

Table 2 Monitoring Well Construction Details

Well ID	Surface Elevation (masl)	Well Depth (mbgs)	Well Casing Stick-up (mags[1])	Screen Details	
				Top of Screen (mbgs)	Bottom of Screen (mbgs)
BH105-23	98.91	3.06	0.92	0.62	3.06
BH106-23	98.64	2.75	1.00	0.31	2.75
BH107-23	98.12	3.05	0.75	0.61	3.05

[1] *metres above ground surface*

All monitoring wells with water were developed after installation. Development involved purging ten well volumes of groundwater or three times dry from the wells by hand pumping with Waterra tubing and a foot valve.

3.2 Physical Laboratory Testing

Physical laboratory testing, including grain size distribution analysis, was completed on four soil samples to confirm textural classification identified during field logging and obtain percolation rate estimates. Analysis results are based on the Unified Soil Classification System (USCS) scale. A summary of results is provided in Table 3. Complete laboratory analysis reports are provided in Appendix C.

Table 3 Grain Size Distribution Analysis Results

Sample Location	Depth (mbgs)	Description	Gravel (%)	Sand (%)	Silt (%)	Clay (%)	T-time (min/cm)
BH101-23 SS3	1.5 to 2.1	Sandy Clayey Silt	0	22	57	21	40
BH101-23 SS6	3.8 to 4.4	Silt some Sand trace Clay	0	19	77	4	20
BH104-23 SS4	2.3 to 2.9	Sandy Silt some Clay	0	25	57	18	35
BH104-23 SS6	3.8 to 4.4	Sandy Silt trace Clay	0	22	74	4	20

4.0 Hydrogeological Assessment

The results obtained for the shallow groundwater assessment are discussed in the following subsections.

4.1 MECP Well Records Assessment

Cambium accessed the MECP Water Well Information System (WWIS) to review water well records within 500 m of the Site (Ministry of the Environment, Conservation and Parks, 2024b). A total of 73 records were identified, 64 of which describe wells installed into bedrock and 9 installed into overburden. The records identified two monitoring/test wells, two abandoned wells, three recharge wells and the remaining wells were either water supply wells or unknown use. The locations of wells records identified within 500 m of the Site are illustrated in

Figure 4. A summary of water well information, including total depth, static water level, and recommended pumping rate, is presented in Table 4. Further details are provided Appendix D.

One well with well record ID 7448964 is identified to be present at the Site by the WWIS. No details are provided on the record, however.

Table 4 MECP Water Well Information Summary

		Depth (mbgs)	Depth Water Found (mbgs)	Static Water Level (mbgs)	Recommended Pumping Rate (L/min)
Bedrock Wells Count = 64	Minimum	10.7	9.8	1.0	18
	Maximum	101.5	100.6	15.0	182
	Average	32.2	27.8	4.4	56
Overburden Wells Count = 9	Minimum	4.9	13.1	4.0	23
	Maximum	50.0	16.8	5.0	46
	Average	15.9	14.7	4.2	38

A summary of other information outlined in the well records is provided below:

- The general lithology described by the well records is a sequence of overburden overlying limestone which is subsequently underlain by sandstone.

- The overburden is described as predominantly sand which is overlain by a clay layer in some locations. Gravel is also present at depth at some wells.
- The average contact depth between overburden materials and limestone bedrock is 16.5 mbgs (4.0 to 63.4 mbgs).
- Water supply in the area surrounding the Site is primarily derived from the bedrock aquifer. Based on the high static water level recorded compared to the depth that water was found, it is inferred that the bedrock aquifer is at least partially confined.
- The bedrock aquifer is productive, with a geometric mean recommended pumping rate of approximately 56 L/min for bedrock wells.

4.2 Door-to-Door Well Survey

A door-to-door survey of all accessible properties within 500 m of the property was conducted by Cambium staff on April 22nd, 2024, to confirm details in the public record and to identify any wells not included in the MECP records assessment. Due to the commercial and industrial development of the surrounding area, a number of properties were not accessible to the general public. Five properties were visited, and in-person interviews were conducted with available office workers regarding the condition and details of their water supply well(s), including the method of construction, water level, pump intake, well, and water level depths, water use, and general water quality and well yield.

If the property was accessible but a representative was not available, a letter was left in the mailbox with a pre-paid return envelope. The letter explained the nature of the proposed project and the survey and provided direct contact information for Cambium's project manager.

Details and responses from the well use survey are provided in Appendix D. Generally, workers indicated that the water supply for the surrounding area is not good quality due to hardness and suspect iron and sulphur.

4.3 Groundwater Quality

Groundwater quality samples were collected BH106-24 during hydraulic testing activities on April 19, 2024.

Samples were submitted for analysis of general organic and inorganic chemistry to Caduceon Environmental Laboratories in Ottawa, which is accredited by the Canadian Association for Laboratory Accreditation Inc. (CALA). Samples were stored at a temperature between 0°C and 10°C prior to and during transport.

Water quality results were compared against Provincial Water Quality Objectives (PWQO) and City of Ottawa Sewer Discharge Bylaw 2003-514 guidelines. Certificates of Analysis for the samples are included in Appendix E. A summary of parameters exceeding the PWQO and Sewer By-law criteria is provided in Table 5, Table 6, and Table 7.

Table 5 Summary of Results Exceeding PWQO Criteria

Parameter	Units	PWQO Criteria	BH106-23	
			2024/04/22 (Total)	2024/08/10 (Dissolved)
Phosphorus	ug/L	10	**8,720**	<10
Arsenic	ug/L	5	**27.5**	1.0
Cadmium	ug/L	0.1	**1.12**	**0.211**
Cobalt	ug/L	0.9	**103**	**1.1**
Copper	ug/L	5	**301**	**5.4**
Lead	ug/L	1	**76.8**	0.08
Thallium	ug/L	0.3	**1.82**	<0.05
Uranium	ug/L	5	**11.4**	4.68
Vanadium	ug/L	6	**327**	0.3
Benzo[a]anthracene	ug/L	0.0004	<0.05*	-
Benzo(g,h,i)perylene	ug/L	0.00002	<0.05*	-
Butyl Benzyl Phthalate	ug/L	0.2	<1*	-
Chrysene	ug/L	0.0001	<0.05*	-
Dibenzo(a,h)anthracene	ug/L	0.002	<0.05*	-
Fluoranthene	ug/L	0.0008	<0.05*	-
Phenanthrene	ug/L	0.03	<0.05*	-
Formaldehyde	ug/L	0.8	<8*	-
Nonylphenols	ug/L	0.04	<1*	-

Bolded *numbers indicate exceedance with respect to applicable guideline value*

** Laboratory Reporting Limit exceeds PWQO value*

Table 6 Summary of Results Exceeding Storm Sewer By-law Criteria

Parameter	Units	Storm Sewer Criteria	BH106-23	
			2024/04/22 (Total)	2024/08/10 (Filtered/Dissolved)
Total Suspended Solids	mg/L	15	**9,480**	<3
Phosphorus	mg/L	0.4	**8.72**	<0.01
Arsenic	mg/L	0.02	**0.0275**	0.001
Chromium	mg/L	0.08	**0.249**	<0.0011
Copper	mg/L	0.04	**0.301**	0.054

Bolded *numbers indicate exceedance with respect to applicable guideline value*

Table 7 Summary of Results Exceeding Sanitary Sewer By-law Criteria

Parameter	Units	Sanitary Sewer Criteria	BH106-23	
			2024/04/22 (Total)	2024/08/10 (Filtered/Dissolved)
Total Suspended Solids	mg/L	350	**9,480**	<3

Bolded *numbers indicate exceedance with respect to applicable guideline value*

Based on the results of the chemical analysis, the following comments on groundwater quality are made.

- Both the unfiltered and filtered samples had numerous parameters measured at concentrations in excess of PWQO criteria. Treatment of excavation water would be required prior to discharge to off-site surface receiving environments.
- The method detection limit concentrations for many total metals and semi-volatile organics were greater than some of the PWQO criteria for these parameters. This is a limitation of laboratory analysis and is not confirmation that the guideline value was exceeded.
- Total suspended solids (TSS), phosphorus, arsenic, chromium, and copper concentrations were above City of Ottawa Storm Sewer Discharge guidelines in the unfiltered sample. The filtered sample had concentrations less than guideline values for all parameters, indicating that filtration is a suitable treatment method to enable discharge to this receptor.

- The filtered water quality sample had concentrations less than City of Ottawa Sanitary Sewer Discharge guideline values for all parameters, indicating that filtration is a suitable treatment method to enable discharge to this receptor.
- It is recommended that a water quality sample of treated water be submitted for laboratory analysis prior to discharge during construction activities to confirm the treatment system adequately reduces elevated parameters to acceptable concentrations.

4.4 Single Well Hydraulic Tests

Cambium staff visited the Site on April 19th, 2024, to perform in-situ single well hydraulic tests (SWHTs) on select monitoring wells.

Rising head tests were conducted in each well by inducing an instantaneous change in head (water level) in the monitoring wells. Water level changes were achieved by introducing/removing a solid slug.

Water level recovery was monitored using a Solinst Levelogger pressure transducer data logger, with manual measurements collected simultaneously at regular intervals.

The hydraulic conductivity of the geological formations adjacent to the screened portion of each well was estimated via the AquiferTest Pro software using the Hvorslev method (Hvorslev, 1951). A summary of results is presented in Table 8. Detailed analytical reports are provided in Appendix F.

Estimated hydraulic conductivities for the tested wells screened within the silty clay unit ranged between 1.9 $x10^{-9}$ and 2.2 $x10^{-7}$ m/s, with an overall geometric mean value of 1.2 $x10^{-8}$ m/s. These values are consistent with published values for the tested materials (unconsolidated silt) (Freeze & Cherry, 1979).

Table 8 Hydraulic Conductivity Estimates derived via SWHTs

Monitoring Well	Screened Lithology	Hydraulic Conductivity, K (m/s)			
		Test 1	Test 2	Test 3	Geometric Mean
BH105-24	Silty sand to Sandy clayey silt	6.4×10^{-9}	3.4×10^{-9}	-	4.6×10^{-9}
BH106-24	Sandy clayey silt	2.2×10^{-7}	1.9×10^{-7}	2.1×10^{-7}	2.1×10^{-7}
BH107-24	Sandy clayey silt to silt	1.9×10^{-9}	-	-	1.9×10^{-9}
Geometric Mean					**1.2×10^{-8}**

5.0 Dewatering Assessment

The requirements for construction dewatering generally depend on the Site's soil and groundwater conditions including soil type, soil permeability or hydraulic conductivity, local groundwater levels, and the design of the proposed works, such as the foundation/basement elevation or pipe invert level, as well as the size of proposed structure/excavation. The following subsections detail the specific excavation parameters and anticipated dewatering rates for the Site.

5.1 Excavation Design Parameters

It is understood that the footprint of the proposed slab-on-grade building will be approximately 1,110 m^2.

For construction purposes, it is assumed that excavation for footings will occur along a linear perimeter with dimensions of 23 m by 55 m. It is further assumed that during footing emplacement, groundwater will be temporarily lowered to a minimum of 1 m below the frost line to ensure dry conditions during footing construction, to a total depth of 2.5 mbgs.

For permanent operations, due to the high-water levels at the Site, permanent dewatering will be required to ensure water levels beneath the building remain below the frost line level (approximately 1.5 mbgs) throughout the year. A maximum water level of 0.30 mbgs was measured in BH106-23 on April 19, 2024.

5.2 Estimated Dewatering Rate – Construction Phase

An estimated dewatering rate for the construction phase of the proposed development was calculated a modified Dupuit-Forchheimer equation developed for linear excavations according to Powers, Corwin, Schmall, & Kaeck (2007):

$$Q = \frac{\pi K(H^2 - h^2)}{\ln(R_0/r_s)} + 2\left[\frac{xK(H^2 - h^2)}{2L}\right]$$

$Where$:

$Q = dewatering\ rate\ (m^3/s)$

$K = hydraulic\ conductivity\ (m/s)$

$H = initial\ hydraulic\ head\ in\ aquifer\ (m)$

$h = target\ hydraulic\ head\ (initial\ hydraulic\ head - target\ drawdown)\ (m)$

$R_0 = distance\ to\ radial\ source\ (from\ excavation\ center)$

$r_s = equivalent\ single\ well\ radius = width\ of\ trench/2\ (m)$

$x = unit\ length\ of\ trench\ (m)$

$L = distance\ to\ line\ source\ (from\ excavation\ center) = R_0/2\ (m)$

A summary of calculated dewatering rates for per 50 m linear excavation, given a target depth to water of 2.5 mbgs, is provided in Table 9. Detailed calculations are provided in Appendix G.

Table 9 Calculated Construction Dewatering Rates

	Hydraulic Conductivity (K)	Radius of Influence (from excavation edge)	Dewatering Rate (Q)	
	m/s	m	m^3/day	L/s
Minimum	1.9 x10^{-9}	0.3	0.14	0.002
Maximum	2.1 x10^{-7}	3.0	4.70	0.05
Geometric Mean	1.2 x10^{-8}	0.7	0.65	0.01

Using the hydraulic conductivity estimates presented in Table 9, the estimated radius of influence from the edge of the excavation ranges from 0.3 to 3.0 m (average 0.7 m). The estimated dewatering rate ranges from 0.14 m^3/day (140 L/day, or 0.002 L/s) to 4.70 m^3/day (4,700 L/day, or 0.05 L/s), with a geometric mean average value of 0.65 m^3/day (650 L/day, or 0.01 L/s).

Applying a safety factor of 2 to account for uncertainty resulting from heterogeneity of subsurface materials and other unknown factors, the estimated dewatering rate for 50 m sections of footing excavation ranges from 0.28 m^3/day (280 L/day, or 0.004 L/s) to 9.4 m^3/day (9,400 L/day, or 0.10 L/s), with a geometric mean average value of 1.30 m^3/day (1,300 L/day, or 0.02 L/s).

It is noted that the above equation is designed to represent steady state pumping conditions. In general, at the beginning of the pumping, the pumping rate required to lower Site water levels to acceptable levels may be greater than the rate estimated for steady state conditions as incoming water replaces the volume of excavated soils. Additionally, the above equation does not account for any precipitation that may occur during the construction process.

5.3 Estimated Dewatering Rate – Operational Phase

An estimated dewatering rate for the operational phase of the proposed development was calculated using a modified Dupuit-Forchheimer equation (Powers, Corwin, Schmall, & Kaeck, 2007). Calculations for a square dewatering area with an equivalent radius were employed.

$$Q = \frac{\pi K(H^2 - h^2)}{\ln(R_0/r_s)}$$

$Where$:

$Q = dewatering\ rate\ (m^3/s)$

$K = hydraulic\ conductivity\ (m/s)$

$H = initial\ hydraulic\ head\ in\ aquifer\ (m)$

$h = target\ hydraulic\ head\ (initial\ hydraulic\ head - target\ drawdown)\ (m)$

$R_0 = zone\ of\ influence\ (from\ excavation\ center) = 3000(H-h)\sqrt{K}\ (m)$

$r_s = equivalent\ single\ well\ radius$

For square excavations, the equivalent radius (r_s) can be determined as the radius of a circle with the same area as the excavation, or with the same perimeter as the excavation.

Here, the equivalent area method was used such that

$$r_s = \sqrt{\frac{excavation\ area}{\pi}}$$

A summary of calculated dewatering rates for per 50 m linear excavation, given a target depth to water of 2.5 mbgs, is provided in Table 10. Detailed calculations are provided in Appendix G

Table 10 Calculated Permanent Dewatering Rate

	Hydraulic Conductivity (K)	Radius of Influence (from excavation edge)	Dewatering Rate (Q)	
	m/s	m	m^3/day	L/s
Minimum	1.9×10^{-9}	0.2	0.4	0.005
Maximum	2.1×10^{-7}	1.6	4.6	0.05
Geometric Mean	1.2×10^{-8}	0.4	1.1	0.01

Using the hydraulic conductivity estimates presented in Table 10, the estimated radius of influence from the edge of the building footprint ranges from 0.2 to 1.6 m (average 0.4 m). The estimated dewatering rate ranges from 0.4 m^3/day (400 L/day, or 0.005 L/s) to 4.6 m^3/day (4,600 L/day, or 0.05 L/s), with a geometric mean average value of 1.1 m^3/day (1,100 L/day, or 0.01 L/s).

Applying a safety factor of 2 to account for uncertainty resulting from heterogeneity of subsurface materials and other unknown factors, the estimated permanent dewatering rate for the building footprint ranges from 0.8 m^3/day (800 L/day, or 0.01 L/s) to 9.2 m^3/day (9,200 L/day, or 0.10 L/s), with a geometric mean average value of 2.2 m^3/day (2,200 L/day, or 0.02 L/s).

It is noted that the above calculations are an approximation only, which can be further refined based on results observed during the construction phase of the proposed development. Cambium recommends reassessment of dewatering rates once construction nears the completion stage.

5.4 Assessment of Required Regulatory Permits or Registration

Any construction dewatering or other water taking in Ontario is governed by the Ontario Water Resources Act (OWRA) (Ontario Regulation 387/04 and/or Ontario Regulation 63/16) and/or the Environmental Protection Act (Registrations under Part II.2).

As of July 1, 2025, O.Reg. 63/16 will be amended such that temporary construction dewatering greater than 50,000 L/day registration of the water taking must be completed through the

Environmental Activity and Sector Registry (EASR) prior to the start of dewatering. Additionally, O.Reg. 387/04 will be amended such that low-risk foundation drainage systems, used primarily for residential purposes, that take less than 379,000 L/day of groundwater will be exempt from requiring environmental permissions.

As the maximum estimated dewatering rate for both construction activities and long-term building operation is less than 9,500 L/day, neither a PTTW nor an EASR registration will be required for the proposed development.

6.0 Water Supply Assessment

6.1 Test Well Installation and Inspection

Test Well 1 (TW1; Well Tag No. A379053, Appendix D) was installed by Air Rock Drilling Company on May 21, 2025. TW1 was completed in a landscaped area in the southeast corner of the Site to a depth of 55 mbgs. The identified lithology is clay from 0 to 11.6 mbgs, boulders/hardpan from 11.6 to 14.7 mbgs, and limestone bedrock to completion depth. Three water bearing units of indeterminate thickness were identified at 22.6, 38.2, and 53 mbgs.

The borehole has a 0.025 m diameter from ground surface to 16.5 mbgs, and a diameter of 0.016 m from 16.5 to 55 mbgs. A 0.016 m inside diameter steel casing was installed from 0.6 m above ground surface to a depth of 16.5 mbgs. Grout was emplaced in the annular space around the casing. A Cambium technician, under the supervision of the hydrogeologist who signed this report, observed the installation and grouting of the well casing (no well screen was installed). The signed and sealed well inspection report certifying that the well meets the minimum well construction requirements in the Wells Regulation and recommendations in this report is provided in Appendix D.

The remaining borehole was then completed and left as open hole in limestone bedrock. All three water bearing zones are below the bottom of the casing. The driller's well yield test provided an estimated pumping rate of 57 L/min, and the recommended pump depth was 30 mbgs.

6.2 Hydraulic Pumping Test

An 8-hour hydraulic pumping test was completed on TW1 by Cambium staff on May 29, 2025. Prior to the test, a Solinst Levelogger (logger) was installed in TW1 and OW1 (the pre-existing water supply well on the site) to monitor water levels before, during, and after the pumping test. Manual measurements were also recorded during the pumping tests to mitigate the possibility of equipment failure. Well water levels measured during pumping test activities are provided in Appendix H. OW1 was not used for at least 12 hours prior to the start of the pumping test, nor was it used during the test or subsequent recovery period.

TW1 was chlorinated by Air Rock Drillers 48 hours prior to testing. The static water level in TW1 prior to the pumping test was 1.73 mbgs and the pump was installed at approximately 50 mbgs, resulting an available drawdown of approximately 48.21 m (height of static water level above pump).

Water from the pumping test was discharged to the drainage ditch at the perimeter of the site, in a downslope direction approximately 15 m from the test well. The pumping rate for the test was controlled by a valve on the discharge line.

Hydraulic testing began at 8:03 a.m. for a duration of 8 hours. The total sewage design flow for the proposed development is 1,800 L/day (Section 8.1.1). Assuming water use is limited to a standard (8-hour) working day, this corresponds to an average rate of 225 L/hour (3.75 L/min).

To account for periods of peak demand, the flow rate during the initial 15 minutes of the test was set to 5 times the average demand (approximately 19 L/min). The pumping rate was then increased and maintained at approximately 10 times the average demand (38 L/min) for the remainder of the test. The total volume of water discharged from TW1 during the pumping test was approximately 17,955 L.

Rainfall of 11.6 mm was recorded at the Ottawa Airport Climate Station (ID # 6106001) on the day of the pumping test (Appendix H). This is reflected in monitoring data collected in TW1 and OW1 during the pumping test. After an initial water level decrease up to 0.25 m within the first hour following the start of pumping, the water level in TW1 gradually increased for the duration of the test. A similar trend was observed in OW1, which experienced a maximum drawdown of approximately 0.1 m within the first hour before progressively increasing throughout the day. Water level fluctuations in TW1 and OW1 mirrored each other, both in terms of timing and magnitude (Appendix H).

The pump in TW was shut off at 4:03 pm. At this time, the water level in TW1 was 1.79 mbgs, which is equivalent to a water level increase of 0.06 m since the start of testing and represents approximately 0.1% of the total available drawdown in the well.

Following pump cessation, water levels were measured for 60 minutes. The water level recovered to greater than 100% of the initial water level in both TW1 and OW1 immediately upon termination of the pumping test.

6.3 Aquifer Parameter Analysis

Drawdown measurements recorded for TW1 during the pumping test were analyzed with Aqtesolv software to obtain an estimate of transmissivity for the water supply aquifer using the Theis method. Although transmissivity of the aquifer is inferred to be very high due to the negligible drawdown over the course of the pumping test, concurrent recovery of the aquifer(s) during the test precludes a reliable estimate of the precise value. A report for the aquifer analysis illustrating the recharge trend in the data is included in Appendix H. Although results are presented based on a Theis analysis of the results, they are considered highly uncertain.

6.4 Groundwater Quality Analysis

Field water quality parameters were measured regularly during pumping to ensure baseline aquifer water qualities were established prior to sampling. Field parameter measurements are summarized in Table 11. All water testing equipment was calibrated prior to use as per manufacturer's instructions; further details about equipment type.

General chemistry parameters (pH, temperature, conductivity, ORP, and DO) were measured using a YSI ProDSS Multiparameter Digital Water Quality Meter. Calibration is completed for each sensor once per week using calibration solutions purchased from Maxim Environmental. For calibration, the YSI probe is placed in a calibration solution, and the calibration is run for the specific parameter. This is repeated for each sensor/parameter.

Turbidity was measured using a HACH 2100Q #50166. Calibration is completed once per week using calibration samples purchased from Maxim Environmental. For calibration, the meter will prompt for one standard and a time. The meter will store the readings and adjust calibration based on the standards. Calibration verifications are completed once per use prior to testing.

Chlorine residual was measured using a Hanna HI701-11 Free Chlorine Checker HC. Calibration verifications are completed once per use prior to testing using Hanna HI701-11 certified standard cuvettes. The unit is calibrated annually by the manufacturer. Residual chlorine was monitored during the supplemental sampling event and was confirmed to be less than 0.01 ppm before sample collection occurred.

Two sets of water quality samples were collected from TW1 and analyzed for the subdivision suite as well as trace metals and volatile organic compounds. The first sample (TW1-1) was collected three hours into the pumping test, and the second sample (TW1-2) within the final hour of the test.

Samples were collected in laboratory supplied containers which included preservatives as required. They were subsequently stored at a temperature between 0 and 10 °C prior to and during transport. Samples were submitted along with laboratory supplied COC forms to Caduceon Environmental Laboratories in Ottawa, Ontario, which is accredited by the Canadian Association for Laboratory Accreditation Inc. All samples were submitted within the required hold-time period.

Table 11 Pumping Test Field Parameter Measurements

Test Hour	Temperature (°C)	Dissolved Oxygen (mg/L)	Electrical Conductivity (µs/cm)	pH	Oxygen Reduction Potential (mV)	Turbidity (NTU)	Chlorine (mg/L)
1	11.2	1.69	663	7.37	-9.4	4.44	<0.01
2	11.5	1.82	669	7.34	-9.4	5.89	<0.01
3	11.6	1.99	682	7.33	-9.3	7.05	<0.01
4	11.4	1.94	684	7.31	-9.6	5.21	<0.01
5	11.3	1.92	688	7.32	-12.8	3.95	<0.01
6	11.7	1.92	690	7.36	-28.2	2.83	<0.01
7	11.8	1.91	691	7.42	-46.1	2.62	<0.01

Water quality results were compared against the Ontario Drinking Water Quality Standards (ODWQS) criteria for parameters outlined in Procedure D-5-5 Tables 1, 2, and 3 (Ministry of the Environment, 1996a). A complete summary of water quality results and certificate of lab

analyses are provided in Appendix E. Parameters reported at concentrations exceeding ODWQS criteria are outlined in Table 12.

Table 12 Summary of Results Exceeding ODWQS Standards

Parameter	Units	ODWQS Criteria	TW Concentration	
			TW1-1	TW1-2
Hardness (as $CaCO_3$)	mg/L	80-100	**389**	**394**
Total Dissolved Solids (Ion Sum)	mg/L	500	**510**	**522**
Turbidity	NTU	5	**8.4**	3.4
Total Iron	mg/L	0.3	0.205	**0.326**
Sodium	mg/L	20 / 200	**38.9**	**40.0**

As suggested by the field parameter measurements, water quality was consistent between samples. All measured parameters were less than the corresponding health related criteria. Hardness, total dissolved solids, turbidity, and total iron exceeded their respective aesthetic/operational guidelines but are below the corresponding Maximum Concentration Considered Reasonably Treatable (MCCRT). Turbidity decreased significantly between sampling events, suggesting well development during pumping resolved the issue.

A high TDS value can indicate some issues with scale buildup as well as aesthetic issues in taste and appearance. Common reported aesthetic issues include a salty or bitter taste as well as a hazy or cloudy appearance. It should be noted that both water quality samples exceeded the OWDQS aesthetic objective of 500 mg/L for TDS by a minimal amount, 510 mg/L for TW1-1 and 522 mg/L for TW1-2. A reverse osmosis unit would reduce TDS concentration if required

Langelier Saturation Index (LSI) and Ryznar Stability Saturation Index (RSI) was not included in the lab analysis suite for the secondary sample collected from TW-1. Considering the water quality results for temperature, calcium concentration, alkalinity, pH, TDS preliminary results for an LSI value would be +0.35, indicating slight scaling, and an RSI value of 7.25 indicating slight corrosion potential. The water would be considered balanced and convention water softening and reverse osmosis treatment can be utilized as needed to address any operational or aesthetic issues as needed.

Alongside the high TDS value, concentrations of chloride, sulphate, calcium, manganese, potassium, and bicarbonates were detected, however not elevated above the ODWQS criteria. Sodium exceeded the maximum acceptable concentration of 20 mg/L, but not above the aesthetic objective. Elevated sodium presents a risk for those who are on restricted sodium diets. Sodium, along with the other parameters discussed above, can be treated with the use of reverse osmosis systems.

Hardness is typically treated through the use of a water softener. A typical water softener uses salt to reduce calcium and magnesium. As sodium is already elevated in the drinking water, a salt-free water conditioner is recommended (i.e. potassium chloride) to prevent scale build-up and reduce hardness.

Iron can be treated with the use of a water softener. Dedicated iron water filters are available in the event the softener is not sufficiently reducing iron. Considering the aesthetic objective for iron is 0.3 mg/L and the two samples collected had concentrations of 0.205 mg/L for TW1-1 and 0.326 mg/L for TW1-2, a water softener would be expected to reduce total iron below 0.3 mg/L.

A water treatment specialist should be consulted to confirm appropriate treatment options.

A detailed assessment of surrounding land use was completed during the Phase Two ESA (Cambium, 2023b). All contaminants of potential concern were less than the Table 6 Site Characterization Standards in all soil and groundwater samples. All VOC concentrations measured during the pumping test were below the project laboratory’s limit of reporting and indicate there are no significant impacts to the quality of the water supply aquifer from historical activities at the Site or surrounding lands.

7.0 Water Balance Assessment

A water balance assessment was completed to determine the potential change in groundwater recharge that could occur due to the proposed development. Generally, any property can be categorized into three broad types of areas: paved, roof, and landscape/vegetated. Currently, the Site is developed as a car wash, with paved roadways and parking and landscaping around the existing septic bed. In the post-development scenario, the amount of paved and roof areas at the Site will increase and the amount of landscape/vegetated area will decrease. This has the potential to impact the amount of water that infiltrates into the ground and is available to replenish natural ground- and surface-water systems, which must be considered as part of the development process.

To compare the difference in infiltration that may result from the proposed development, a water balance calculation was completed to determine the amount of surplus water that is currently generated at the Site. Site characteristics such as surficial soil type, topography, and the amount of pervious and impervious areas were then used to estimate the volume of water infiltrating at the Site. Calculations were completed for both pre-and post-development scenarios, so that a comparison could be made to identify potential changes in infiltration as well as mitigation measures which could be employed to reduce development impacts.

Figure 6 presents the post-development plans of the proposed development. As a detailed breakdown of landscape and building details are yet to be determined, the paved, roof, and landscape areas for the developed lots were calculated based on an assumption that each surface type comprises 10%, 50%, and 40% of the total developed lot area, respectively. Table 13 provides a summary of statistics for the total areas for each type of surface at the Site for both pre- and post-development scenarios. Further discussion of each component completed for the water balance assessment is provided in the following subsections.

Table 13 Summary of Pre- and Post-Development Areas

Type of Land Coverage	Pre-Development Areas (m^2)	Post-Development Areas (m^2)
Paved Area	855	2,236
Roof Area	353	1,261
Landscape/Vegetated Area	3,472	1,183
Total (m^2)	**4,680**	**4,680**

7.1 Water Budget and Total Water Surplus

Based on the Thornthwaite and Mather methodology (1957), the water balance is an accounting of water in the hydrologic cycle. Precipitation (P) falls as rain and snow. It can run off towards lakes and streams (R), infiltrate to the groundwater table (I), or evaporate from the ground or be used for transpiration by vegetation (ET). When long-term average values of P, R, I, and ET are used, there is minimal or no net change to groundwater storage (ΔS).

The annual water budget can be expressed as:

$$P = R + I + ET + \Delta S$$

Where:

P = Precipitation (mm/year)

R = Run-off (mm/year)

I = Infiltration (mm/year)

ET = Evapotranspiration (mm/year)

ΔS = Change in soil water storage (mm/year)

Total water surplus is defined as the difference between precipitation and evapotranspiration. It is the amount of water per unit area that can either infiltrate into on-site soils or be directed off-site as runoff. An assumption for the calculation of water surplus is that changes in soil water storage are negligible over the course of a year. It is also assumed that the catchment area for the water balance described above is completely contained within Site boundaries (i.e. the model does not account for catchment areas that extend off-site).

An annual water budget for the Site was calculated using the thirty-year climate normal data (1981-2010) provided by Environment Canada for the Ottawa MacDonald-Cartier International Airport (Climate ID 6106000), located approximately 114 km north (Environment Canada, 2024). A detailed table outlining the calculations is provided in Appendix I. In summary, the average annual precipitation and evapotranspiration at the Site is estimated to be 944 mm/year and 547 mm/year, respectively. Therefore, the water surplus at the Site is estimated to be 397 mm/year.

7.2 Annual Infiltration and Runoff

To determine the amount of water infiltrated into on-site soils annually, the total volume of water available is multiplied by an infiltration factor (IF). The total volume of water available is obtained by multiplying the water surplus value determined from the water balance described above by the total permeable landscape area at the Site. The infiltration factor, which ranges from 0 to 1, is estimated based on topography, soils and cover as per the Stormwater Management Planning and Design Manual (Ministry of the Environment, 2003). As outlined in Table 14, the infiltration factor at the Site was assigned a value of 0.45.

Table 14 Determination of Infiltration Factor

Factor	Value
Topography	Rolling to Hilly Land= 0.15
Soil	Silty Loam = 0.2
Cover	Cultivated Land = 0.1
Infiltration Factor (IF)	**0.45**

The annual volume of water that infiltrates at the site is calculated as follows:

$$I\ (m^3/year) = Water\ Surplus\ (m/year) * Total\ landscape\ area(m^2/year) * Infiltration\ Factor$$

The annual infiltration at the Site is expected to vary based on a number of factors (i.e. actual precipitation, variation in soil composition, soil compaction, etc.).

The annual runoff that occurs at the Site varies between permeable and impermeable surfaces. On permeable landscape surfaces, the runoff is calculated as the difference between total precipitation and annual infiltration. On impermeable surfaces where there is no

infiltration, the runoff is calculated as 90% of precipitation, with the remaining 10% of precipitation lost directly to evaporation.

Annual infiltration and runoff volumes were calculated for the Site for both pre- and post-development scenarios. Details of the calculations are provided in Appendix I. A discussion of the water balance used to calculate the infiltration and runoff volumes for each scenario is provided in Section 7.3 and Section 7.4.

7.3 Pre-Development Water Balance

The water balance for existing conditions at the Site is summarized in Table 15. The pre-development infiltration rate and runoff rate was calculated to be 620 m³/year and 1,784 m³/year, respectively.

Table 15 Pre-Development Water Balance

Land Use		Area (m²)	Precipitation (m³)	Evapotranspiration (m³)	Infiltration (m³)	Run-off (m³)
Impervious Areas	Paved Area	855	807	81	-	726
	Roof Area	353	333	33	-	300
Pervious Areas	Landscape Area	3,472	3,278	1,899	620	758
Total		**4,680**	**4,418**	**2,013**	**620**	**1,784**

7.4 Post-Development Water Balance

The water balance for proposed conditions at the Site is summarized in Table 16. The post-development infiltration rate and runoff rate was calculated to be 211 m³/year and 3,229 m³/year, respectively.

Table 16 Post-Development Water Balance

Land Use		Area (m^2)	Precipitation (m^3)	Evapotranspiration (m^3)	Infiltration (m^3)	Run-off (m^3)
Impervious Areas	Paved Area	2,236	2,111	211	-	1,900
	Roof Area	1,261	1,190	119	-	1,071
Pervious Areas	Landscape Area	1,183	1,117	647	211	258
Total		**4,680**	**4,418**	**977**	**211**	**3,229**

Assuming no infiltration occurring in paved and roof areas, and 10% of precipitation to be evaporated from paved and roof areas.

7.5 Water Balance Comparison

A comparison of water balances for the pre-development and post-development scenarios is summarized in Table 17. There is a net infiltration deficit of approximately 409 m^3/year, compared to the pre-development infiltration. The run-off rate upon development of the Site is projected to increase by 1,445 m^3/year.

Table 17 Water Balance Comparison

	Precipitation (m^3)	Evapotranspiration (m^3)	Infiltration (m^3)	Run-off (m^3)
Pre-Development	4,418	2,013	620	1,784
Post-Development	4,418	977	211	3,229
Change in Volume	**-**	**-1,036**	**-409**	**1,445**
Change in %	**-**	**-51**	**-66**	**81**

7.6 Required Infiltration from Roof Runoff

To compensate for the post-development infiltration deficit, a portion of roof run-off water can be captured and directed towards infiltration. As the infiltration deficit volume is 409 m^3 /year and the total roof run-off volume is projected to be 1,071 m^3/year, the percentage of roof run-off that is required to be redirected to maintain pre-development infiltration volumes is 38%. These details are summarized in Table 18.

Table 18 Requirement of Infiltration from Roof Runoff

Volume of Pre-Development Infiltration (m^3/year)	620
Volume of Post-Development Infiltration (m^3/year)	211
Deficit from Pre to Post Development Infiltration (m^3/year)	409
Percentage of Roof Runoff required to match the pre-development infiltration (%)	38

7.7 Water Balance Assessment Summary

Based on the calculations detailed in the preceding subsections, a summary of the water balance assessment is as follows:

- Impervious post-development area (roof and pavement) is projected to increase by approximately 1,945 m^2 when compared to pre-development conditions.
- Without implementing any mitigation measures, it is estimated that the reduction of pervious surfaces at the Site will create a net deficit in infiltration of approximately 409 m^3/year.
- To regain the lost volume of water infiltrated, a diversion of approximately 38% of roof run-off would be required to maintain pre-development water balance conditions (assuming 100% of diverted water is infiltrated).
- Implementation of Low Impact Development measures would enhance the Site's ability to infiltrate diverted roof run-off water into pervious areas. Due to the high groundwater levels however, a civil design engineer should be involved in designing any suitable infiltration measures across the Site
- The infiltration deficit will be maintained to pre-development volumes through the use of infiltration features. Please refer to the D.B Grey Engineering documents (submitted under separate cover) for stormwater management information.

8.0 Wastewater Assessment

8.1 Conceptual Wastewater Design

Part 8 of the Ontario Building Code (OBC) details the design, construction, operation, and maintenance of sewage systems. A conceptual peak sewage design flow was calculated following a review of OBC Table 8.2.1.3.B is summarized as follows:

- Warehouse: 150 L/day/loading bay x 4 loading bays = 600 L/day
- Factory: 75 L/employee per 8 hr shift x 16 person occupancy = 1,200 L/day
    - Total sewage design flow = **1,800 L/day**

8.1.1 Concept Design Details

A daily sewage design flow volume of 1,800 L/day is calculated for the proposed light industrial building.

8.1.2 Treatment Unit

It is understood the client is proposing to use a Waterloo Biofilter advanced treatment system which includes:

- Anaerobic Digestor with Internal Pump Chamber (Model ADIPC-6000)
- Biofilter Tank (Model BFCN-4800)
- WaterNOx-LS Tank (for nitrogen removal)

8.1.3 Leaching Bed

Following the subsurface investigation, native soils were observed to be similar, consisting of a surficial layer of topsoil and silty sand fill to depths ranging from 0.3 to 1.0 mbgs overlying sandy clayey silt and sandy silt. Groundwater was encountered between 0.6 and 2.0 mbgs across all boreholes. Soil sample results are summarized in Section 3.2 above and have estimated percolation rates between 20 and 40 min/cm.

Considering the available land constraints and using a conservative estimated percolation rate of 40 min/cm, a partially raised Type A area bed has been conceptually designed below using the following information and calculations:

- Design flow (Q) = 1,800 L/day
- Native Soil T-time (T) = 40 min/cm
- Configuration: partially raised
- Stone area = Q/75 when Q < 3,000 L/day = 1,800/75 = 24 m^2
  - Proposed concept design: 5.6 m x 4.5 m = 25.2 m^2
- Mantle area (imported sand fill) = QT/400 = 1,800 x 40 / 400 = 180 m^2
  - Proposed concept design: 21.6 m x 8.5 m = 183.6 m^2

Based on the filter bed mantle requirement, the total bed footprint would be approximately 21.6 m by 8.5 m, as shown on Figure 7.

The Type A Area Bed will likely require to be raised above original grade. Assuming a raised height of 1.0 m, setback distances shown on Figure 7 were increased accordingly.

The area of the Site appears to provide adequate space for the installation of an on-site sewage system and appears to meet the required setback distances outlined in OBC Tables 8.2.1.6.A and 8.2.1.6.B. However, this should be considered and evaluated during the detailed sewage system design stage. The Site conditions appear feasible to install an on-site sewage system.

8.2 Septic System Impact Assessment

Guideline D-5-4 (Ministry of the Environment, 1996b) outlines a three-step process for assessing potential groundwater impact from individual on-site sewage systems. The first two steps involve lot size and system isolation considerations. If risk is identified through either of these two steps, the assessment must progress to the third step, which is detailed consideration of nitrate loading and contaminant attenuation.

8.3 Step One: Lot Size Consideration

As the Site size is less than 1 ha, the assessment automatically progresses to Step Two.

8.4 Step Two: System Isolation Considerations

Water supply at the Site and surrounding area is predominately sourced from a bedrock aquifer which is overlain by a significant layer of overburden material (Section 4.1). Given this information, it is expected that the water supply aquifer will be hydraulically isolated from the proposed septic system at the Site. Regardless of the potential isolation, based on the small lot site size and the large amount of impermeable ground surface, nitrate loading is a consideration for the Site. As such, the assessment progresses to Step Three.

8.4.1 Step Three: Assessment of Nitrate Loading and Contaminant Attenuation

A daily flow of 1,800 L/day of sewage effluent is anticipated at the Site. Total nitrogen (all species) ultimately converts to nitrate through the wastewater treatment process. Nitrate is considered to be the critical contaminant in sewage effluent. A nitrate loading of 40 grams/lot/day is typically used to determine the effluent loading from conventional septic systems on the receiving groundwater system. The proposed Waterloo Biofilter advanced treatment system, (Section 8.1.2), has a nitrate reduction system (WaterNOx-LS) which takes a nominal amount of additional space and can achieve between 80.3 and 91.6% reduction in total nitrogen (Appendix J). Provided the WaterNOx-LS tank is installed and using the conservative 80.3% total nitrogen reduction, the system will have a theoretical nitrate loading of 7.88 g/day. This value is used in the following equations.

The volume of available dilution water (Q_i)

$$Q_i(\frac{L}{day}) = Surplus\left(\frac{m}{day}\right) * Infiltration\ Factor * Permable\ Area\ (m^2) * 1000\frac{L}{m^3}$$

A mass balance calculation is used to determine the sewage loading for nitrate on the property boundary:

$$C_t = \frac{Q_e C_e + Q_i C_i}{Q_t}$$

Where:

Q_t = Total volume (Q_e + Q_i)

C_t = Total concentration of nitrate at the property boundary

Q_e = Volume of septic effluent

C_e = Concentration of nitrate in effluent (7.88 mg/L)

Q_i = Volume of available dilution water

C_i = Concentration of nitrate in infiltration water (0.1 mg/L)

8.4.2 Estimate of Nitrate Concentrations at Lot Boundaries

The predictive assessment indicates the proposed development will result in an estimated nitrate concentration of 5.99 mg/L at lot boundaries if wastewater is treated via the proposed Waterloo Biofilter advanced treatment system and only dilution water from infiltration within permeable areas is considered. The treatment system capable of 80.3% or greater nitrate reduction is well below the ODWQS criteria of 10 mg/L using only dilution water from infiltration within permeable areas.

A summary of these results is provided in Table 19. Detailed calculations are included in Appendix I.

Table 19 Predictive Assessment of Nitrate Concentration

Variable	Waterloo Biofilter Advanced Treatment System
Q_e (L/day)	1,800
C_e (mg/L)	7.88
Q_i (L/day)	579
C_i (mg/L)	0.1
Q_t (L/day)	2,379
C_t (mg/L)	5.99

9.0 Conclusions and Recommendations

Cambium was retained by the Client to complete a hydrogeological assessment for proposed redevelopment of the land located at 1386 and 1394 Greely Lane, Ottawa, Ontario. Development plans include construction of one 1,110 m^2 (12,000 ft^2) slab-on grade building which will be divided in three 370 m^2 (4,000 ft^2) light industrial use units.

The subsurface investigation completed at the site indicates the lithology is comprised primarily of surficial deposits of pavements or topsoil overlying a relatively thin deposit of fill overlying native deposits of clays and silts. T-times estimated from laboratory analysis of soil samples collected from the native deposits range from 20 to 40 min/cm.

Monitoring wells installed in three locations (BH105-23, BH106-23, and BH107-23) indicate water levels vary across the site and fluctuate seasonally. A minimum water level of 1.3 mbgs was measured in BH105-23 on March 15, 2023, and a maximum water level of 0.30 mbgs was measured in BH106-23 on April 19, 2024. Hydraulic testing (rising head slug tests) provided hydraulic conductivity estimates for the shallow aquifer ranging from 1.9 $x10^{-9}$ to 2.2 $x10^{-7}$ m/s with a geometric mean estimate of 1.2 $x10^{-8}$ m^2/s.

9.1 Construction Dewatering

Water Quality Analysis

Analysis of water quality samples from BH106-23 identified a number of parameters with concentrations exceeding PWQO criteria in both unfiltered and filtered samples. All parameters had concentrations below City of Ottawa storm and sanitary sewer discharge guidelines, indicating that filtration is a suitable treatment method to enable discharge to these receptors. Should on-site treatment and discharge to surface (i.e. drainage ditch) be the preferred option for dewatering, it is recommended that a water quality sample of treated water be submitted for laboratory analysis prior to discharge during construction activities to confirm the treatment system adequately reduces elevated parameters to acceptable concentrations.

Dewatering Assessment

Due to the high groundwater levels at the Site, dewatering during both the construction phase and permanent building operation will be required. During construction, it is estimated than an average dewatering rate of 1.30 m^3/day (1,300 L/day, or 0.02 L/s) will be needed to achieve dry conditions per 50 m section of footing excavation. This rate represents steady state pumping conditions and higher volumes may be required to lower Site water levels to acceptable levels during the initial stage of pumping. Additionally, the estimate does not account for any precipitation that may occur during the construction process.

For permanent operations, it is estimated that an estimated average dewatering rate of 2.2 m^3/day (2,200 L/day, or 0.02 L/s) will be required to ensure water levels beneath the building remain below the frost line level (approximately 1.5 mbgs) throughout the year. It is recommended that dewatering rates be reassessed however, once building construction nears the completion stage.

The maximum estimated dewatering rate for both construction activities and long-term building operation are less than 9,500 L/day. As such, neither a PTTW nor an EASR registration will be required for the proposed development.

The monitoring wells installed for the hydrogeological assessment should be decommissioned in accordance with O.Reg. 903 prior to redevelopment of the Site.

9.2 Private Servicing

Water Supply

Test Well 1 was installed on May 21, 2025, in a landscaped area in the southeast corner of the Site to a depth of 55 mbgs. The identified lithology is clay from 0 to 11.6 mbgs, boulders/hardpan from 11.6 to 14.7 mbgs, and limestone bedrock to completion depth. Three water bearing units of indeterminate thickness were identified at 22.6, 38.2, and 53 mbgs. A Cambium technician observed the installation and grouting of the well casing (no well screen was installed).

An 8-hour hydraulic pumping test was completed on TW1 by Cambium staff on May 29, 2025 and the pre-existing water supply well on the Site was used to monitor water levels before, during, and after the pumping test. OW1 was not used for at least 12 hours prior to the start of the pumping test, nor was it used during the test or subsequent recovery period.

The total sewage design flow for the proposed development is 1,800 L/day corresponds to an average rate of 3.75 L/min for an 8-hour business day. To account for periods of peak demand, the flow rate during the initial 15 minutes of the test was set to 5 times the average demand (approximately 19 L/min). The pumping rate was then increased and maintained at approximately 10 times the average demand (38 L/min) for the remainder of the test. The total volume of water discharged from TW1 during the pumping test was approximately 17,955 L.

Rainfall of 11.6 mm was recorded on the day of the pumping test and resulted in a gradual increase in water level in both TW1 and OW1 over the duration of the test. Water level fluctuations in TW1 and OW1 mirrored each other, both in terms of timing and magnitude.

Following pump cessation, water levels were measured for 60 minutes. The water level recovered to greater than 100% of the initial water level in both TW1 and OW1 immediately upon termination of the pumping test.

Cambium notes that the pre-existing water supply well must be appropriately abandoned with consideration to Wells Regulation when it is no longer in use.

Water quality was satisfactory with hardness, TDS, Turbidity, Iron, and Sodium exceeding ODWQS criteria. Water quality is considered to be treatable with treatment options including a reverse osmosis unit that would reduce TDS and sodium concentrations if required. Hardness and Iron can be treated using water softener and/or a salt-free water conditioner (i.e. potassium chloride) to prevent scale build-up and reduce hardness. Dedicated iron filters are available in the event the softener is not sufficiently reducing iron. A water treatment specialist should be consulted to confirm appropriate treatment options.

Water Balance

It is projected that impervious post-development area (roof and pavement) will increase by approximately 2,289 m^2 when compared to pre-development conditions, which will create a net

deficit in infiltration to groundwater of approximately 409 m^3/year if no mitigation measures are implanted.

To regain the lost volume of water infiltrated, a diversion of approximately 38% of roof run-off would be required to maintain pre-development water balance conditions (assuming 100% of diverted water is infiltrated).

Implementation of Low Impact Development measures would enhance the Site's ability to infiltrate diverted roof run-off water into pervious areas. The infiltration deficit will be maintained to pre-development volumes through the use of infiltration features. Please refer to the D.B Grey Engineering documents (submitted under separate cover) for stormwater management information.

Conceptual Wastewater Design

A daily sewage design flow volume of 1,800 L/day was calculated for the proposed light industrial building. Given the site lithology and estimated T-times, a total septic bed footprint of approximately 21.6 m by 8.5 m, with a 6,000 L septic tank and a Waterloo Biofilter advanced treatment system, will be required. The bed will be at least partially raised due to Site conditions, with the specific height to be determined during the final building design.

The predictive assessment indicates the proposed development will result in an estimated nitrate concentration of 5.99 mg/L at lot boundaries if wastewater is treated via the proposed Waterloo Biofilter advanced treatment system and only dilution water from infiltration within permeable areas is considered.

Overall, the Site conditions appear feasible to install an on-site sewage system, and there is adequate space for the installation which appears to meet the required OBC setback distances. However, this should be considered and evaluated during the detailed sewage system design stage.

It is noted that the existing septic system at the Site must be appropriately decommissioned in line with guidelines provided by the Ottawa Septic System Office.

10.0 Closing

We trust that the information in this submission meets your current requirements. If you have any questions regarding the contents of this report, please contact the undersigned.

Respectfully submitted,

Cambium Inc.

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Coordinator, Hydrogeologist

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Jeremy Tracey, P.Eng.
Project Manager

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J. P. TRACEY
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2026-01-14

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11.0 References

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12.0 Standard Limitations

Limited Warranty

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Reliance on Materials and Information

The findings and results presented in reports prepared by Cambium are based on the materials and information provided by the client to Cambium and on the facts, conditions and circumstances encountered by Cambium during the performance of the work requested by the client. In formulating its findings and results into a report, Cambium assumes that the information and materials provided by the client or obtained by Cambium from the client or otherwise are factual, accurate and represent a true depiction of the circumstances that exist. Cambium relies on its client to inform Cambium if there are changes to any such information and materials. Cambium does not review, analyze or attempt to verify the accuracy or completeness of the information or materials provided, or circumstances encountered, other than in accordance with applicable accepted industry practice. Cambium will not be responsible for matters arising from incomplete, incorrect or misleading information or from facts or circumstances that are not fully disclosed to or that are concealed from Cambium during the provision of services, work or reports.

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When preparing reports, Cambium considers applicable legislation, regulations, governmental guidelines and policies to the extent they are within its knowledge, but Cambium is not qualified to advise with respect to legal matters. The presentation of information regarding applicable legislation, regulations, governmental guidelines and policies is for information only and is not intended to and should not be interpreted as constituting a legal opinion concerning the work completed or conditions outlined in a report. All legal matters should be reviewed and considered by an appropriately qualified legal practitioner.

Site Assessments

A site assessment is created using data and information collected during the investigation of a site and based on conditions encountered at the time and particular locations at which fieldwork is conducted. The information, sample results and data collected represent the conditions only at the specific times at which and at those specific locations from which the information, samples and data were obtained and the information, sample results and data may vary at other locations and times. To the extent that Cambium's work or report considers any locations or times other than those from which information, sample results and data was specifically received, the work or report is based on a reasonable extrapolation from such information, sample results and data but the actual conditions encountered may vary from those extrapolations.

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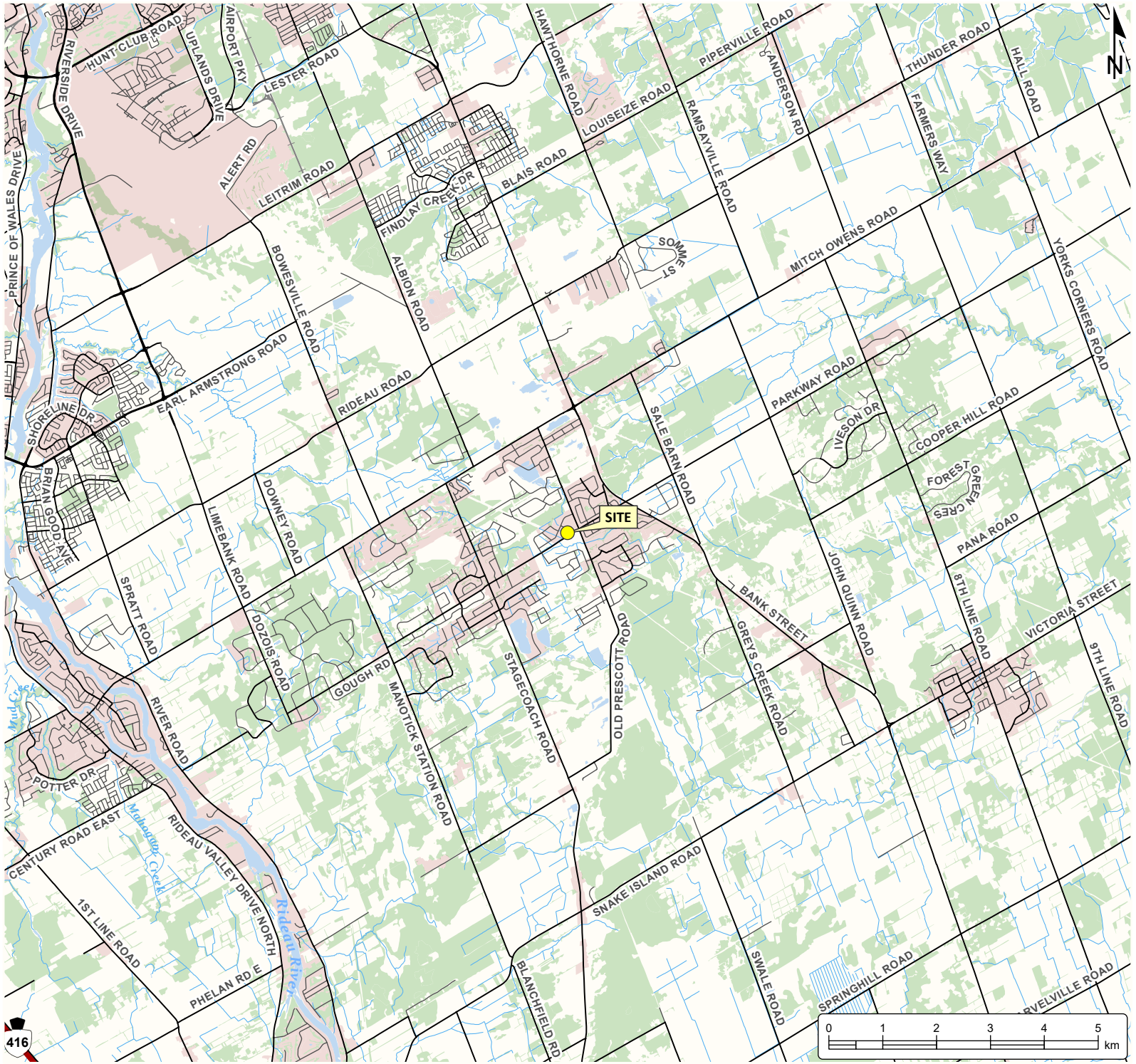
Appended Figures



HYDROGEOLOGICAL ASSESSMENT REPORT

CASSIDY EW CONSTRUCTION CONSULTANT LTD.
1386 and 1387 Greely Lane
Ottawa, Ontario

LEGEND

- Highway
- Major Road
- Minor Road
- Railway
- Watercourse
- Federal Protected Areas
- First Nations Reserve
- Water Area
- Provincial Park
- Wooded Area
- Built Up Area
- Airport

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SITE LOCATION PLAN

Project No.:	17281-002	Date:	January 2026
		Rev.:	
Scale:	1:100,000	Projection:	NAD 1983 UTM Zone 18N
Created by:	NLB	Checked by:	JT
Figure:	1		

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HYDROGEOLOGICAL ASSESSMENT REPORT

CASSIDY EW CONSTRUCTION CONSULTANT LTD.
1386 and 1387 Greely Lane
Ottawa, Ontario

LEGEND

- Test Well
- Observation Well
- Monitoring Well
- Borehole
- Benchmark
- Site (approximate)

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SITE PLAN WITH BOREHOLE LOCATIONS

Project No.: 17281-002		Date: January 2026 Rev.:	
Scale: 1:600		Projection: NAD 1983 UTM Zone 17N	
Created by: NLB	Checked by: JT	Figure:	2

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HYDROGEOLOGICAL ASSESSMENT REPORT

CASSIDY EW CONSTRUCTION CONSULTANT LTD.
1386 and 1387 Greely Lane
Ottawa, Ontario

LEGEND

- (98.29) Groundwater Elevations April 2024
- Benchmark
- Borehole
- Monitoring Well
- Test Well
- Observation Well
- Groundwater Contour April 2024
- Site (approximate)
- Groundwater Flow Direction April 2024

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GROUNDWATER CONFIGURATION PLAN

Project No.: 17281-002		Date: January 2026 Rev.:	
Scale: 1:600		Projection: NAD 1983 UTM Zone 18N	
Created by: LD	Checked by: JT	Figure:	3

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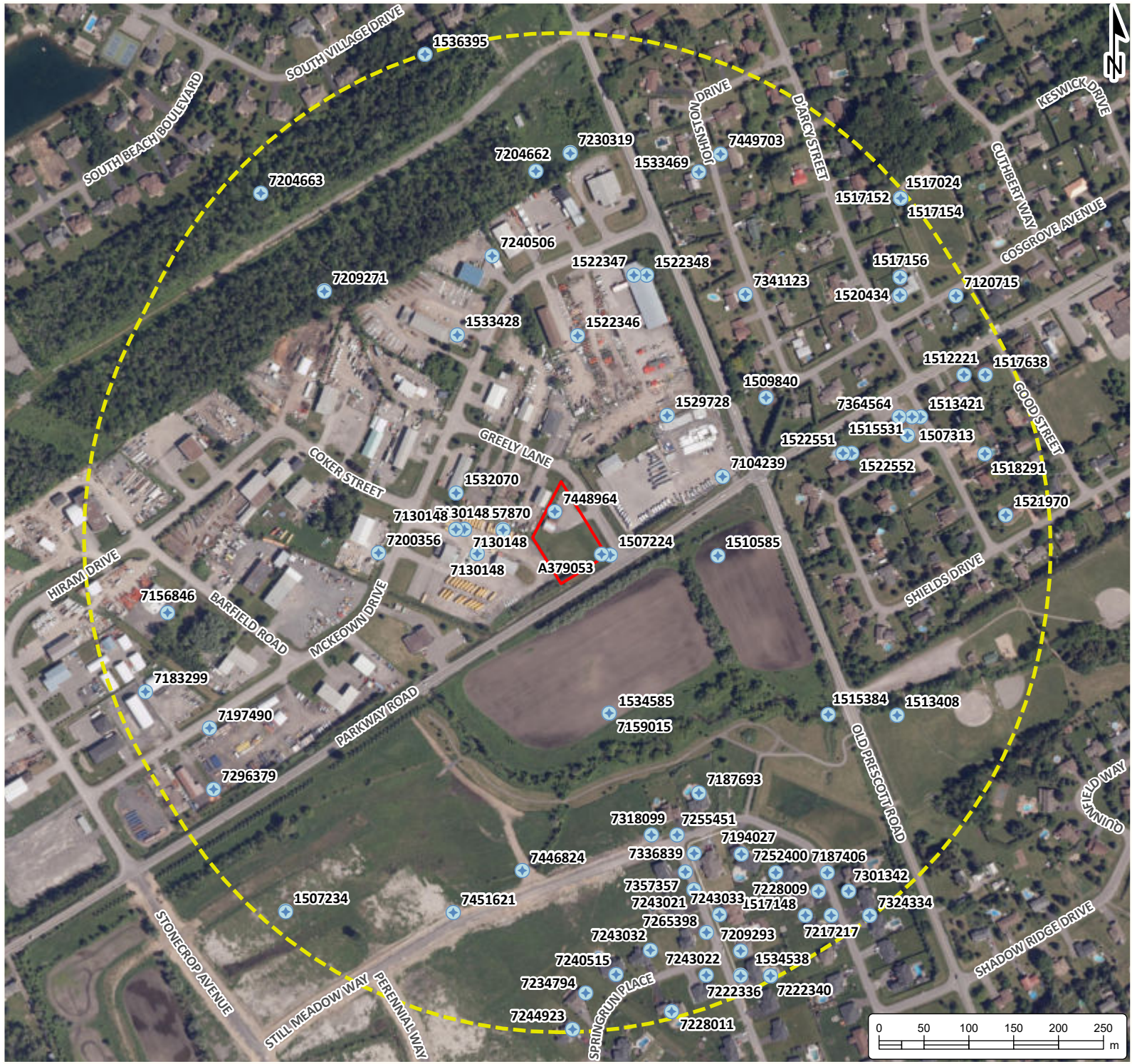

HYDROGEOLOGICAL ASSESSMENT REPORT

CASSIDY EW CONSTRUCTION CONSULTANT LTD.

1386 and 1387 Greely Lane
Ottawa, Ontario

LEGEND

- Water Well Record
- 500m Study Area
- Site (approximate)
- Roads - Labels Only
- Railway - Labels Only
- Water Area - Labels Only
- Watercourse - Lables Only

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MECP WELL RECORDS WITHIN 500m

Project No.: 17281-002		Date: January 2026	Rev.:
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Pre-Development	Area (m^2)
Roofed	353
Paved	855
Landscaped	3472
Total	4680

HYDROGEOLOGICAL ASSESSMENT REPORT

CASSIDY EW CONSTRUCTION CONSULTANT LTD.
1386 and 1387 Greely Lane
Ottawa, Ontario

LEGEND

- Roofed
- Paved
- Landscaped
- Site (approximate)
- Roads - Labels Only
- Railway - Labels Only
- Water Area - Labels Only
- Watercourse - Lables Only

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PRE-DEVELOPMENT PLAN

Project No.: 17281-002		Date: January 2026 Rev.:	
Scale: 1:600		Projection: NAD 1983 UTM Zone 18N	
Created by: NLB	Checked by: JT	Figure:	5

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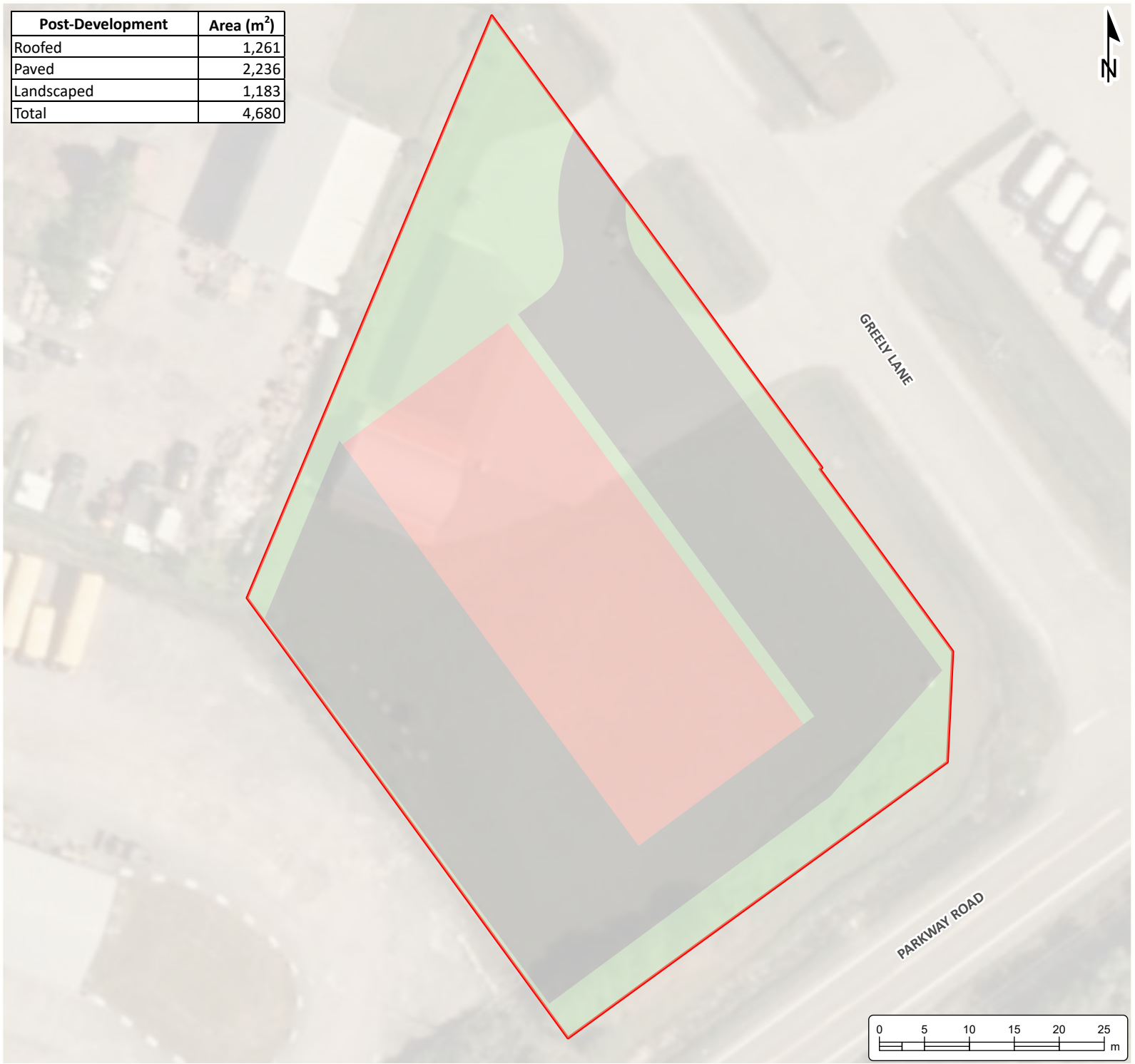
Post-Development	Area (m^2)
Roofed	1,261
Paved	2,236
Landscaped	1,183
Total	4,680



HYDROGEOLOGICAL ASSESSMENT REPORT

CASSIDY EW CONSTRUCTION CONSULTANT LTD.
1386 and 1387 Greely Lane
Ottawa, Ontario

LEGEND

- Roofed
- Paved
- Landscaped
- Site (approximate)

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POST-DEVELOPMENT PLAN

Project No.: 17281-002		Date: January 2026	Rev.:
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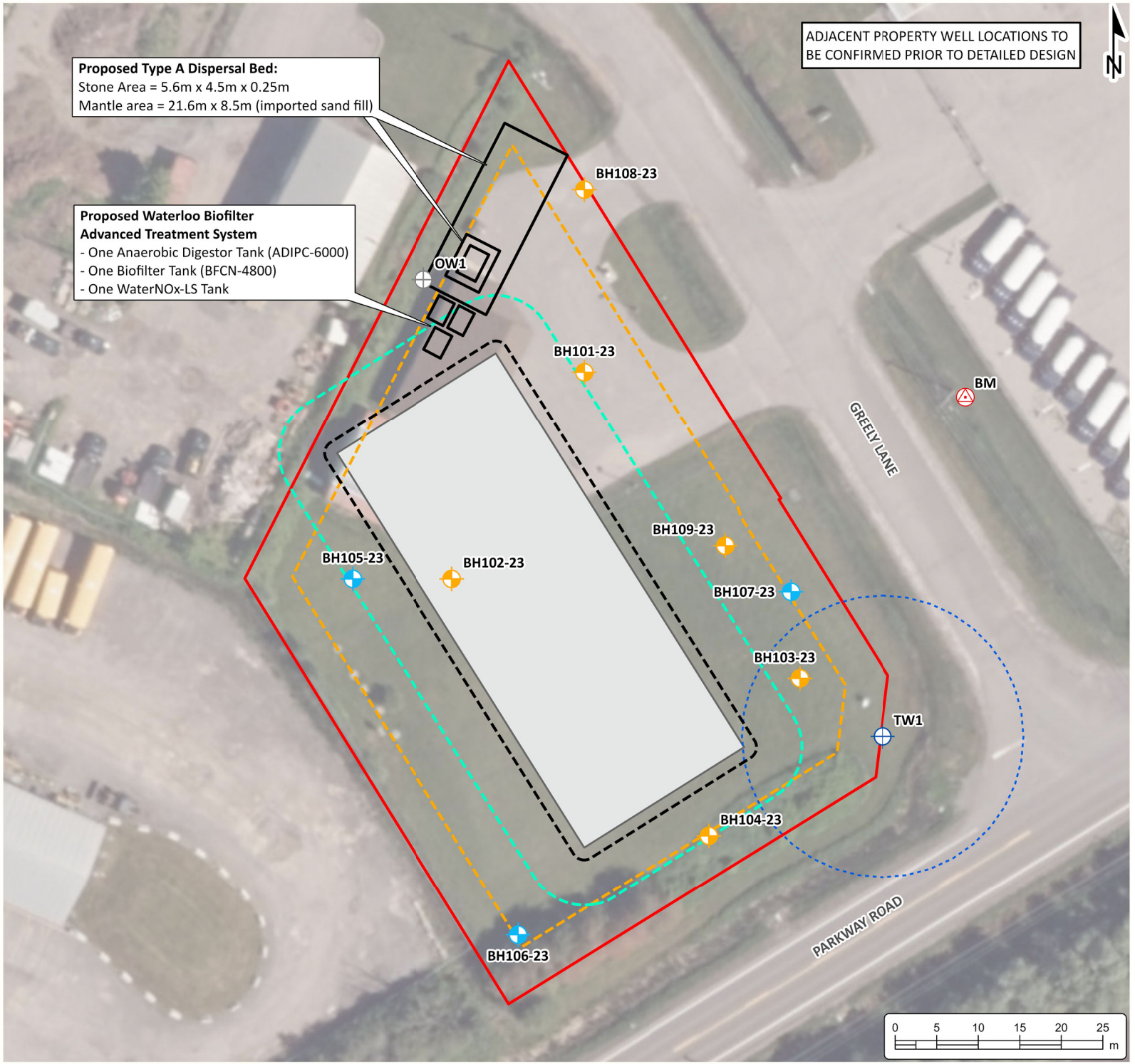

HYDROGEOLOGICAL ASSESSMENT REPORT

CASSIDY EW CONSTRUCTION CONSULTANT LTD.
1386 and 1387 Greely Lane
Ottawa, Ontario

LEGEND

- Benchmark
- Borehole
- Monitoring Well
- Observation Well (To be Decomissioned)
- Test Well
- 17m Well Buffer
- 5m Property Line Setback
- 1.5m Building Setback
- 7m Building Setback
- Proposed Building
- Proposed Septic
- Site (approximate)

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CONCEPTUAL SEWAGE SYSTEM DESIGN

Project No.: 17281-002		Date: January 2026	Rev.:
Scale: 1:650		Projection: NAD 1983 UTM Zone 18N	
Created by: NLB	Checked by: JT	Figure: 7	

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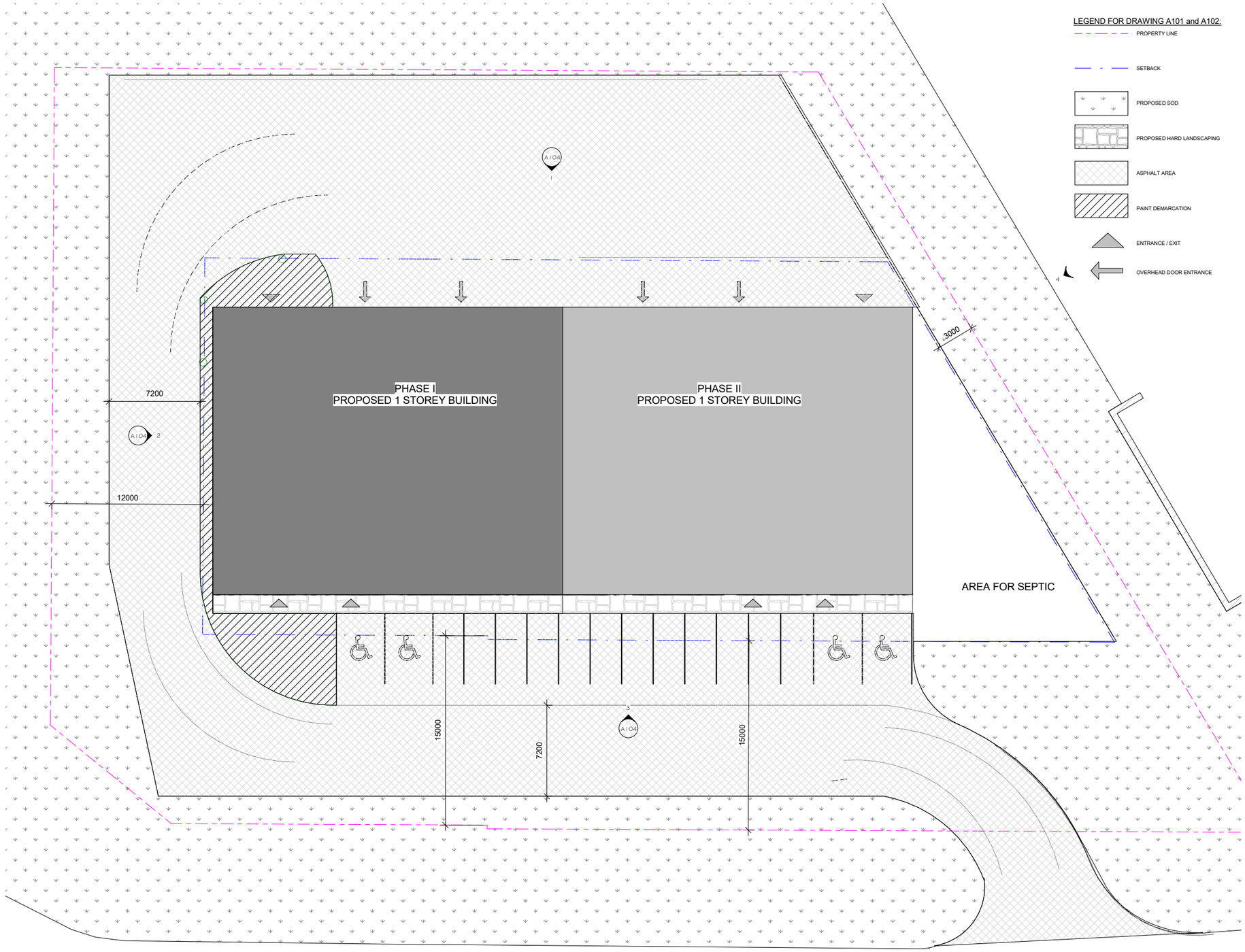
Appendix A

Property and Land Information



Proposed Site Plan- Phase 2

CONSTRUCTION NORTH

TRUE NORTH

No (N°)	ISSUE/REVISION (ÉMISSION/RÉVISION)	DATE (DD/MM/YY)

BRYDEN GIBSON
ARCHITECTS INCORPORATED
1066 Somerset Street West, Suite 200, Ottawa Ontario, K1Y 4T3
Telephone: 613.724.9914 E-mail: architecture@brydengibson.ca

PROJECT NAME / NOM DU PROJET
1386-1394 GREELY LANE

1386 - 1394 Greely Ln, Greely, ON K4P 1A1

DRAWING TITLE / TITRE DU DESSIN
Proposed Site Plan - Phase 2

JOB No / N° DE PROJET	DATE / DATE
768-23	22.12.2023

SCALE / ECHELLE: As indicated

PRINTING SCALE/ ÉCHELLE D'IMPRESSION

IF THIS BAR IS NOT 25 mm LONG. ADJUST YOUR PRINTING SCALE.

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CONCEPTION BY / CONÇUS PAR: SG

DRAWN BY / DESSINÉ PAR: TD

CHECKED BY / VÉRIFIÉ PAR: SG

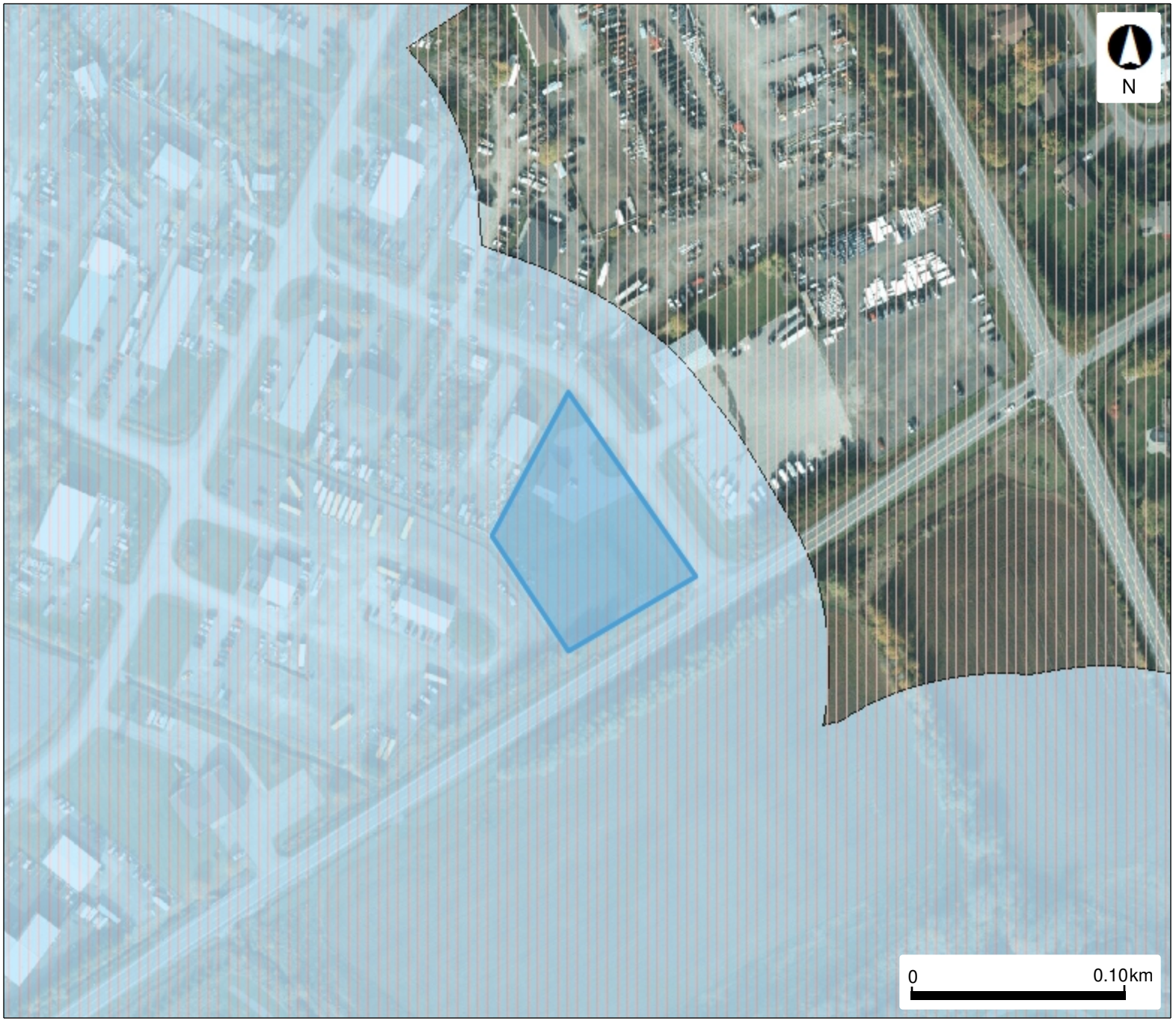
ARCHITECT'S STAMP

SCEAU D'ARCHITECTE

DRAWING No / DESSIN N°: A102

REVISION No / REVISION N°

Source Protection Information Atlas Map



Legend

- Highly Vulnerable Aquifers
- Intake Protection Zone 3

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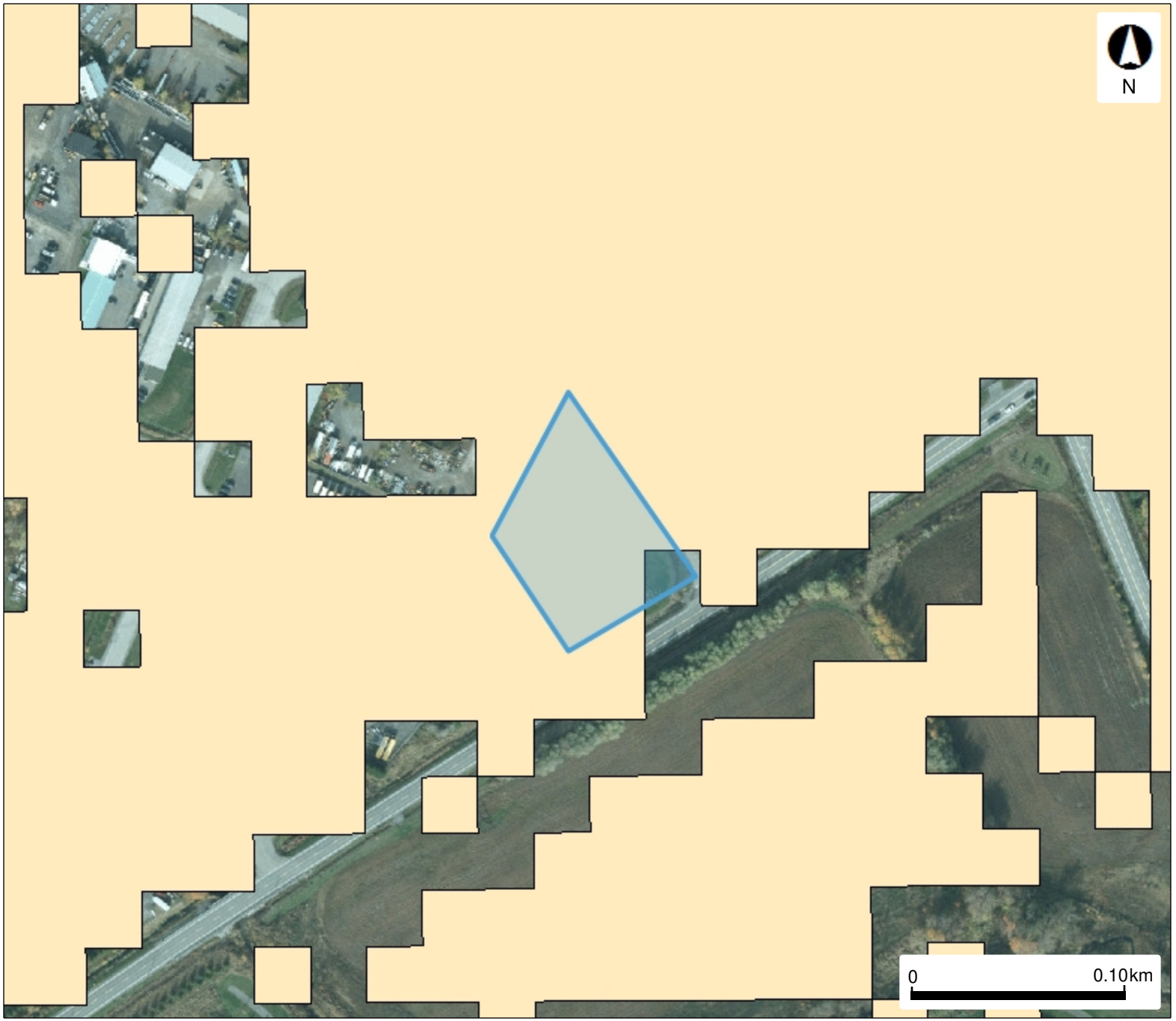
Ontario

Map Created: 5/27/2024
Map Center: 45.25878 N, -75.57137 W

Source Protection Information Atlas - SGRA Map



Legend

Significant Groundwater Recharge Area

- N/A
- 0
- 2
- 4
- 6

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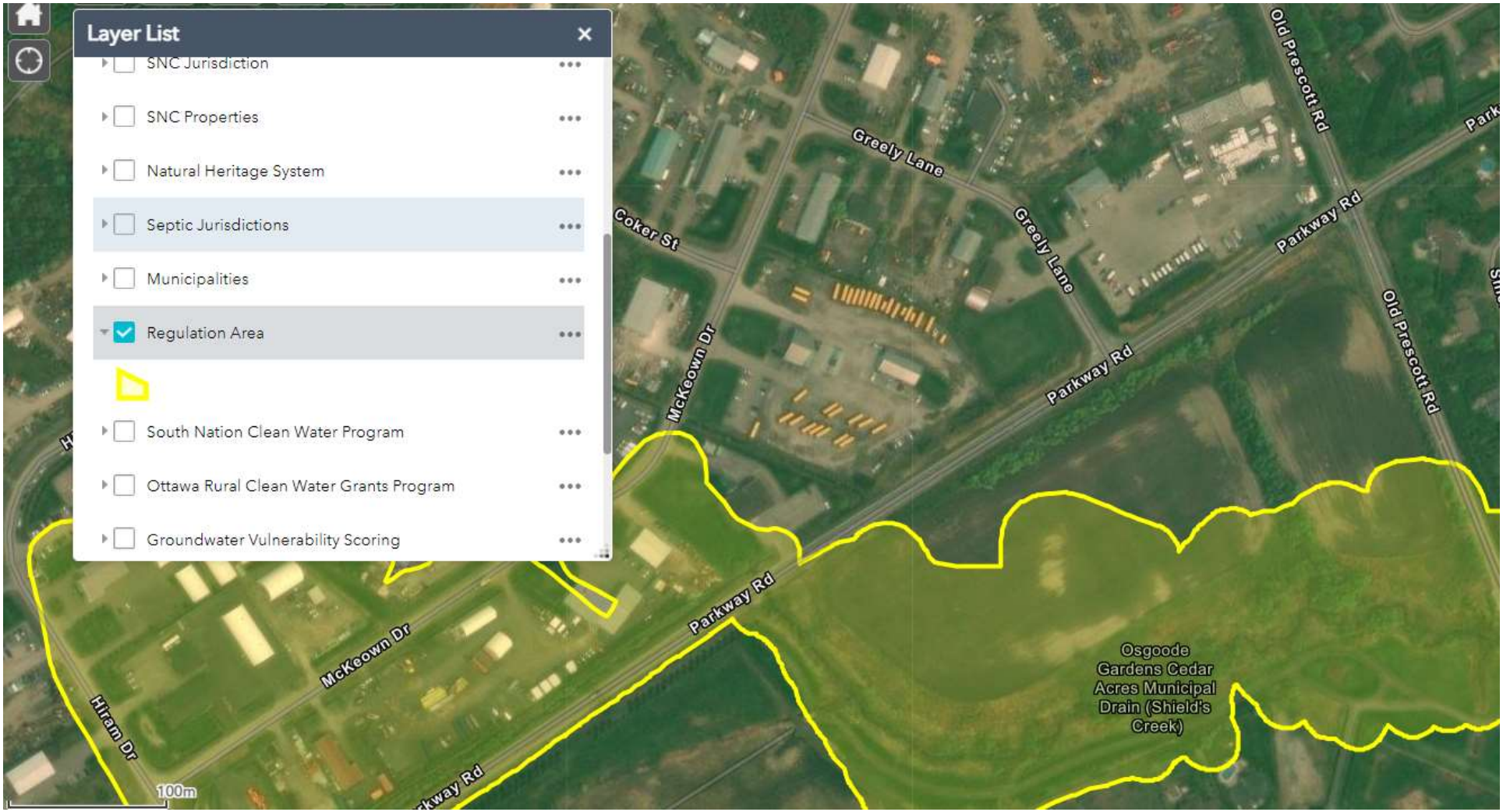
Ontario

Map Created: 5/27/2024
Map Center: 45.25878 N, -75.57137 W



File Number: D07-12-25-0004

December 17, 2025

Chris Poirier
Dandax Developments Inc.
Via email: chris@cassidyewconstruction.com

Subject: Site Plan Control – 1386-1394 Greely Lane – Formal Review Comments

Please find below the consolidated comments from the formal review of the above noted application.

Planning

List of Studies and Plans Reviewed:

- **Site Plan,** A101, prepared by Bryden Gibson Architects Incorporated, dated January 10, 2025, revision 3 dated October 9, 2025.
- **Site Plan Details,** A103, prepared by Bryden Gibson Architects Incorporated, dated March 2025, revision 3 dated October 9, 2025.
- **Elevations,** A102, prepared by Bryden Gibson Architects Incorporated, dated January 10, 2025, revision 3 dated October 9, 2025.

Comments:

1. Thank you for placing the File Number **D07-12-25-0004** and Plan Number **#19344** on the plans; however, on the Site Plan, Site Plan Details and Elevations the file number and plan number are in the incorrect spot. Please reference the image below (and also the civil plan and landscape plans for this application) on the placement of these numbers:

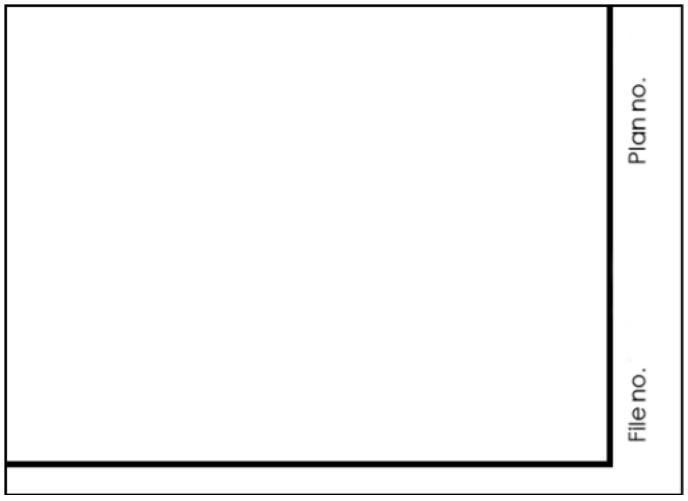


Figure 1: Example of how the file application number and plan number should be added to all plans for this application .

2. The details provided for the accessible parking sign and the fire route sign on the Site Plan on the details plan is inadequate. Please see below example on what is expected for the two sign details.

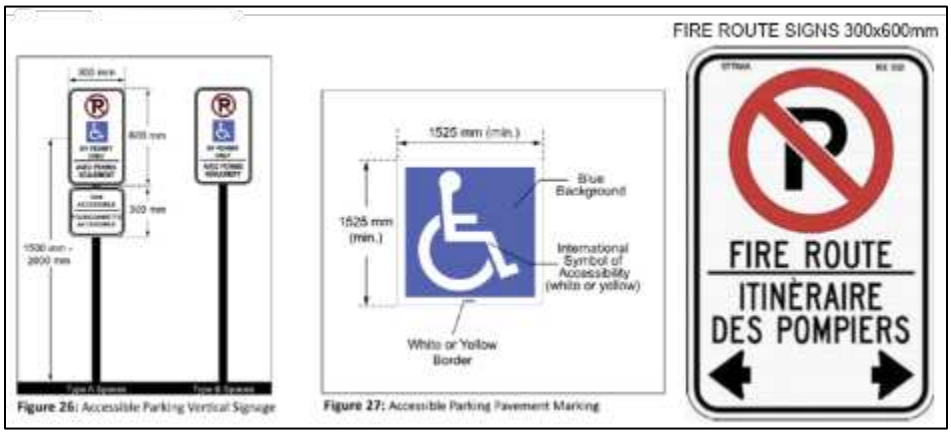


Figure 2: Example of site plan sign details for the accessible parking and fire route signs.

Please contact Jerrica Gilbert, Planner II - Rural, for follow-up questions.

Engineering

List of Studies and Plans Reviewed:

- **Site Servicing Plan,** drawing no.: C-1 of 9; prepared by D.B. Gray Engineering Inc., dated September 20, 2024,, rev no. 10, dated November 20, 2025.
- **Grading Plan,** drawing no.: C-4 of 9 prepared by: D.B. Gray Engineering Inc., dated September 20, 2024, revision 8, dated November 20, 2025.
- **Erosion & Sediment Control Plan,** drawing no.: C-5 of 9; prepared by D.B. Gray Engineering Inc., dated September 20, 2024, rev no. 8, dated November 20, 205.
- **Post-Development Drainage Plan**, drawing no.: C-6 of 9, prepared by D. B. Gray Engineering Inc., dated September 20, 2024, revision 7, dated November 20, 2025
- **Pre-Development Drainage Plan**, drawing no.: C-7 of 9, prepared by D. B. Gray Engineering Inc., dated September 20, 2024, revision 4, dated September 29, 2025
- **Details & Schedule,** drawing no.: C-8 of 9, prepared by D.B. Gray Engineering Inc., dated September 20, 2024,, revision 7, dated November 20, 2025.
- **Notes & Details,** drawing no.: C-9 of 9, prepared by: D.B. Gray Engineering Inc., dated September 20, 2024,, revision 6, dated September 29, 2025.
- **Proposed Grande Retained Wall**, drawing no.: Figure 1, prepared by Paterson Group, dated May, 2025
- **Geotechnical Investigation Report,** file reference: 17281-001, prepared by Cambium Inc., dated February 27, 2025.

- **Site Servicing Study & Stormwater Management Report,** report no.: 24015, prepared by D.B. Gray Engineering Inc., dated January 13, 2025, revised November 20, 2025.
- **Phase 1 Environmental Site Assessment,** document no.: ER1015, prepared by CM3 Environmental, dated January 24, 2023.
- **Phase Two Environmental Site Assessment,** file reference: 17281-001, prepared by Cambium Inc., dated April 24, 2023.

Comments:

3. The Phase I ESA report has stale-dated and needs an update.

4. The Phase II ESA report has stale-dated and needs an update.

Please contact Damien Whittaker, Infrastructure Project Manager, for follow-up questions.

Hydrogeology

List of Studies and Plans Reviewed:

- **Hydrogeological Assessment Report 1386 & 1394 Greely Land**, Ref. No 17281-002, prepared by Cambium Inc., dated November 11, 2024, revision dated July 25, 2025.
- **Site Servicing Study & Stormwater Management Report**, prepared by D.B. Gray Engineering Ltd., dated January 13, 2025, revision dated November 20, 2025.
- **Septic System**, C-2, prepared by D.B. Gray Engineering Ltd., dated September 20, 2024, revision 7 dated September 28, 2025.
- **Grading Plan**, C-4, prepared by D.B. Gray Engineering Ltd., dated September 20, 2024, revision 8 dated November 20, 2025.

Comments:

5. The responses in *Response to City of Ottawa Review Questions Re. Hydrogeological Assessment Report – 1386 & 1394 Greely Lane, Ottawa, Ontario (Cambium Reference: 17281-002)*, dated November 25, 2025 must be addressed by making the corresponding changes to the formal report. The City requires that the final approved hydrogeological report include all final recommendations and assessments/rationale made to justify the planning decision.

a. In section 6.4, clarify the equipment manufacturer, model number, and calibration records for the field equipment used per pdf page 59 of the City's Hydrogeological and Terrain Analysis Guidelines.

b. In section 6.4, please provide a discussion regarding any encrustation or taste issues as a result of the TDS exceeding 500mg/L and whether there are any

potential treatment systems to remedy. LSI & RSI calculations should support the discussion of scaling and corrosion. Taste issues should be reviewed based on the concentrations of individual parameters and whether they are elevated (chlorides, sulphates, calcium, magnesium, potassium, sodium, bicarbonates, and organic matter).

c. In section 6.4, please discuss a proposed treatment system for the exceedance of hardness above the operational guidance. Hardness above 300 mg/L is considered 'very hard' per MECP D-5-5 and a treatment system should be discussed.

d. In section 6.4, please discuss the recommended treatment system for iron which exceeded the aesthetic objective of 0.3 mg/L in the second sample.

e. Section 7.0 – Water Balance Assessment is not agreed to as the recommended infiltration measures have not been implemented in the design of the site. The reporting should be clear of the purpose of the water balance calculations and update how they inform with or correspond with the design of infiltration features on the site.

f. In Section 8.0 and Appendix I, the Predictive Assessment for Nitrate Concentrations considers that the property will rely on 240 m3/year of infiltration by other means (diverting roof runoff proposed) in addition to the available infiltration volume from pervious areas of 277.9 m3/year, based on the post-development scenario. Revision should be made to clarify if this still applies or can be removed.

g. In section 9.0, please discuss the recommended treatment systems discussed in section 6.4 (sodium above warning, hardness, iron, and as required, TDS).

h. In section 9.0 – *Conclusions and Recommendations*, in the Water Balance header, the reporting notes that there will be a net deficit of 555 m3/year but does not present or reference how that infiltration design will be implemented or whether it is feasible. Maintaining the water balance should be a joint effort between hydrogeological and stormwater management consultants. A design must be provided, indicating how the infiltration component required to meet nitrate dilution requirements are met and will be met in the long term.

i. In Appendix G, pdf page 129, turbidity is reported as 7070 NTU. Please confirm if that is the correct value.

Please contact Travis Smith, Senior Project Manager, Hydrogeology, for follow-up questions.

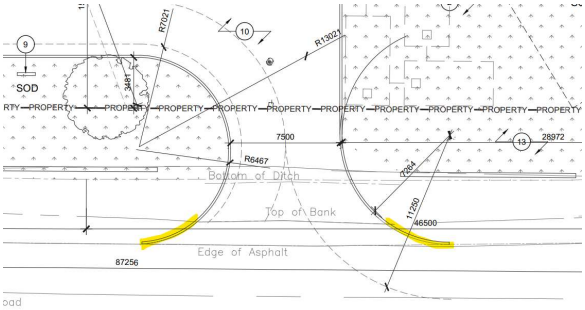
Transportation

List of Studies and Plans Reviewed:

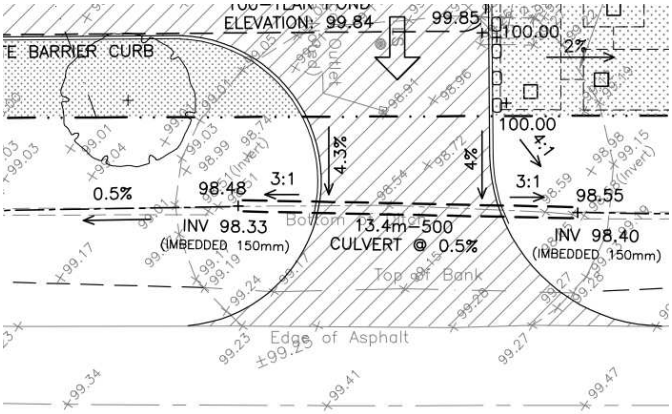
- **Site Plan,** A101, prepared by Bryden Gibson Architects Incorporated, dated April 2025, revision 3 dated September 10, 2025.

Comments:

6. The Site Plan is now showing a full-height curb across the shoulder, which is not supported. Ensure the section over the shoulder is either a depressed curb or terminate the curb prior to connecting with the roadway and change the linework to show edge of pavement.



Note this is adequately shown on the Grading Plan:



Please contact Josiane Gervais, Transportation Project Manager, for follow-up questions.

South Nation Conservation Authority

7. SNC's previous comments have been addressed with the exception of comment # 29 of the D.B. Gray Engineering Inc. Comment-Response Letter, dated November 20, 2025.

   The original comment notified the applicant that according to the Stormwater Management Planning and Design Manual (2003), "The outlet invert elevation

from any SWMP should be higher than the 2-year flood line, and the overflow elevation must be above the 25-year flood line." Since the SWM pond is discharging into the roadside ditch, the applicant was to confirm that the outlet complies with these guidelines.

The applicant's response was that the 1:100-year floodplain does not affect the subject property. Therefore, the outlet is above the 2 and 25-year flood line, which complies with these guidelines.

The SNC responds that the outlet elevation should be above the 2-year flood elevation in the Parkway Roadside Ditch. Confirmation needs to be provided by the applicant that the 2-year flood elevations of the Parkway Roadside ditch will not affect the function of the SWM system.

8. Please see attached correspondence for the full response from the South Nation Conservation Authority.

Contact: James Holland, jholland@nation.on.ca, 613-984-2948

Next Submission

- ☐ The next submission should address each of the comments, to ensure the effectiveness and consistency of the next review.
- ☐ A cover letter must be included that states how each comment was addressed in the resubmission. Please co-ordinate the numbering of each resubmission comment with the above noted comment number.
- ☐ Plans are to be standard A1 size (594 mm x 841 mm) or Arch D size (609.6 mm x 914.4 mm) sheets, utilizing an appropriate Metric scale (1:200, 1:250, 1:300, 1:400 or 1:500).
- ☐ All addenda or revisions to any studies or plans must be provided in PDF. All PDF documents are to be unlocked, flattened and not saved as a portfolio file.

Should there be any questions on the above, please do not hesitate to contact myself or the contact identified for the above areas / disciplines.

Sincerely,

Jerrica Gilbert
Planner II, Development Review – Rural

c.c. Damien Whittaker, Infrastructure Project Manager
Travis Smith, Senior Project Manager, Hydrogeology
Josiane Gervais, Transportation Project Manager
Mark Elliott, Environmental Planner
Hayley Murray, Forester
Warren Bedford, Parks Planner
Stephan Kukkonen, Planner I

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November 25, 2025

1-1011 Thomas Spratt Place,
Ottawa, Ontario K1G 5L5

Attn: Chris Poirier

Re: Response to City of Ottawa Review Questions Re. Hydrogeological Assessment Report – 1386 & 1394 Greely Lane, Ottawa, Ontario
Cambium Reference: 17281-002

Dear Chris Poirier,

Cambium Inc. (Cambium) prepared a Hydrogeological Assessment (report) dated July 25, 2025, to support the proposed development at 1386 & 1394 Greely Lane, Ottawa, Ontario (Site). The City of Ottawa review comments sent via email on August 20, 2025, pertaining to the report are summarized below with Cambium's responses to each in italics.

Review Comments

4. In Section 6.4, clarify the equipment manufacturer, model number and calibration records for the field equipment used per pdf page 59 of the City's Hydrogeological and Terrain Analysis Guidelines

General chemistry parameters (pH, temperature, conductivity, ORP, and DO) were measured using a YSI ProDSS Multiparameter Digital Water Quality Meter. Calibration is completed for each sensor once per week using calibration solutions purchased from Maxim Environmental. For calibration, the YSI probe is placed in a calibration solution and the calibration is run for the specific parameter. This is repeated for each sensor/parameter.

Turbidity was measured using a HACH 2100Q #50166. Calibration is completed once per week using calibration samples purchased from Maxim Environmental. For calibration, the meter will prompt for one standard and a time. The meter will store the readings and adjust calibration based on the standards. Calibration verifications are completed once per use prior to testing.

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November 25, 2025

Chlorine residual was measured using a Hanna HI701-11 Free Chlorine Checker HC. Calibration verifications are completed once per use prior to testing using Hanna HI701-11 certified standard cuvettes. The unit is calibrated annually by the manufacturer.

5. In Section 6.4, please provide a discussion regarding any encrustation or taste issues as a result of the TDS exceeding 500 mg/L and whether there are any potential treatment systems to remedy. LSI & RSI calculations should support the discussion of scaling and corrosion. Taste issues should be reviewed based on the concentrations of individual parameters and whether they are elevated (chlorides, sulphates, calcium, magnesium, potassium, sodium, bicarbonates, and organic matter)

A high TDS value can indicate some issues with scale buildup as well as aesthetic issues in taste and appearance. Common reported aesthetic issues include a salty or bitter taste as well as a hazy or cloudy appearance. It should be noted that both water quality samples exceeded the OWDQS aesthetic objective of 500 mg/L for TDS by a minimal amount, 510 mg/L for TW1-1 and 522 mg/L for TW1-2. A reverse osmosis unit would reduce TDS concentration if required

Langelier Saturation Index (LSI) and Ryznar Stability Saturation Index (RSI) was not included in the lab analysis suite for the secondary sample collected from TW-1. Considering the water quality results for temperature, calcium concentration, alkalinity, pH, TDS preliminary results for an LSI value would be +0.35, indicating slight scaling, and an RSI value of 7.25 indicating slight corrosion potential. The water would be considered balanced and convention water softening and reverse osmosis treatment can be utilized as needed to address any operational or aesthetic issues as needed.

Alongside the high TDS value, concentrations of chloride, sulphate, calcium, manganese, potassium, and bicarbonates were detected, however not elevated above the ODWQS criteria. Sodium was reported above the maximum acceptable concentration, but not above the aesthetic objective. Elevated sodium presents a risk for those who are on restricted sodium diets. Sodium, along with

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the other parameters discussed above, can be treated with the use of reverse osmosis systems.

A water treatment specialist should be consulted to confirm appropriate treatment options.

6. In Section 6.4, please discuss a proposed treatment system for the exceedance of hardness above the operational guideline. Hardness above 300 mg/L is considered 'very hard' per MECP D-5-5 and a treatment system should be discussed

Hardness is typically treated through the use of a water softener. A typical water softener uses salt to reduce calcium and magnesium. As sodium is already elevated in the drinking water, a salt-free water conditioner is recommended (i.e. potassium chloride) to prevent scale build-up and reduce hardness.

7. In Section 6.4, please discuss the recommended treatment system for iron which exceeded the aesthetic objective of 0.3 mg/L in the second sample.

At the concentration reported, iron can be treated with the use of a water softener. Dedicated iron water filters are available in the event the softener is not sufficiently reducing iron. Considering the aesthetic objective for iron is 0.3 mg/L and the two samples collected had concentrations of 0.205 mg/L for TW1-1 and 0.326 mg/L for TW1-2, a water softener would be expected to reduce total iron below 0.3 mg/L.

8. Section 7.0 – Water Balance Assessment is not agreed to as the recommended infiltration features have not been implemented in the design of the Site.

Please refer to the D.B Grey Engineering documents (submitted under separate cover) for detailed design of infiltration features.

9. Section 7.0 – Water Balance Assessment must be clear on how the water budget target implemented on the Site will address the requirements of the applicable subwatershed plan. The applicable higher-level study, *Shield's Creek Subwatershed Study,* June 2004, notes in Section 6.3.4.7 – Infiltration/Groundwater Protection that 'existing infiltration levels are to be

maintained as part of a stormwater management plan for future development to protect the groundwater resources and maintain current hydrologic functions for flow regime protection, erosion control, and low flow maintenance (fisheries).

The pre- and post-development water balance was updated and enclosed in the document to be consistent with D.B Grey Engineering water balance calculations. The results of the water balance indicate that there will be an infiltration deficit of 409 m^3/year upon development of the Site, which will be maintained to pre-development volumes through the use of infiltration features. Please refer to the D.B Grey Engineering documents (submitted under separate cover) for stormwater management information.

10. In Section 8.0 and Appendix I, the Predictive Assessment for Nitrate Concentrations considers that the property will rely on 240 m^3/year of infiltration by other means (diverting roof runoff proposed) in addition to the available infiltration volume from pervious areas of 277.9 m^3/year, based on the post development scenario

    A. Provide supporting rationale and design for the system or systems that contribute to the “minimum infiltration amount” (240 m^3/yr) assumed in the Nitrate Attenuation calculations. Roof runoff infiltration appeared to be assumed with no justification, currently shown on the *Site Servicing Plan* as downspouts discharging to the surface and into SWM measures. Supporting rationale and data must be provided to support that the “minimum infiltration volume” can be achieved given the groundwater table elevations observed on the property.

The minimum infiltration amount (240 m^3/year) was a carry forward error from Cambium’s March 21, 2025 hydrogeological assessment report which used a ECOFLO STB-650B advanced treatment system. In Cambium’s July 25, 2025 hydrogeological assessment report, the sewage system was updated to a Waterloo Biofilter advanced treatment system with nitrate treatment capable of achieving 6.0 mg/L of nitrate at the property boundary with no added infiltration amount. The updated nitrate attenuation calculations are enclosed.

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B. In Appendix I, clarify how the Q_i of 891.34 was calculated. It appears that based on available infiltration area (1171 m^2) and potential infiltration (0.397 m/yr x 0.7 = 0.2779 m/yr).

As is demonstrated in the updated nitrate attenuation calculations (attached), the value of Q_i was calculated as follows:

$$Q_i\left(\frac{L}{day}\right) = Surplus\left(\frac{m}{day}\right) * Infiltration\ Factor * Permable\ Area\ (m^2) * 1000\frac{L}{m^3}$$

$$Q_i\left(\frac{L}{day}\right) = \frac{0.00108767m}{day} * 0.45 * 1{,}183\ m^2 * 1000\frac{L}{m^3}$$

$$Q_i = 579.02\frac{L}{day}$$

C. The *Site Servicing Study & Stormwater Management Report* does not quantify how the “minimum infiltration rate” (240 m^3/yr) would be provided.

Please refer to the response for comment 12 A above.

11. In Section 9.0, please discuss the recommended treatment systems discussed in Section 6.4 (sodium above warning, hardness, iron, and as required, TDS).

Please refer to responses to comments 7, 8, and 9 above.

12. In Section 9.0 – *Conclusion and Recommendations,* in the Water Balance header, the reporting notes that there will be a net deficit of 555 m^3/year but does not present or reference how that infiltration design will be implemented or whether it is feasible. Maintaining the water balance should be a joint effort between hydrogeological and stormwater management consultants. A design must be provided, indicating how the infiltration component required to meet nitrate dilution requirements are met and will be met in the long term.

Updated water balance by Cambium calculates a net infiltration deficit of 409 m^3/year. Please refer to the D.B Grey Engineering documents for stormwater

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management details and infiltration feature design (provided under separate cover).

13. In Appendix G, pdf page 129, turbidity is reported as 7070 NTU. Please confirm if that is the correct value.

The turbidity reported as 7070 NTU is not an error in the report. Turbidity reported for groundwater samples collected from monitoring wells are often elevated due to the sample collection method. The monitoring wells are developed by purging the wells of 3 well volumes of water or purged dry 3 times using dedicated polyethylene tubing (Waterra™) installed with an inertial foot valve. The well development will disturb suspended sediment in the well and elevate turbidity. The TSS concentration is also very high (9,480 mg/L), attributing to the increase in turbidity.

14. For future resubmissions, please provide a hydrogeological response to City comments.

This letter is provided as a response to the City of Ottawa's comments.

15. Note: For future laboratory water quality results, please report trace metals in mg/L (as Ontario Water Quality Standards are expressed in mg/L), though this appears to be only applicable to 2024 laboratory samples.

No further water sampling is required, but requirements are noted for any further work that may occur for the future.

16. Note: For future water quality tests, the field parameters should be expanded to include colour and hydrogen sulphide (using the methylene blue method) per pdf page 59 of the City's Hydrogeological and Terrain Analysis Guidelines. For these results, laboratory values confirm that true colour and hydrogen sulphide were low or non-detect, so no additional testing for these parameters is suggested. Based on field notes, odour and colour were noted as "none" which supports the lab values.

No additional water quality is required, but requirements are noted for any further work that may occur for the future.

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November 25, 2025

CLOSING

Cambium trusts that this letter report will be suitable for your project. Should you have any questions or require further clarification with respect to this document, please do not hesitate to contact the undersigned at (705) 742-7900.

Best regards,

Cambium Inc.

DocuSigned by:
6C8CA15FD6B4444...

Warren Young, P.Eng.,
Project Coordinator - Hydrogeologist

DocuSigned by:
5230E648B0C64BD...

Stew Dolstra, Honours, B. Sc., Dipl. BCIN
Well Technician - Senior Project Manager

DocuSigned by:
C9F8935E96D14CC...

Jeremy Tracey, P.Eng.,
Project Manager

Signed by:

LICENSED PROFESSIONAL ENGINEER
J. P. TRACEY
100226672
PROVINCE OF ONTARIO

2025-12-01

WDY/JPT/spd

Encl. *Cambium Qualifications & Limitations*
Water Balance Calculations
Nitrate Attenuation Calculations

\\cambiumincstorage.file.core.windows.net\projects\17200 to 17299\17281-002 Cassidy EW Construction Consultant Ltd - HydroG - Greely Lane\Correspondence\Letters\City of Ottawa Response\2025-11-25 LTR - City of Ottawa Response.docx

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November 25, 2025

CAMBIUM QUALIFICATIONS AND LIMITATIONS

Limited Warranty

In performing work on behalf of a client, Cambium relies on its client to provide instructions on the scope of its retainer and, on that basis, Cambium determines the precise nature of the work to be performed. Cambium undertakes all work in accordance with applicable accepted industry practices and standards. Unless required under local laws, other than as expressly stated herein, no other warranties or conditions, either expressed or implied, are made regarding the services, work or reports provided.

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The findings and results presented in reports prepared by Cambium are based on the materials and information provided by the client to Cambium and on the facts, conditions and circumstances encountered by Cambium during the performance of the work requested by the client. In formulating its findings and results into a report, Cambium assumes that the information and materials provided by the client or obtained by Cambium from the client or otherwise are factual, accurate and represent a true depiction of the circumstances that exist. Cambium relies on its client to inform Cambium if there are changes to any such information and materials. Cambium does not review, analyze or attempt to verify the accuracy or completeness of the information or materials provided, or circumstances encountered, other than in accordance with applicable accepted industry practice. Cambium will not be responsible for matters arising from incomplete, incorrect or misleading information or from facts or circumstances that are not fully disclosed to or that are concealed from Cambium during the provision of services, work or reports.

Facts, conditions, information and circumstances may vary with time and locations and Cambium's work is based on a review of such matters as they existed at the particular time and location indicated in its reports. No assurance is made by Cambium that the facts, conditions, information, circumstances or any underlying assumptions made by Cambium in connection with the work performed will not change after the work is completed and a report is submitted. If any such changes occur or additional information is obtained, Cambium should be advised and requested to consider if the changes or additional information affect its findings or results.

When preparing reports, Cambium considers applicable legislation, regulations, governmental guidelines and policies to the extent they are within its knowledge, but Cambium is not qualified to advise with respect to legal matters. The presentation of information regarding applicable legislation, regulations, governmental guidelines and policies is for information only and is not intended to and should not be interpreted as constituting a legal opinion concerning the work completed or conditions outlined in a report. All legal matters should be reviewed and considered by an appropriately qualified legal practitioner.

Site Assessments

A site assessment is created using data and information collected during the investigation of a site and based on conditions encountered at the time and particular locations at which fieldwork is conducted. The information, sample results and data collected represent the conditions only at the specific times at which and at those specific locations from which the information, samples and data were obtained and the information, sample results and data may vary at other locations and times. To the extent that Cambium's work or report considers any locations or times other than those from which information, sample results and data was specifically received, the work or report is based on a reasonable extrapolation from such information, sample results and data but the actual conditions encountered may vary from those extrapolations.

Only conditions at the site and locations chosen for study by the client are evaluated; no adjacent or other properties are evaluated unless specifically requested by the client. Any physical or other aspects of the site chosen for study by the client, or any other matter not specifically addressed in a report prepared by Cambium, are beyond the scope of the work performed by Cambium and such matters have not been investigated or addressed.

Reliance

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The client expressly agrees that Cambium employees shall have no personal liability to the client with respect to a claim, whether in contract, tort and/or other cause of action in law. Furthermore, the client agrees that it will bring no proceedings nor take any action in any court of law against Cambium employees in their personal capacity.

Water Balance Calculations

1386 and 1394 Greely Lane, Ottawa, Ontario

THORNTHWAITE-TYPE MONTHLY WATER-BALANCE MODEL

modified from Dingman 2015: Box 6-8 (pg 299) using ET model of Hamon (1963)

	Input Data					Computed Values							
											Surplus	397	mm/yr
Weather Station Location:	OTTAWA MACDONALD-CARTIER INT'L A *					**Latitude:**	45.3	degree					
Solar Declination (degree)	-20.6	-12.6	-1.5	10.0	19.0	23.1	21.0	13.4	2.6	-9.0	-18.5	-23.0	
DayLength (hr)*	9.0	10.3	11.8	13.4	14.7	15.4	15.0	13.9	12.4	10.8	9.4	8.6	

Available Water Storage Capacity	0.21	m/m	**Root Depth**	460	mm	**SOILmax** 96.6 mm

MONTHLY WATER BALANCE DATA

Temperatures in C, water-balance terms in mm.

Month:	J	F	M	A	M	J	J	A	S	O	N	D	Year
TEMPERATURE (T)	-10.3	-8.1	-2.3	6.3	13.3	18.5	21.0	19.8	15.0	8.0	1.5	-6.2	
PRECIPITATION (P)	65.4	54.3	64.4	74.5	80.3	92.8	91.9	85.5	90.1	86.1	81.9	76.4	944
RAIN	25.0	18.7	31.1	63.0	80.1	92.8	91.9	85.5	90.1	82.2	64.5	33.5	758
SNOW	40	36	33	12	0	0	0	0	0	4	17	43	185
MELT FACTOR (F)	0.00	0.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.25	0.00	
PACK	96	132	165	0	0	0	0	0	0	0	13	56	
MELT	0	0	0	177	0	0	0	0	0	4	4	0	185
INPUT (W)	25	19	31	240	80	93	92	86	90	86	69	34	944
POTENTIAL ET (PET)	0	0	0	41	73	101	118	101	65	38	21	0	557
NET INPUT (ΔW)	25	19	31	199	8	-8	-26	-16	25	48	48	34	
SOIL MOISTURE (SOIL)	97	97	97	97	97	89	68	58	82	97	97	97	
ΔSOIL	0	0	0	0	0	-8	-21	-10	25	14	0	0	0
ET	0	0	0	41	73	100	113	96	65	38	21	0	547
SURPLUS=W-ET-DSOIL	25	19	31	199	8	0	0	0	0	34	48	34	397

Notes:

Precipitation, Rain, Temperature, and Latitude are inputted parameters

SOILmax = available water storage capacity * root depth

m = month

D = Day length (hrs) $=2*\cos^{-1}(-\tan(\text{Latitude})*\tan(\text{Declination}))/0.2618$ [calculation is in radians]

$SNOW_m = P_m - RAIN_m$

$F_m = 0$ if $T_m <= 0^oC$; $F_m = 0.167*T_m$ if $0^oC<T_m<6^oC$; $F_m = 1$ if $T_m>=6^oC$

$PACK_m = (1-F_m)*(SNOW_m+PACK_{m-1})$

$MELT = F_m*(SNOW_m+PACK_{m-1})$

$W_m = RAIN_m+MELT_m$.

PET = 0 if $T_m<0$; otherwise $PET = 2.98*0.611*\exp(17.3*T_m/(T_m+237))/(T_m+237.2)*$Number of days in month [Hamon ET model (1963)]

$\Delta W_m = W_m - PET_m$

$SOIL = \min\{[\Delta W_m+SOIL_{m-1}], SOILmax\}$, if $\Delta Wm>0$; otherwise $SOIL = SOIL_{m-1} * \exp(\Delta W/SOILmax)$

$\Delta SOIL = SOIL_{m-1} - SOIL_m$

ET = PET if W_m > PET; otherwise, $ET=W_m-\Delta SOIL$

Pre- and Post-Development Water Balance Calculations

1386 AND 1394 Greely Lane, Ottawa, ON

1 Climate Information

Precipitation	944	mm/yr
Actual Evapotranspiration	547	mm/yr
Water Surplus	397	mm/yr

2 Infiltration Rates

Table 2 Approach - Infiltration factors	
Topography: Rolling to Hilly Land	0.15
Soil Type: Silty Loam	0.2
Cover: Cultivated land	0.1
Total Infiltration Factor	**0.45**

Infiltration (Water Surplus * Infiltration Factor)	**179**	mm/yr
Run-off (Water Surplus - Infiltration)	**218**	mm/yr

Table 3 Approach - Typical Recharge Rates		
Coarse Sand and Gravel	>250	mm/yr
Fine to medium sand	200-250	mm/yr
Silty sand to sandy silt	150-200	mm/yr
Silt	125-150	mm/yr
Clayey Silt	100- 125	mm/yr
Clay	<100	mm/yr

Site development area is underlain predominantly by silty sand to clayey silt.

Based on the above, the recharge rate is typically	125-150	mm/yr

3 Pre-Development Property Statistics	**ha**	**m^2**
Total Paved Area	0.08	855
Total Roof Area	0.04	353
Total Landscape Area	0.35	3,472
Total	**0.47**	**4,680**

4 Post-Development Property Statistics	**ha**	**m^2**
Total Paved Area	0.22	2,236
Total Roof Area	0.13	1,261
Total Landscape Area	0.12	1,183
Total	**0.47**	**4,680**

Pre- and Post-Development Water Balance Calculations

1386 AND 1394 Greely Lane, Ottawa, ON

5 Pre-Development Water Balance

Land Use		Area (m^2)	Precipitation (m^3)	Evapotranspiration (m^3)	Infiltration (m^3)	Run-off (m^3)
Impervious Areas	Paved Area	855	807	81	-	726
	Roof Area	353	333	33	-	300
Pervious Areas	Landscape Area	3,472	3,278	1,899	620	758
	Totals	4,680	4,418	2,013	620	1,784

Assuming no infiltration occurring in paved and roof areas, and 10% of precipitation to be evaporated from paved and roof areas.

6 Post-Development Water Balance

Land Use		Area (m^2)	Precipitation (m^3)	Evapotranspiration (m^3)	Infiltration (m^3)	Run-off (m^3)
Impervious Areas	Paved Area	2,236	2,111	211	-	1,900
	Roof Area	1,261	1,190	119	-	1,071
Pervious Areas	Landscape Area	1,183	1,117	647	211	258
	Totals	4,680	4,418	977	211	3,229

Assuming no infiltration occurring in paved and roof areas, and 10% of precipitation to be evaporated from paved and roof areas.

7 Comparision of Pre- and Post -Development

	Precipitation (m^3)	Evapotranspiration (m^3)	Infiltration (m^3)	Run-off (m^3)
Pre-Development	4,418	2,013	620	1,784
Post-Development	4,418	977	211	3,229
Change in Volume	-	- 1,036	- 409	1,445
Change in %	-	- 51	- 66	81

8 Requirement for Infiltration of Roof Run-off

Volume of Pre-Development Infiltration (m^3/yr)	620
Volume of Post-Development Infiltration (m^3/yr)	211
Deficit from Pre to Post Development Infiltration (m^3/yr)	409
Percentage of Roof Runoff required to match the pre-development infiltration (%)	38

Nitrate Attenuation

Input Data			Computed Values

Areas	LOT #1	Total
LOT AREA (m^2)	1183	1183

Surplus water

0.397	m/yr
1.0876712E-03	m/day
1.286715068	m^3/day

Infiltration Factor

Rolling to Hilly Land	0.15
Silty Loam	0.2
Cultivated land	0.1
Total	0.45

Infiltrated water

0.000489452	m/day
0.579021781	m^3/day

Runoff 0.707693288 m^3/day

Combined Concentrations at Property Boundaries

	Without Reduction	Waterloo Biofilter System (80.3% Nitrate Reduction)
Qe	1800	1800
Ce	40	7.88
Qi	579.0218	579.0217808
Ci	0.1	0.1
Qt	2379.022	2379.021781
mg/L	**30.29**	**5.99**

Appendix B
Borehole Logs

CAMBIUM

Client: Cassidy EW Construction
Contractor: OGS Inc.
Project No.: 17281-001 - B
Location: Ottawa, ON

Project Name: 1386 & 1394 Greely Lane
Method: Track Mounted Hollow Stem Auger
Elevation: 99.01 mASL
UTM: 18 T **N:** 5011868 **E:** 455169

Log of Borehole: BH101-23
Page: 1 of 1
Date Completed: March 8, 2023

Elevation (m)	Depth	Description	Number	Type	% Recovery	SPT (N)	Atterberg Limits (%) LL PL PI / % Moisture	Shear Strength Cu, kPa (nat V., rem V.) / SPT (N)	Well Installation	Log Notes
							25 50 75	20 40 60 80		
99	0	**ASPHALT:** 75 mm (98.93)								
		FILL: (SM) GRAVELLY SAND: brown, moist, some silt [base material] (0.08)	1A	SS	100	75	10%	75		
98.5	0.5	(98.55)	1B	SS			12.7%			
		FILL: (SM) SILTY SAND: grey, moist, gravelly (0.46)	2A	SS			18.8%			
98	1	(97.94)			83	7		7		
		(ML) sandy CLAYEY SILT: grey, cohesive, w>PL, firm (1.07)	2B	SS			18.8%			
97.5	1.5									1.5m: ATT SS3: 19.8%LL 12.5%PL
			3	SS	75	4	18.8%	4		
97	2									
			4A	SS	67	9	21%	9		
96.5	2.5	(96.42)	4B	SS			19.5%			
		(SM) SILTY SAND: grey, wet, trace clay (2.59)								
96	3	(95.96)								
		(ML) SILT: grey, non-cohesive, wet, compact, some sand, trace clay (3.05)	5	SS	63	15	17.1%	15		
95.5	3.5	-becomes moist, dense								
			6	SS	67	46	13.3%	46		
95	4									
94.5	4.5	-becomes very dense	7	SS	88	88	14%	88		
94	5									
93.5	5.5	-becomes wet, compact	8	SS	67	20	18%	20		
93	6	(92.91)								
		Borehole terminated @ 6.1 mbgs target depth achieved. (6.10)								Borehole caved at 2.1 mbgs. Groundwater encountered at 1.1 mbgs following completion.
92.5	6.5									
92	7									
91.5										

GRAINSIZE DISTRIBUTION

SAMPLE	GRAVEL	SAND	SILT	CLAY
SS1B	20	53	27	
SS3	0	22	57	21
SS6	0	19	77	4

Logged By: FI **Input By:** BV

Peterborough, Barrie, Whitby, Kingston, Ottawa

CAMBIUM

Client: Cassidy EW Construction
Contractor: OGS Inc.
Project No.: 17281-001 - B
Location: Ottawa, ON

Project Name: 1386 & 1394 Greely Lane
Method: Track Mounted Hollow Stem Auger
Elevation: 98.72 mASL
UTM: 18 T **N:** 5011843 **E:** 455153

Log of Borehole: BH102-23
Page: 1 of 1
Date Completed: March 8, 2023

Elevation (m)	Depth	Lithology	Description	Elevation / Depth	Number	Type	% Recovery	SPT (N)	Atterberg Limits (%) LL PL PI — 25 50 75 / % Moisture 25 50 75	Shear Strength Cu, kPa (nat V., rem V.) 20 40 60 80 / SPT (N) 20 40 60 80	Well Installation	Log Notes
98.7	0		**TOPSOIL:** 100 mm	98.62	1A	SS			20.9%			
			FILL: (SM) SILTY SAND: brown, wet, compact, gravelly, with roots	0.10	1B	SS	63	11	13.6%	11		
98.2	0.5											
				97.75	2A	SS			20%			
97.7	1		**(ML) sandy CLAYEY SILT:** grey, cohesive, w>PL, firm	0.97	2B	SS	92	4	19.5%	4		
97.2	1.5											
					3	SS	75	4	19.8%	4		
96.7	2		-becomes soft									
96.2	2.5				4	SS	67	3	18.5%	3		
95.7	3			95.67								
			(ML) SILT: grey, non-cohesive, wet, compact, some sand, trace clay	3.05	5	SS	42	18	15.3%	18		
95.2	3.5		-becomes very dense									
94.7	4				6	SS	71	63	13.4%	63		
94.2	4.5				7	SS	79	69	13.0%	69		
93.7	5											
93.2	5.5				8	SS	75	56	14.4%	56		
92.7	6			92.62								
			Borehole terminated @ 6.1 mbgs target depth achieved.	6.10								
92.2	6.5											
91.7	7											Borehole caved at 4.0 mbgs. Groundwater measured at 1.5 mbgs following completion.
91.2												

GRAINSIZE DISTRIBUTION: SAMPLE | GRAVEL | SAND | SILT | CLAY

Logged By: FI **Input By:** BV

CAMBIUM

Client: Cassidy EW Construction
Contractor: OGS Inc.
Project No.: 17281-001 - B
Location: Ottawa, ON

Project Name: 1386 & 1394 Greely Lane
Method: Track Mounted Hollow Stem Auger
Elevation: 98.71 mASL
UTM: 18 T **N:** 5011831 **E:** 455195

Log of Borehole: BH103-23
Page: 1 of 1
Date Completed: March 8, 2023

Elevation (m)	Depth	Lithology	Description	Elevation / Depth	Number	Type	% Recovery	SPT (N)	Atterberg Limits (%) LL PL PI — 25 50 75 / % Moisture 25 50 75	Shear Strength Cu, kPa (nat V., rem V.) 20 40 60 80 / SPT (N) 20 40 60 80	Well Installation	Log Notes
98.7	0		**TOPSOIL:** 300 mm	98.41	1A	SS	67	2	39.1%	2		
98.2	0.5		**FILL:** (SM) SILTY SAND: grey, wet, trace gravel	0.30 / 97.72	1B	SS			22%			
					2A	SS			22.4%			
97.7	1		**(ML) sandy CLAYEY SILT:** grey, cohesive, w>PL, firm	0.99	2B	SS	79	4	19.3%	4		
97.2	1.5		-becomes stiff		3	SS	88	8	16.7%	8		
96.7	2		-decrease in clay content, becomes CL-ML									
96.2	2.5				4	SS	92	10	15.6%	10		
95.7	3			95.66								
95.2	3.5		**(ML) sandy SILT:** grey, non-cohesive, wet, compact, trace clay	3.05	5	SS	88	17	14.3%	17		
94.7	4				6	SS	79	15	14.1%	15		
94.2	4.5		-becomes dense		7	SS	71	39	13.6%	39		
93.7	5											
93.2	5.5				8	SS	71	47	13.7%	47		
92.7	6			92.61								
92.2	6.5		Borehole terminated @ 6.1 mbgs target depth achieved.	6.10								Borehole caved at 4.9 mbgs. Groundwater measured at 0.9 mbgs following completion.
91.7	7											
91.2												

GRAINSIZE DISTRIBUTION: SAMPLE | GRAVEL | SAND | SILT | CLAY

Logged By: FI **Input By:** BV

Peterborough, Barrie, Whitby, Kingston, Ottawa

CAMBIUM

Client: Cassidy EW Construction
Contractor: OGS Inc.
Project No.: 17281-001 - B
Location: Ottawa, ON

Project Name: 1386 & 1394 Greely Lane
Method: Track Mounted Hollow Stem Auger
Elevation: 98.78 mASL
UTM: 18 T **N:** 5011812 **E:** 455184

Log of Borehole: BH104-23
Page: 1 of 1
Date Completed: March 8, 2023

Elevation (m)	Depth	Lithology	Description	Elevation / Depth	Number	Type	% Recovery	SPT (N)	Atterberg Limits (%) LL PL PI / % Moisture	Shear Strength Cu, kPa (nat V., rem V.) / SPT (N)	Well Installation	Log Notes
98.8	0		**TOPSOIL:** 125 mm	98.65	1A	SS			38.1%			
					1B	SS			18.5%			
			FILL: (SM) SILTY SAND: brown, wet, very loose	0.13	1C	SS	42	3	31.3%	3		
98.3	0.5											
					2A	SS			20.7%			
97.8	1			97.81								
			(ML) sandy CLAYEY SILT: grey, cohesive, w>PL, firm	0.97	2B	SS	75	4	20%	4		
97.3	1.5											
					3	SS	79	4	18.2%	4		
96.8	2		-decrease in clay content, becomes CL-ML, soft									
					4	SS	100	3	18%	3		2.3m: ATT SS4: 18.5%LL 13.1%PL
96.3	2.5											
95.8	3		-100 mm silty sand seam	95.55								
			(ML) sandy SILT: grey, non-cohesive, wet, compact, trace clay	3.23	5	SS	83	10	16.3%	10		
95.3	3.5											
94.8	4				6	SS	75	26	14.3%	26		
94.3	4.5											
					7	SS	83	28	13.9%	28		
93.8	5											
93.3	5.5		-becomes dense									
					8	SS	79	39	13.9%	39		
92.8	6			92.68								
			Borehole terminated @ 6.1 mbgs target depth achieved.	6.10								
92.3	6.5											Borehole caved at 4.6 mbgs. Groundwater measured at 0.6 mbgs following completion.
91.8	7											
91.3												

GRAINSIZE DISTRIBUTION

SAMPLE	GRAVEL	SAND	SILT	CLAY
SS4	0	25	57	18
SS6	0	22	74	4

Logged By: FI **Input By:** BV

Peterborough, Barrie, Whitby, Kingston, Ottawa

CAMBIUM

Client: Cassidy EW Construction
Contractor: OGS Inc.
Project No.: 17281-001 - B
Location: Ottawa, ON

Project Name: 1386 & 1394 Greely Lane
Method: Track Mounted Hollow Stem Auger
Elevation: 98.91 mASL
UTM: 18 T **N:** 5011843 **E:** 455141

Log of Borehole: BH105-23
Page: 1 of 1
Date Completed: March 8, 2023

Elevation (m)	Depth	Lithology	Description	Elevation / Depth	Number	Type	CSV (ppm)	OV (ppm)	% Recovery	SPT (N)/DCPT	Atterberg Limits (%) LL PL PI 25 50 75 / % Moisture 25 50 75	Shear Strength Cu, kPa nat V. rem V. 20 40 60 80 / SPT (N) / DCPT 20 40 60 80	Well Installation	Log Notes
98.9	0		**TOPSOIL:** 150 mm	98.76	1A	SS	ND	ND			37.1%		Cap	
			FILL: (SM) SILTY SAND: brown, wet, loose, some gravel, trace clay	0.15	1B	SS	ND	ND	67	7	19.3%	7	Bentonite Plug, Riser	
98.4	0.5		-becomes grey, decrease in silt content		2A	SS	ND	ND			12.1%			
97.9	1				2B	SS	ND	ND	63	11	15.4%	11		
97.4	1.5			97.39										
			(ML) sandy CLAYEY SILT: grey, cohesive, w>PL, firm	1.52	3	SS	ND	ND	92	4	19.9%	4	Sand Pack, PVC Screen	Groundwater measured at 2.0 mbgs following completion.
96.9	2													
				96.47	4A	SS	ND	ND			20%			
96.4	2.5		**(ML) SILT:** grey, non-cohesive, wet, loose, some sand, trace clay	2.44	4B	SS	ND	ND	63	5	18.1%	5		
95.9	3		-becomes compact											
95.4	3.5			95.25	5	SS	ND	ND	50	16	16.1%	16	Cap	
			Borehole terminated @ 3.7 mbgs target depth achieved.	3.66										
94.9	4													
94.4	4.5													
93.9	5													
93.4	5.5													
92.9	6													
92.4	6.5													
91.9	7													
91.4														

GRAINSIZE DISTRIBUTION: SAMPLE | GRAVEL | SAND | SILT | CLAY

Logged By: FI **Input By:** BV

Peterborough, Barrie, Ottawa, Kingston, Whitby

CAMBIUM

Client: Cassidy EW Construction
Contractor: OGS Inc.
Project No.: 17281-001 - B
Location: Ottawa, ON

Project Name: 1386 & 1394 Greely Lane
Method: Track Mounted Hollow Stem Auger
Elevation: 98.64 mASL
UTM: 18 T **N:** 5011800 **E:** 455161

Log of Borehole: BH106-23
Page: 1 of 1
Date Completed: March 7, 2023

Elevation (m)	Depth	Lithology	Description	Elevation / Depth	Number	Type	CSV (ppm)	OV (ppm)	% Recovery	SPT (N)/DCPT	Atterberg Limits (%) LL PL PI — 25 50 75 / % Moisture 25 50 75	Shear Strength Cu, kPa (nat V., rem V.) 20 40 60 80 / SPT (N) / DCPT 20 40 60 80	Well Installation	Log Notes
98.6	0		**TOPSOIL:** 125 mm	98.51	1A	SS	ND	ND			51.5%		Cap	
				0.13	1B	SS	ND	ND			19.4%			
			FILL: (SM) SILTY SAND: brown, wet, very loose, trace gravel		1C	SS	ND	ND	54	3	31.1%	3	Bentonite Plug Riser	
98.1	0.5													
				97.78	2A	SS	ND	ND			22.9%			
97.6	1		**(ML) sandy CLAYEY SILT:** grey, cohesive, w>PL, soft	0.86	2B	SS	ND	ND	75	3	19%	3		
97.1	1.5		-becomes firm											Groundwater measured at 1.5 mbgs following completion.
					3	SS	ND	ND	100	5	18.3%	5	Sand Pack PVC Screen	
96.6	2													
96.1	2.5				4	SS	ND	ND	92	6	19.1%	6		
95.6	3			95.59										
			(ML) sandy SILT: grey, non-cohesive, wet, compact, trace clay	3.05	5	SS	ND	ND	75	15	15.8%	15		
95.1	3.5			94.98										
			Borehole terminated @ 3.7 mbgs target depth achieved.	3.66									Cap	
94.6	4													
94.1	4.5													
93.6	5													
93.1	5.5													
92.6	6													
92.1	6.5													
91.6	7													
91.1														

GRAINSIZE DISTRIBUTION: SAMPLE | GRAVEL | SAND | SILT | CLAY

Logged By: FI **Input By:** BV

Peterborough, Barrie, Ottawa, Kingston, Whitby

CAMBIUM

Client: Cassidy EW Construction
Contractor: OGS Inc.
Project No.: 17281-001 - B
Location: Ottawa, ON

Project Name: 1386 & 1394 Greely Lane
Method: Track Mounted Hollow Stem Auger
Elevation: 98.12 mASL
UTM: 18 T **N:** 5011845 **E:** 455203

Log of Borehole: BH107-23
Page: 1 of 1
Date Completed: March 8, 2023

Elevation (m)	Depth	Description	Number	Type	CSV (ppm)	OV (ppm)	% Recovery	SPT (N)/DCPT	Atterberg Limits (%) LL PL PI	% Moisture	Shear Strength Cu, kPa (nat V., rem V.)	SPT (N) / DCPT	Well Installation	Log Notes
98.1	0	**TOPSOIL:** 75 mm (98.04 / 0.08)	1A	SS	ND	ND	79	6	53.8%	24%		6	Cap	Groundwater measured at 1.8 mbgs following completion.
		FILL: (SM) SILTY SAND: brown, wet, trace clay (97.82 / 0.30)	1B	SS	ND	ND							Bentonite Plug	
97.6	0.5	**(ML) sandy CLAYEY SILT:** grey, cohesive, w>PL, stiff	1C	SS	ND	ND				17.2%			Riser	
97.1	1		2	SS	ND	ND	79	9		15.9%		9		
96.6	1.5	-becomes firm												
			3	SS	ND	ND	100	7		15.4%		7	Sand Pack	
96.1	2	(95.83)											PVC Screen	
95.6	2.5	**(ML) sandy SILT:** grey, non-cohesive, wet, compact, trace clay (2.29)	4	SS	ND	ND	75	17		15.1%		17		
95.1	3													
94.6	3.5	(94.46)	5	SS	ND	ND	63	16		14.8%		16	Cap	
94.1	4	Borehole terminated @ 3.7 mbgs target depth achieved. (3.66)												
93.6	4.5													
93.1	5													
92.6	5.5													
92.1	6													
91.6	6.5													
91.1	7													
90.6														

GRAINSIZE DISTRIBUTION: SAMPLE | GRAVEL | SAND | SILT | CLAY

Logged By: FI **Input By:** BV

CAMBIUM

Client: Cassidy EW Construction
Contractor: OGS Inc.
Project No.: 17281-001 - B
Location: Ottawa, ON

Project Name: 1386 & 1394 Greely Lane
Method: Track Mounted Hollow Stem Auger
Elevation: 99.06 mASL
UTM: 18 T **N:** 5011890 **E:** 455169

Log of Borehole: BH108-23
Page: 1 of 1
Date Completed: March 8, 2023

Elevation (m)	Depth	Lithology	Description	Elevation / Depth	Number	Type	% Recovery	SPT (N)	Atterberg Limits (%) LL PL PI — 25 50 75 / % Moisture 25 50 75	Shear Strength Cu, kPa (nat V., rem V.) 20 40 60 80 / SPT (N) 20 40 60 80	Well Installation	Log Notes
99.1	0		**ASPHALT:** 50 mm	99.01 / 0.05								
			FILL: (SM) GRAVELLY SAND, brown, wet, some silt [base material]	98.45	1A	SS	100	64	14.2%	64		
98.6	0.5		**FILL:** (SM) SAND and SILT: grey, wet	0.61	1B	SS			31.5%			
98.1	1			97.99	2A	SS			16.6%			
			(ML) sandy CLAYEY SILT: grey, non-cohesive, w>PL, firm	1.07 / 97.54	2B	SS	67	3	20.1%	3		
97.6	1.5		Borehole terminated @ 1.5 mbgs target depth achieved.	1.52								
97.1	2											Borehole remained open. Groundwater measured at 0.8 mbgs following completion.
96.6	2.5											
96.1	3											
95.6	3.5											
95.1	4											
94.6	4.5											
94.1	5											
93.6	5.5											
93.1	6											
92.6	6.5											
92.1	7											
91.6												

GRAINSIZE DISTRIBUTION

SAMPLE	GRAVEL	SAND	SILT	CLAY
SS1B	0	63	37	

Logged By: FI **Input By:** BV

CAMBIUM

Client: Cassidy EW Construction
Contractor: OGS Inc.
Project No.: 17281-001 - B
Location: Ottawa, ON

Project Name: 1386 & 1394 Greely Lane
Method: Track Mounted Hollow Stem Auger
Elevation: 98.60 mASL
UTM: 18 T **N:** 5011847 **E:** 455186

Log of Borehole: BH109-23
Page: 1 of 1
Date Completed: March 7, 2023

Elevation (m)	Depth	Lithology	Description	Elevation / Depth	Number	Type	% Recovery	SPT (N)	Atterberg Limits (%) LL PL PI 25 50 75 / % Moisture 25 50 75	Shear Strength Cu, kPa nat V. rem V. 20 40 60 80 / SPT (N) 20 40 60 80	Well Installation	Log Notes
98.6	0		**TOPSOIL:** 915 mm	97.69	1	SS	25	2	44.3%	2		
98.1	0.5											
97.6	1		**FILL:** (SM) SILTY SAND: grey, wet	0.91 / 97.58	2A	SS			28.9%			
97.1	1.5		**(ML) sandy CLAYEY SILT:** grey, non-cohesive, w>PL, soft	1.02 / 97.08	2B	SS	83	3	20.4%	3		
96.6	2		Borehole terminated @ 1.5 mbgs target depth achieved.	1.52								Borehole remained open. Groundwater measured at 1.1 mbgs following completion.
96.1	2.5											
95.6	3											
95.1	3.5											
94.6	4											
94.1	4.5											
93.6	5											
93.1	5.5											
92.6	6											
92.1	6.5											
91.6	7											
91.1												

GRAINSIZE DISTRIBUTION: SAMPLE | GRAVEL | SAND | SILT | CLAY

Logged By: FI **Input By:** BV

Peterborough, Barrie, Whitby, Kingston, Ottawa

Appendix C

Grain Size Analysis Results

Grain Size Distribution Chart

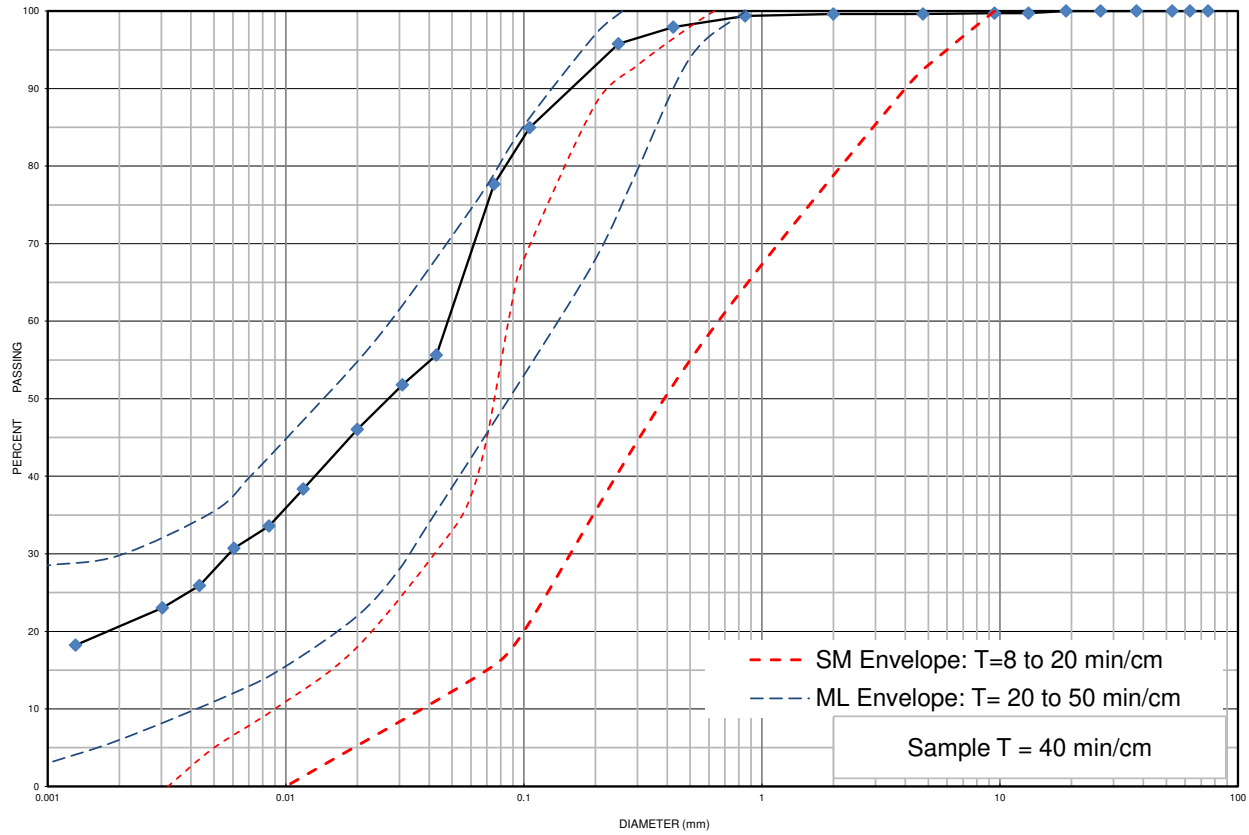
Project Number: *17281-002* **Client:** *Cassidy E.W. Construction Consultant Ltd.*

Project Name: *Hydrogeological Assessment - 1386 & 1394 Greely Lane, Ottawa*

Sample Date: *March 7-8, 2023* **Sampled By:** *Farhan Imtiaz - Cambium Inc.*

Location: *BH 101-23 SS 3* **Depth:** *1.5 m to 2.1 m* **Lab Sample No:** *S-23-0475*

UNIFIED SOIL CLASSIFICATION SYSTEM					
CLAY & SILT (<0.075 mm)	SAND (<4.75 mm to 0.075 mm)			GRAVEL (>4.75 mm)	
	FINE	MEDIUM	COARSE	FINE	COARSE



MIT SOIL CLASSIFICATION SYSTEM								
CLAY	SILT	FINE	MEDIUM	COARSE	FINE	MEDIUM	COARSE	BOULDERS
		SAND			GRAVEL			

Borehole No.	Sample No.	Depth	Gravel	Sand	Silt	Clay	Moisture
BH 101-23	SS 3	1.5 m to 2.1 m	0	22	57	21	18.8

Description	Classification	D_{60}	D_{30}	D_{10}	C_u	C_c
Sandy Clayey Silt	ML	0.0480	0.0058	-	-	-

Additional information availabe upon request

Issued By: _____ *Date Issued:* *March 20, 2024*

(Senior Project Manager)

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Grain Size Distribution Chart

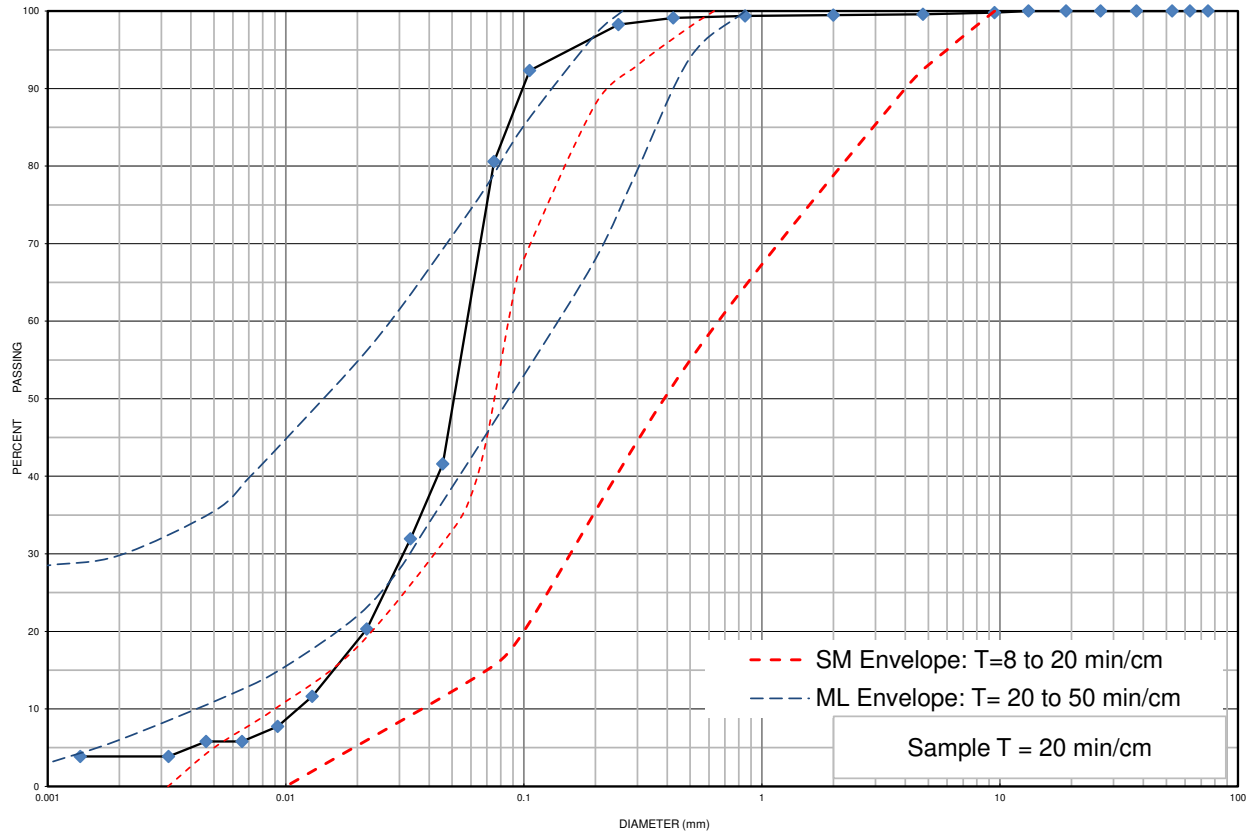
Project Number: *17281-002* **Client:** *Cassidy E.W. Construction Consultant Ltd.*

Project Name: *Hydrogeological Assessment - 1386 & 1394 Greely Lane, Ottawa*

Sample Date: *March 7-8, 2023* **Sampled By:** *Farhan Imtiaz - Cambium Inc.*

Location: *BH 101-23 SS 6* **Depth:** *3.8 m to 4.4 m* **Lab Sample No:** *S-23-0476*

UNIFIED SOIL CLASSIFICATION SYSTEM					
CLAY & SILT (<0.075 mm)	SAND (<4.75 mm to 0.075 mm)			GRAVEL (>4.75 mm)	
	FINE	MEDIUM	COARSE	FINE	COARSE



MIT SOIL CLASSIFICATION SYSTEM								
CLAY	SILT	FINE	MEDIUM	COARSE	FINE	MEDIUM	COARSE	BOULDERS
		SAND			GRAVEL			

Borehole No.	Sample No.	Depth	Gravel	Sand	Silt	Clay	Moisture
BH 101-23	SS 6	3.8 m to 4.4 m	0	19	77	4	13.3

Description	Classification	D_{60}	D_{30}	D_{10}	C_u	C_c
Silt some Sand trace Clay	ML	0.057	0.032	0.012	4.75	1.50

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Grain Size Distribution Chart

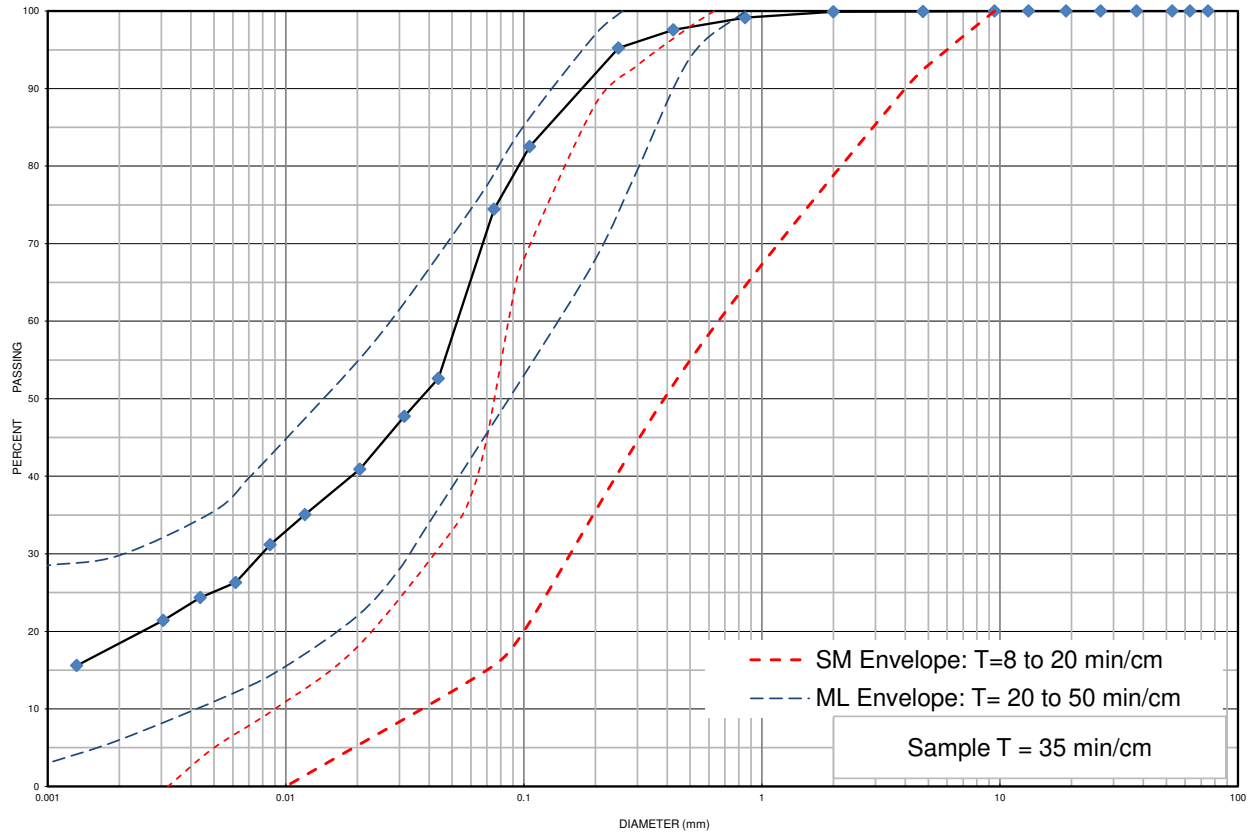
Project Number: *17281-002* **Client:** *Cassidy E.W. Construction Consultant Ltd.*

Project Name: *Hydrogeological Assessment - 1386 & 1394 Greely Lane, Ottawa*

Sample Date: *March 7-8, 2023* **Sampled By:** *Farhan Imtiaz - Cambium Inc.*

Location: *BH 104-23 SS 4* **Depth:** *2.3 m to 2.9 m* **Lab Sample No:** *S-23-0477*

UNIFIED SOIL CLASSIFICATION SYSTEM					
CLAY & SILT (<0.075 mm)	SAND (<4.75 mm to 0.075 mm)			GRAVEL (>4.75 mm)	
	FINE	MEDIUM	COARSE	FINE	COARSE



MIT SOIL CLASSIFICATION SYSTEM								
CLAY	SILT	FINE	MEDIUM	COARSE	FINE	MEDIUM	COARSE	BOULDERS
		SAND			GRAVEL			

Borehole No.	Sample No.	Depth	Gravel	Sand	Silt	Clay	Moisture
BH 104-23	SS 4	2.3 m to 2.9 m	0	25	57	18	18.0

Description	Classification	D_{60}	D_{30}	D_{10}	C_u	C_c
Sandy Silt some Clay	ML	0.053	0.008	-	-	-

Additional information availabe upon request

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Grain Size Distribution Chart

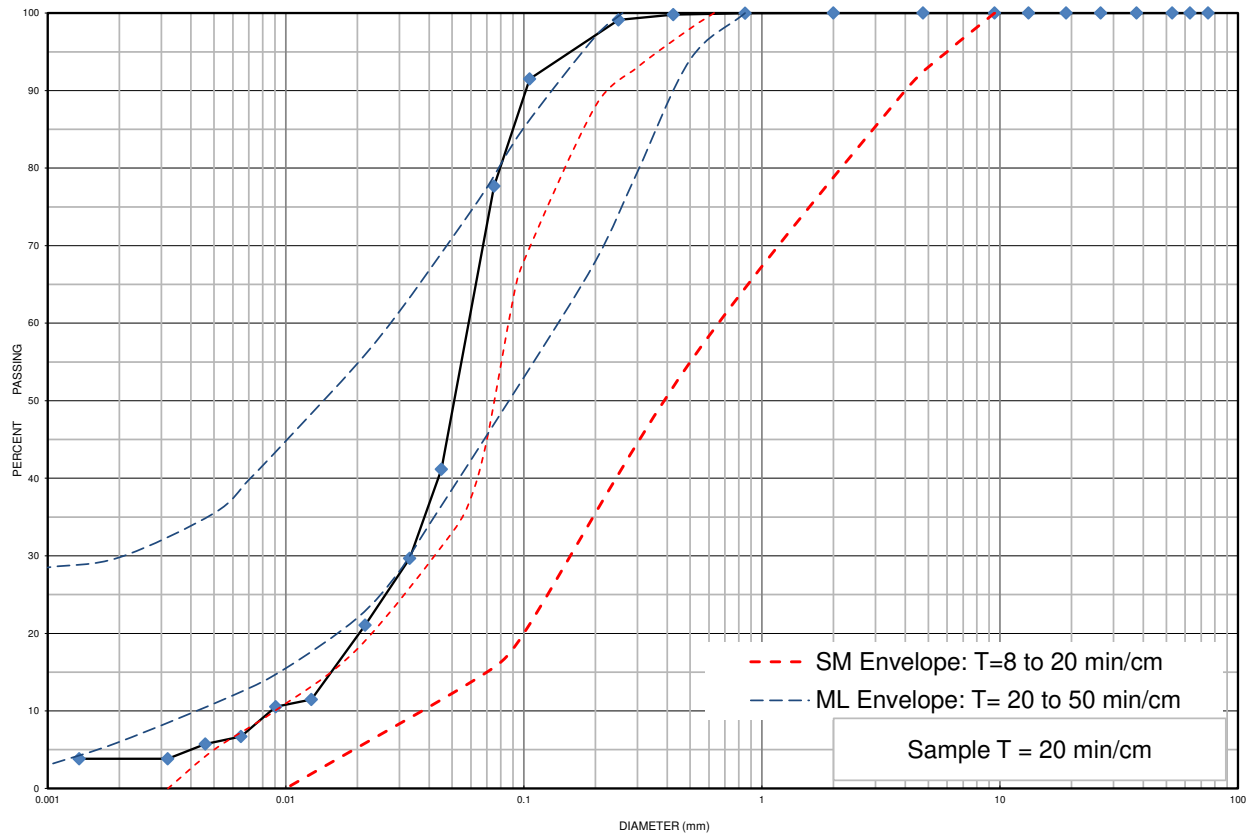
Project Number: *17281-002* **Client:** *Cassidy E.W. Construction Consultant Ltd.*

Project Name: *Hydrogeological Assessment - 1386 & 1394 Greely Lane, Ottawa*

Sample Date: *March 7-8, 2023* **Sampled By:** *Farhan Imtiaz - Cambium Inc.*

Location: *BH 104-23 SS 6* **Depth:** *3.8 m to 4.4 m* **Lab Sample No:** *S-23-0478*

UNIFIED SOIL CLASSIFICATION SYSTEM					
CLAY & SILT (<0.075 mm)	SAND (<4.75 mm to 0.075 mm)			GRAVEL (>4.75 mm)	
	FINE	MEDIUM	COARSE	FINE	COARSE



MIT SOIL CLASSIFICATION SYSTEM								
CLAY	SILT	FINE	MEDIUM	COARSE	FINE	MEDIUM	COARSE	BOULDERS
		SAND			GRAVEL			

Borehole No.	Sample No.	Depth	Gravel	Sand	Silt	Clay	Moisture
BH 104-23	SS 6	3.8 m to 4.4 m	0	22	74	4	14.3

Description	Classification	D_{60}	D_{30}	D_{10}	C_u	C_c
Sandy Silt trace Clay	ML	0.0590	0.0340	0.0087	6.78	2.25

Additional information availabe upon request

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Appendix D

Well Inventory Survey Results

Water Well Records Summary Report

Produced by Cambium Inc. using MOECP Water Well Information System (WWIS)

All units in meters unless otherwise specified

Well ID: 1507224
Construction Date: 1965-09-22
Easting: 455211
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 100 m - 300 m

Well Depth:	20.7	**Water Kind**	FRESH	**Pump Rate (LPM):**	23
Well Diameter (cm):	15.2	**Final Status**	Water Supply	**Recommended Pump Rate:**	23
Water First Found:	16.8	**Primary Water Use:**	Domestic	**Pumping Duration (h:m):**	0 : 30
Static Level:	6				

Layer:	Driller's Description:	Top:	Bottom:
1	MEDIUM SAND	0	4.57
2	LIMESTONE	4.57	20.7

Well ID: 1507232
Construction Date: 1964-07-06
Easting: 454801
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 100 m - 300 m

Well Depth:	20.4	**Water Kind**	FRESH	**Pump Rate (LPM):**	32
Well Diameter (cm):	5.08	**Final Status**	Water Supply	**Recommended Pump Rate:**	23
Water First Found:	20.4	**Primary Water Use:**	Domestic	**Pumping Duration (h:m):**	2 : 0
Static Level:	2				

Layer:	Driller's Description:	Top:	Bottom:
1	MEDIUM SAND	0	5.49
2	BOULDERS	5.49	14.0
3	LIMESTONE	14.0	20.4

Well ID: 1507234
Construction Date: 1964-07-06
Easting: 454851
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 100 m - 300 m

Well Depth:	20.7	**Water Kind**	FRESH	**Pump Rate (LPM):**	45
Well Diameter (cm):	5.08	**Final Status**	Water Supply	**Recommended Pump Rate:**	23
Water First Found:	20.7	**Primary Water Use:**	Domestic	**Pumping Duration (h:m):**	2 : 0
Static Level:	1				

Layer:	Driller's Description:	Top:	Bottom:
1	MEDIUM SAND	0	5.49
2	BOULDERS	5.49	14.3
3	LIMESTONE	14.3	20.7

Well ID: 1507313
Construction Date: 1966-12-06
Easting: 455541
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 100 m - 300 m

Well Depth:	18.3	**Water Kind**	FRESH	**Pump Rate (LPM):**	27
Well Diameter (cm):	12.7	**Final Status**	Water Supply	**Recommended Pump Rate:**	23
Water First Found:	15.2	**Primary Water Use:**	Domestic	**Pumping Duration (h:m):**	1 : 0
Static Level:	6				

Layer:	Driller's Description:	Top:	Bottom:
1	GRAVEL	0	5.49
2	LIMESTONE	5.49	18.3

Well ID: 1509840
Construction Date: 1968-08-21
Easting: 455391
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 100 m - 300 m

Well Depth: 12.8
Well Diameter (cm): 10.2
Water First Found: 12.8
Static Level: 6
Water Kind FRESH
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 45
Recommended Pump Rate: 23
Pumping Duration (h:m): 0 : 30

Layer:	Driller's Description:	Top:	Bottom:
1	TOPSOIL	0	0.91
2	HARDPAN	0.91	3.96
3	LIMESTONE	3.96	12.8

Well ID: 1510585
Construction Date: 1970-05-28
Easting: 455331
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 100 m - 300 m

Well Depth: 32.9
Well Diameter (cm): 15.2
Water First Found: 32
Static Level: 5
Water Kind FRESH
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 45
Recommended Pump Rate: 36
Pumping Duration (h:m): 1 : 0

Layer:	Driller's Description:	Top:	Bottom:
1	TOPSOIL	0	1.52
2	GRAVEL	1.52	5.18
3	LIMESTONE	5.18	32.9

Well ID: 1512221
Construction Date: 1973-01-12
Easting: 455604
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 300 m - 1 km

Well Depth: 14.6
Well Diameter (cm): 15.2
Water First Found: 14.0
Static Level: 4
Water Kind FRESH
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 91
Recommended Pump Rate: 23
Pumping Duration (h:m): 1 : 0

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	2.74
2	SAND	2.74	12.2
3	LIMESTONE	12.2	14.6

Well ID: 1513408
Construction Date: 1973-09-10
Easting: 455523
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 10.7
Well Diameter (cm): 12.7
Water First Found: 9.75
Static Level: 6
Water Kind FRESH
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 36
Recommended Pump Rate: 23
Pumping Duration (h:m): 1 : 57

Layer:	Driller's Description:	Top:	Bottom:
1	HARDPAN	0	7.62
2	LIMESTONE	7.62	10.7

Well ID: 1513421
Construction Date: 1973-09-26
Easting: 455556
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 300 m - 1 km

Well Depth: 13.1
Well Diameter (cm): 12.7
Water First Found: 13.1
Static Level: 5
Water Kind FRESH
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 68
Recommended Pump Rate: 45
Pumping Duration (h:m): 1 : 10

Layer:	Driller's Description:	Top:	Bottom:
1	HARDPAN	0	13.1

Well ID: 1515384
Construction Date: 1976-06-19
Easting: 455451
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 38.1
Well Diameter (cm):
Water First Found: 12.8
Static Level: 6
Water Kind Not stated
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 18
Recommended Pump Rate: 18
Pumping Duration (h:m): 2 : 0

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	5.79
1	SAND	0	5.79
1	SAND	0	5.79
1	SAND	0	5.79
2	LIMESTONE	5.79	38.1
2	LIMESTONE	5.79	38.1
2	LIMESTONE	5.79	38.1
2	LIMESTONE	5.79	38.1

Well ID: 1515531
Construction Date: 1976-08-13
Easting: 455551
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 16.8
Well Diameter (cm): 15.2
Water First Found: 16.1
Static Level: 6
Water Kind FRESH
Final Status Water Supply
Primary Water Use: Municipal
Pump Rate (LPM): 91
Recommended Pump Rate: 68
Pumping Duration (h:m): 1 : 30

Layer:	Driller's Description:	Top:	Bottom:
1	GRAVEL	0	8.23
2	HARDPAN	8.23	15.2
3	SANDSTONE	15.2	15.5
4	UNKNOWN TYPE	15.5	16.8

Well ID: 1517024
Construction Date: 1979-07-09
Easting: 455530
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 15.5
Well Diameter (cm): 15.2
Water First Found: 14.6
Static Level: 6
Water Kind FRESH
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 91
Recommended Pump Rate: 55
Pumping Duration (h:m): 1 : 30

Layer:	Driller's Description:	Top:	Bottom:
1	HARDPAN	0	4.88
2	SAND	4.88	13.7
3	GRAVEL	13.7	14.3
4	LIMESTONE	14.3	15.5

Well ID: 1517148 **Easting:** 455430 **UTM Zone** 18
Construction Date: 1979-10-05 **Northing:** 5E+06 **Positional Accuracy:** margin of error : 30 m - 100 m

Well Depth:	16.8	**Water Kind**	FRESH	**Pump Rate (LPM):**	91
Well Diameter (cm):	15.2	**Final Status**	Water Supply	**Recommended Pump Rate:**	45
Water First Found:	13.7	**Primary Water Use:**	Livestock	**Pumping Duration (h:m):**	1 : 30
Static Level:	2				

Layer:	Driller's Description:	Top:	Bottom:
1	HARDPAN	0	11.6
2	SAND	11.6	13.7
3	LIMESTONE	13.7	16.8

Well ID: 1517152 **Easting:** 455530 **UTM Zone** 18
Construction Date: 1979-10-05 **Northing:** 5E+06 **Positional Accuracy:** margin of error : 30 m - 100 m

Well Depth:	15.5	**Water Kind**	FRESH	**Pump Rate (LPM):**	114
Well Diameter (cm):	15.2	**Final Status**	Water Supply	**Recommended Pump Rate:**	68
Water First Found:	14.9	**Primary Water Use:**	Domestic	**Pumping Duration (h:m):**	1 : 30
Static Level:	5				

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	10.7
2	HARDPAN	10.7	12.2
3	LIMESTONE	12.2	15.5

Well ID: 1517154 **Easting:** 455530 **UTM Zone** 18
Construction Date: 1979-10-05 **Northing:** 5E+06 **Positional Accuracy:** margin of error : 30 m - 100 m

Well Depth:	16.2	**Water Kind**	FRESH	**Pump Rate (LPM):**	82
Well Diameter (cm):	15.2	**Final Status**	Water Supply	**Recommended Pump Rate:**	45
Water First Found:	14.9	**Primary Water Use:**	Domestic	**Pumping Duration (h:m):**	1 : 30
Static Level:	6				

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	13.1
2	LIMESTONE	13.1	16.1

Well ID: 1517156 **Easting:** 455530 **UTM Zone** 18
Construction Date: 1979-10-05 **Northing:** 5E+06 **Positional Accuracy:** margin of error : 30 m - 100 m

Well Depth:	15.2	**Water Kind**	FRESH	**Pump Rate (LPM):**	82
Well Diameter (cm):	15.2	**Final Status**	Water Supply	**Recommended Pump Rate:**	36
Water First Found:	14.3	**Primary Water Use:**	Domestic	**Pumping Duration (h:m):**	2 : 20
Static Level:	5				

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	12.5
2	LIMESTONE	12.5	15.2

Well ID: 1517638 **Easting:** 455630 **UTM Zone** 18
Construction Date: 1981-09-08 **Northing:** 5E+06 **Positional Accuracy:** margin of error : 30 m - 100 m

Well Depth:	12.5	**Water Kind**	FRESH	**Pump Rate (LPM):**	136
Well Diameter (cm):	15.2	**Final Status**	Water Supply	**Recommended Pump Rate:**	23
Water First Found:	12.2	**Primary Water Use:**	Domestic	**Pumping Duration (h:m):**	1 : 0
Static Level:	4				

Layer:	Driller's Description:	Top:	Bottom:

1	CLAY	0	9.45
2	SHALE	9.45	12.5

Well ID: 1518000 **Easting:** 455630 **UTM Zone** 18
Construction Date: 1982-11-29 **Northing:** 5E+06 **Positional Accuracy:** margin of error : 30 m - 100 m

Well Depth: 13.1 **Water Kind** FRESH **Pump Rate (LPM):** 91
Well Diameter (cm): 15.2 **Final Status** Water Supply **Recommended Pump Rate:** 45
Water First Found: 12.8 **Primary Water Use:** Domestic **Pumping Duration (h:m):** 1 : 0
Static Level: 5

Layer:	Driller's Description:	Top:	Bottom:
1	TOPSOIL	0	1.83
2	QUICKSAND	1.83	12.2
3	SAND	12.2	12.5
4	LIMESTONE	12.5	13.1

Well ID: 1518291 **Easting:** 455630 **UTM Zone** 18
Construction Date: 1983-06-20 **Northing:** 5E+06 **Positional Accuracy:** margin of error : 30 m - 100 m

Well Depth: 14.6 **Water Kind** FRESH **Pump Rate (LPM):** 45
Well Diameter (cm): 15.2 **Final Status** Water Supply **Recommended Pump Rate:** 23
Water First Found: 14.3 **Primary Water Use:** Public **Pumping Duration (h:m):** 1 : 0
Static Level: 4

Layer:	Driller's Description:	Top:	Bottom:
1	SILT	0	3.66
2	TILL	3.66	11.9
3	STONES	11.9	14.6

Well ID: 1518419 **Easting:** 455430 **UTM Zone** 18
Construction Date: 1983-08-24 **Northing:** 5E+06 **Positional Accuracy:** margin of error : 30 m - 100 m

Well Depth: 19.8 **Water Kind** FRESH **Pump Rate (LPM):** 136
Well Diameter (cm): 15.2 **Final Status** Water Supply **Recommended Pump Rate:** 23
Water First Found: 19.2 **Primary Water Use:** Domestic **Pumping Duration (h:m):** 1 : 0
Static Level: 2

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	3.35
2	SAND	3.35	9.14
3	HARDPAN	9.14	17.1
4	LIMESTONE	17.1	19.8

Well ID: 1518420 **Easting:** 455430 **UTM Zone** 18
Construction Date: 1983-08-24 **Northing:** 5E+06 **Positional Accuracy:** margin of error : 30 m - 100 m

Well Depth: 19.8 **Water Kind** FRESH **Pump Rate (LPM):** 68
Well Diameter (cm): 15.2 **Final Status** Water Supply **Recommended Pump Rate:** 23
Water First Found: 19.2 **Primary Water Use:** Domestic **Pumping Duration (h:m):** 1 : 0
Static Level: 2

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	1.22
2	SAND	1.22	6.1
3	HARDPAN	6.1	15.2
4	SAND	15.2	16.8

5	LIMESTONE	16.8	19.8

Well ID: 1518698 **Easting:** 455530 **UTM Zone** 18
Construction Date: 1983-11-24 **Northing:** 5E+06 **Positional Accuracy:** margin of error : 30 m - 100 m

Well Depth: 22.9 **Water Kind** FRESH **Pump Rate (LPM):** 45
Well Diameter (cm): 15.2 **Final Status** Water Supply **Recommended Pump Rate:** 23
Water First Found: 20.4 **Primary Water Use:** Domestic **Pumping Duration (h:m):** 1 : 0
Static Level: 4

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	2.44
2	SAND	2.44	11.6
3	SAND	11.6	14.6
4	HARDPAN	14.6	18.3
5	LIMESTONE	18.3	22.9

Well ID: 1520434 **Easting:** 455527 **UTM Zone** 18
Construction Date: 1986-02-20 **Northing:** 5E+06 **Positional Accuracy:** margin of error : 100 m - 300 m

Well Depth: 19.5 **Water Kind** FRESH **Pump Rate (LPM):** 68
Well Diameter (cm): 15.2 **Final Status** Water Supply **Recommended Pump Rate:** 68
Water First Found: 15.9 **Primary Water Use:** Domestic **Pumping Duration (h:m):** 0 : 30
Static Level: 5

Layer:	Driller's Description:	Top:	Bottom:
1	GRAVEL	0	1.83
1	GRAVEL	0	1.83
2	CLAY	1.83	7.32
2	CLAY	1.83	7.32
3	CLAY	7.32	13.4
3	CLAY	7.32	13.4
4	LIMESTONE	13.4	19.5
4	LIMESTONE	13.4	19.5

Well ID: 1522346 **Easting:** 455172 **UTM Zone** 18
Construction Date: 1988-06-21 **Northing:** 5E+06 **Positional Accuracy:** margin of error : 100 m - 300 m

Well Depth: 38.4 **Water Kind** FRESH **Pump Rate (LPM):** 91
Well Diameter (cm): 15.2 **Final Status** Water Supply **Recommended Pump Rate:** 91
Water First Found: 29 **Primary Water Use:** Industrial **Pumping Duration (h:m):** 1 : 30
Static Level: 3

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	2.44
2	SAND	2.44	17.1
3	LIMESTONE	17.1	38.4

Well ID: 1522347 **Easting:** 455239 **UTM Zone** 18
Construction Date: 1988-06-21 **Northing:** 5E+06 **Positional Accuracy:** margin of error : 100 m - 300 m

Well Depth: 18.9 **Water Kind** FRESH **Pump Rate (LPM):** 182
Well Diameter (cm): 15.2 **Final Status** Recharge Well **Recommended Pump Rate:** 2E+
Water First Found: 18.3 **Primary Water Use:** Cooling And A **Pumping Duration (h:m):** 0 : 45
Static Level: 3

Layer:	Driller's Description:	Top:	Bottom:

1	SAND	0	2.74
2	SAND	2.74	17.4
3	LIMESTONE	17.4	18.9

Well ID: 1522348 **Easting:** 455254 **UTM Zone** 18
Construction Date: 1988-06-21 **Northing:** 5E+06 **Positional Accuracy:** margin of error : 100 m - 300 m

Well Depth: 18.9 **Water Kind** FRESH **Pump Rate (LPM):** 182
Well Diameter (cm): 15.2 **Final Status** Recharge Well **Recommended Pump Rate:** 2E+
Water First Found: 18.3 **Primary Water Use:** Cooling And A **Pumping Duration (h:m):** 1 : 0
Static Level: 3

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	2.74
2	SAND	2.74	17.4
3	LIMESTONE	17.4	18.9

Well ID: 1522551 **Easting:** 455474 **UTM Zone** 18
Construction Date: 1988-08-18 **Northing:** 5E+06 **Positional Accuracy:** margin of error : 100 m - 300 m

Well Depth: 19.8 **Water Kind** FRESH **Pump Rate (LPM):** 91
Well Diameter (cm): 15.2 **Final Status** Recharge Well **Recommended Pump Rate:** 45
Water First Found: 15.9 **Primary Water Use:** Cooling And A **Pumping Duration (h:m):** 0 : 45
Static Level: 3

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	2.74
1	SAND	0	2.74
2	TILL	2.74	10.7
2	TILL	2.74	10.7
3	GRAVEL	10.7	14.6
3	GRAVEL	10.7	14.6
4	LIMESTONE	14.6	19.8
4	LIMESTONE	14.6	19.8

Well ID: 1522552 **Easting:** 455484 **UTM Zone** 18
Construction Date: 1988-08-18 **Northing:** 5E+06 **Positional Accuracy:** margin of error : 100 m - 300 m

Well Depth: 19.8 **Water Kind** FRESH **Pump Rate (LPM):** 91
Well Diameter (cm): 15.2 **Final Status** Water Supply **Recommended Pump Rate:** 45
Water First Found: 17.1 **Primary Water Use:** Domestic **Pumping Duration (h:m):** 0 : 45
Static Level: 2

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	2.44
1	SAND	0	2.44
2	TILL	2.44	9.75
2	TILL	2.44	9.75
3	GRAVEL	9.75	14.6
3	GRAVEL	9.75	14.6
4	LIMESTONE	14.6	19.8
4	LIMESTONE	14.6	19.8

Well ID: 1529728
Construction Date: 1997-12-22
Easting: 455273
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 100 m - 300 m

Well Depth: 23.2
Well Diameter (cm): 15.2
Water First Found: 17.1
Static Level: 2
Water Kind Not stated
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 227
Recommended Pump Rate: 23
Pumping Duration (h:m): 1 : 0

Layer:	Driller's Description:	Top:	Bottom:
1	TOPSOIL	0	1.22
2	CLAY	1.22	2.74
3	CLAY	2.74	10.4
4	SAND	10.4	15.5
5	LIMESTONE	15.5	18.9
6	LIMESTONE	18.9	23.2

Well ID: 1532070
Construction Date: 2001-07-17
Easting: 455043
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 10 - 30 m

Well Depth: 18.3
Well Diameter (cm): 15.2
Water First Found: 16.8
Static Level: 4
Water Kind Not stated
Final Status Water Supply
Primary Water Use: Commerical
Pump Rate (LPM): 45
Recommended Pump Rate: 45
Pumping Duration (h:m): 1 : 0

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	1.52
1	SAND	0	1.52
2	CLAY	1.52	11.9
2	CLAY	1.52	11.9
3	COARSE GRAVEL	11.9	18.3
3	COARSE GRAVEL	11.9	18.3

Well ID: 1533428
Construction Date: 2002-12-17
Easting: 455042
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 100 m - 300 m

Well Depth: 68
Well Diameter (cm): 15.2
Water First Found: 65.8
Static Level: 11
Water Kind Not stated
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 45
Recommended Pump Rate: 23
Pumping Duration (h:m): 1 : 0

Layer:	Driller's Description:	Top:	Bottom:
1	TOPSOIL	0	1.22
1	TOPSOIL	0	1.22
2	SAND	1.22	3.66
2	SAND	1.22	3.66
3	CLAY	3.66	9.14
3	CLAY	3.66	9.14
4	SAND	9.14	17.7
4	SAND	9.14	17.7
5	LIMESTONE	17.7	48.8
5	LIMESTONE	17.7	48.8
6	SANDSTONE	48.8	68

6	SANDSTONE	48.8	68

Well ID: 1533469 **Easting:** 455311 **UTM Zone** 18
Construction Date: 2002-12-23 **Northing:** 5E+06 **Positional Accuracy:** margin of error : 100 m - 300 m

Well Depth: 102 **Water Kind** Not stated **Pump Rate (LPM):** 41
Well Diameter (cm): 20.3 **Final Status** Water Supply **Recommended Pump Rate:** 41
Water First Found: 101 **Primary Water Use:** Domestic **Pumping Duration (h:m):** 1 : 0
Static Level: 15

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	18.9
1	SAND	0	18.9
2	LIMESTONE	18.9	57.3
2	LIMESTONE	18.9	57.3
3	LIMESTONE	57.3	69.2
3	LIMESTONE	57.3	69.2
4	SANDSTONE	69.2	102
4	SANDSTONE	69.2	102

Well ID: 1534585 **Easting:** 455214 **UTM Zone** 18
Construction Date: 2004-03-31 **Northing:** 5E+06 **Positional Accuracy:** margin of error : 100 m - 300 m

Well Depth: 41.8 **Water Kind** Not stated **Pump Rate (LPM):** 84
Well Diameter (cm): **Final Status** Test Hole **Recommended Pump Rate:** 36
Water First Found: 41.1 **Primary Water Use:** Not Used **Pumping Duration (h:m):** 6 : 0
Static Level: 3

Layer:	Driller's Description:	Top:	Bottom:
1	CLAY	0	10.1
1	CLAY	0	10.1
2	SANDSTONE	10.1	15.2
2	SANDSTONE	10.1	15.2
3	LIMESTONE	15.2	41.8
3	LIMESTONE	15.2	41.8

Well ID: 1536286 **Easting:** 454797 **UTM Zone** 18
Construction Date: 2006-04-12 **Northing:** 5E+06 **Positional Accuracy:** margin of error : 10 - 30 m

Well Depth: 45.7 **Water Kind** **Pump Rate (LPM):** 91
Well Diameter (cm): **Final Status** Water Supply **Recommended Pump Rate:** 91
Water First Found: 43.2 **Primary Water Use:** Domestic **Pumping Duration (h:m):** 1 :
Static Level: 10

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	12.2
1	SAND	0	12.2
2	LIMESTONE	12.2	45.7
2	LIMESTONE	12.2	45.7

Well ID: 1536661
Construction Date: 2006-09-07
Easting: 454807
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 10 - 30 m

Well Depth: 25
Well Diameter (cm):
Water First Found: 16.8
Static Level: 3
Water Kind
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 91
Recommended Pump Rate: 91
Pumping Duration (h:m): 1 : 0

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	5.18
1	SAND	0	5.18
1	SAND	0	5.18
1	SAND	0	5.18
2	CLAY	5.18	11
2	CLAY	5.18	11
2	CLAY	5.18	11
2	CLAY	5.18	11
3	LIMESTONE	11	25
3	LIMESTONE	11	25
3	LIMESTONE	11	25
3	LIMESTONE	11	25

Well ID: 1536715
Construction Date: 2006-10-11
Easting: 454725
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 10 - 30 m

Well Depth: 56.7
Well Diameter (cm):
Water First Found: 54.3
Static Level: 10
Water Kind
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 91
Recommended Pump Rate: 91
Pumping Duration (h:m): 1 : 0

Layer:	Driller's Description:	Top:	Bottom:
1	CLAY	0	2.74
1	CLAY	0	2.74
2	SAND	2.74	13.1
2	SAND	2.74	13.1
3	LIMESTONE	13.1	46.0
3	LIMESTONE	13.1	46.0
4	SANDSTONE	46.0	56.7
4	SANDSTONE	46.0	56.7

Well ID: 7040754
Construction Date: 2007-02-12
Easting: 454738
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 10 - 30 m

Well Depth: 48.8
Well Diameter (cm):
Water First Found: 19.8
Static Level: 10
Water Kind
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 91
Recommended Pump Rate: 91
Pumping Duration (h:m): 1 : 0

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	12.5
1	SAND	0	12.5
1	SAND	0	12.5

1	SAND	0	12.5
2	LIMESTONE	12.5	45.7
2	LIMESTONE	12.5	45.7
2	LIMESTONE	12.5	45.7
2	LIMESTONE	12.5	45.7
3	SANDSTONE	45.7	48.8
3	SANDSTONE	45.7	48.8
3	SANDSTONE	45.7	48.8
3	SANDSTONE	45.7	48.8

Well ID: 7048698
Construction Date: 2007-08-29
Easting: 454767
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 10 - 30 m

Well Depth: 48.8
Well Diameter (cm):
Water First Found: 45.7
Static Level: 9
Water Kind
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 91
Recommended Pump Rate: 91
Pumping Duration (h:m): 1 : 0

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	12.2
1	SAND	0	12.2
1	SAND	0	12.2
1	SAND	0	12.2
2	LIMESTONE	12.2	43
2	LIMESTONE	12.2	43
2	LIMESTONE	12.2	43
2	LIMESTONE	12.2	43
3	SANDSTONE	43	48.8
3	SANDSTONE	43	48.8
3	SANDSTONE	43	48.8
3	SANDSTONE	43	48.8

Well ID: 7104239
Construction Date: 2008-04-28
Easting: 455341
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 10 - 30 m

Well Depth: 18.9
Well Diameter (cm):
Water First Found:
Static Level:
Water Kind
Final Status Abandoned-Ot
Primary Water Use:
Pump Rate (LPM):
Recommended Pump Rate:
Pumping Duration (h:m):

Layer:	Driller's Description:	Top:	Bottom:
1		0	18.9

Well ID: 7120715
Construction Date: 2009-03-19
Easting: 455600
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 50
Well Diameter (cm):
Water First Found:
Static Level: 4
Water Kind
Final Status
Primary Water Use:
Pump Rate (LPM): 82
Recommended Pump Rate: 46
Pumping Duration (h:m): 1 :

Layer:	Driller's Description:	Top:	Bottom:
1		0	50

Well ID: 7130148
Construction Date: 2009-09-22

Easting: 455051
Northing: 5E+06

UTM Zone 18
Positional Accuracy: margin of error : 10 - 30 m

Well Depth: 4.88
Well Diameter (cm): 5.2
Water First Found:
Static Level:

Water Kind
Final Status Monitoring an
Primary Water Use: Monitoring an

Pump Rate (LPM):
Recommended Pump Rate:
Pumping Duration (h:m):

Layer:	Driller's Description:	Top:	Bottom:
1	GRAVEL	0	0.61
1	GRAVEL	0	0.61
1	GRAVEL	0	0.61
1	GRAVEL	0	0.61
2	SAND	0.61	1.5
2	SAND	0.61	1.5
2	SAND	0.61	1.5
2	SAND	0.61	1.5
3	CLAY	1.5	2.74
3	CLAY	1.5	2.74
3	CLAY	1.5	2.74
3	CLAY	1.5	2.74
4	SILT	2.74	4.88
4	SILT	2.74	4.88
4	SILT	2.74	4.88
4	SILT	2.74	4.88

Well ID: 7156846
Construction Date: 2010-12-29

Easting: 454720
Northing: 5E+06

UTM Zone 18
Positional Accuracy: margin of error : 10 - 30 m

Well Depth: 36.6
Well Diameter (cm): 15.2
Water First Found: 19.8
Static Level: 1

Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic

Pump Rate (LPM): 91
Recommended Pump Rate: 91
Pumping Duration (h:m): 1 :

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	8.53
1	SAND	0	8.53
1	SAND	0	8.53
2	SAND	8.53	16.5
2	SAND	8.53	16.5
2	SAND	8.53	16.5
3	LIMESTONE	16.5	36.6
3	LIMESTONE	16.5	36.6
3	LIMESTONE	16.5	36.6

Well ID: 7157870
Construction Date: 2011-01-17

Easting: 455093
Northing: 5E+06

UTM Zone 18
Positional Accuracy: margin of error : 10 - 30 m

Well Depth: 54.9
Well Diameter (cm): 15.2
Water First Found: 53.0
Static Level: 3

Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic

Pump Rate (LPM): 91
Recommended Pump Rate: 91
Pumping Duration (h:m): 1 : 0

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	17.1
1	SAND	0	17.1
1	SAND	0	17.1
1	SAND	0	17.1
1	SAND	0	17.1
1	SAND	0	17.1
1	SAND	0	17.1
1	SAND	0	17.1
2	LIMESTONE	17.1	54.9
2	LIMESTONE	17.1	54.9
2	LIMESTONE	17.1	54.9
2	LIMESTONE	17.1	54.9
2	LIMESTONE	17.1	54.9
2	LIMESTONE	17.1	54.9
2	LIMESTONE	17.1	54.9
2	LIMESTONE	17.1	54.9

Well ID: 7159015
Construction Date: 2011-02-10

Easting: 455214
Northing: 5E+06

UTM Zone 18
Positional Accuracy: margin of error : 10 - 30 m

Well Depth:
Well Diameter (cm):
Water First Found:
Static Level:

Water Kind
Final Status Abandoned-Ot
Primary Water Use:

Pump Rate (LPM):
Recommended Pump Rate:
Pumping Duration (h:m):

Layer:	Driller's Description:	Top:	Bottom:

Well ID: 7183294
Construction Date: 2012-06-29

Easting: 455487
Northing: 5E+06

UTM Zone 18
Positional Accuracy: margin of error : 100 m - 300 m

Well Depth: 32
Well Diameter (cm): 15.2
Water First Found: 30.2
Static Level: 4

Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic

Pump Rate (LPM): 91
Recommended Pump Rate: 91
Pumping Duration (h:m): 1 :

Layer:	Driller's Description:	Top:	Bottom:
1	CLAY	0	1.83
2	SAND	1.83	12.8
3	LIMESTONE	12.8	30.2
4	LIMESTONE	30.2	32

Well ID: 7183299
Construction Date: 2012-06-29
Easting: 454693
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 61.3
Well Diameter (cm): 15.1
Water First Found: 56.4
Static Level: 5
Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 55
Recommended Pump Rate: 55
Pumping Duration (h:m): 1 :

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	1.52
1	SAND	0	1.52
1	SAND	0	1.52
1	SAND	0	1.52
2	CLAY	1.52	6.40
2	CLAY	1.52	6.40
2	CLAY	1.52	6.40
2	CLAY	1.52	6.40
3	SAND	6.40	18.3
3	SAND	6.40	18.3
3	SAND	6.40	18.3
3	SAND	6.40	18.3
4	LIMESTONE	18.3	34.8
4	LIMESTONE	18.3	34.8
4	LIMESTONE	18.3	34.8
4	LIMESTONE	18.3	34.8
5	SANDSTONE	34.8	54.6
5	SANDSTONE	34.8	54.6
5	SANDSTONE	34.8	54.6
5	SANDSTONE	34.8	54.6
6	SANDSTONE	54.6	56.4
6	SANDSTONE	54.6	56.4
6	SANDSTONE	54.6	56.4
6	SANDSTONE	54.6	56.4
7	SANDSTONE	56.4	61.3
7	SANDSTONE	56.4	61.3
7	SANDSTONE	56.4	61.3
7	SANDSTONE	56.4	61.3

Well ID: 7187406
Construction Date: 2012-09-20
Easting: 455459
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 29.9
Well Diameter (cm): 15.9
Water First Found: 18.3
Static Level: 4
Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 82
Recommended Pump Rate: 46
Pumping Duration (h:m): 1 :

Layer:	Driller's Description:	Top:	Bottom:
1	TOPSOIL	0	2.74

1	TOPSOIL	0	2.74
2	CLAY	2.74	4.87
2	CLAY	2.74	4.87
3	SAND	4.87	9.14
3	SAND	4.87	9.14
4	GRAVEL	9.14	11.3
4	GRAVEL	9.14	11.3
5	LIMESTONE	11.3	29.9
5	LIMESTONE	11.3	29.9

Well ID: 7187693
Construction Date: 2012-09-22
Easting: 455312
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 27.4
Well Diameter (cm): 15.9
Water First Found: 24.7
Static Level: 3
Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 91
Recommended Pump Rate: 91
Pumping Duration (h:m): 1 :

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	11.6
1	SAND	0	11.6
2	LIMESTONE	11.6	24.7
2	LIMESTONE	11.6	24.7
3	LIMESTONE	24.7	27.4
3	LIMESTONE	24.7	27.4

Well ID: 7194027
Construction Date: 2012-12-21
Easting: 455351
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 61
Well Diameter (cm): 15.4
Water First Found: 33.2
Static Level: 9
Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 91
Recommended Pump Rate: 91
Pumping Duration (h:m): 1 : 0

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	15.2
1	SAND	0	15.2
1	SAND	0	15.2
1	SAND	0	15.2
2	LIMESTONE	15.2	33.2
2	LIMESTONE	15.2	33.2
2	LIMESTONE	15.2	33.2
2	LIMESTONE	15.2	33.2
3	LIMESTONE	33.2	52.4
3	LIMESTONE	33.2	52.4
3	LIMESTONE	33.2	52.4
3	LIMESTONE	33.2	52.4
4	SANDSTONE	52.4	58.5
4	SANDSTONE	52.4	58.5

4	SANDSTONE	52.4	58.5
4	SANDSTONE	52.4	58.5
5	SANDSTONE	58.5	61
5	SANDSTONE	58.5	61
5	SANDSTONE	58.5	61
5	SANDSTONE	58.5	61

Well ID: 7197490
Construction Date: 2013-02-19

Easting: 454766
Northing: 5E+06

UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 42.7
Well Diameter (cm): 14.9
Water First Found: 36.3
Static Level: 3

Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic

Pump Rate (LPM): 91
Recommended Pump Rate: 91
Pumping Duration (h:m): 1 :

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	17.4
1	SAND	0	17.4
1	SAND	0	17.4
1	SAND	0	17.4
2	LIMESTONE	17.4	36.3
2	LIMESTONE	17.4	36.3
2	LIMESTONE	17.4	36.3
2	LIMESTONE	17.4	36.3
3	LIMESTONE	36.3	37.5
3	LIMESTONE	36.3	37.5
3	LIMESTONE	36.3	37.5
3	LIMESTONE	36.3	37.5
4	LIMESTONE	37.5	42.7
4	LIMESTONE	37.5	42.7
4	LIMESTONE	37.5	42.7
4	LIMESTONE	37.5	42.7

Well ID: 7200356
Construction Date: 2013-04-15

Easting: 454958
Northing: 5E+06

UTM Zone 18
Positional Accuracy: margin of error : 100 m - 300 m

Well Depth: 61
Well Diameter (cm): 14.9
Water First Found: 46.9
Static Level: 5

Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic

Pump Rate (LPM): 91
Recommended Pump Rate: 91
Pumping Duration (h:m): 1 : 0

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	13.7
1	SAND	0	13.7
1	SAND	0	13.7
1	SAND	0	13.7
2	LIMESTONE	13.7	42.1
2	LIMESTONE	13.7	42.1
2	LIMESTONE	13.7	42.1

2	LIMESTONE	13.7	42.1
3	SANDSTONE	42.1	46.9
3	SANDSTONE	42.1	46.9
3	SANDSTONE	42.1	46.9
3	SANDSTONE	42.1	46.9
4	SANDSTONE	46.9	55.5
4	SANDSTONE	46.9	55.5
4	SANDSTONE	46.9	55.5
4	SANDSTONE	46.9	55.5
5	SANDSTONE	55.5	61
5	SANDSTONE	55.5	61
5	SANDSTONE	55.5	61
5	SANDSTONE	55.5	61

Well ID: 7204662 **Easting:** 455133 **UTM Zone** 18
Construction Date: 2013-07-16 **Northing:** 5E+06 **Positional Accuracy:** margin of error : 30 m - 100 m

Well Depth: 91.4 **Water Kind** Untested **Pump Rate (LPM):** 55
Well Diameter (cm): 15.6 **Final Status** Water Supply **Recommended Pump Rate:** 55
Water First Found: 89 **Primary Water Use:** Domestic **Pumping Duration (h:m):** 1 : 0
Static Level: 10

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	3.35
1	SAND	0	3.35
2	SAND	3.35	10.4
2	SAND	3.35	10.4
3	SAND	10.4	18.3
3	SAND	10.4	18.3
4	LIMESTONE	18.3	38.1
4	LIMESTONE	18.3	38.1
5	SANDSTONE	38.1	41.5
5	SANDSTONE	38.1	41.5
6	LIMESTONE	41.5	49.1
6	LIMESTONE	41.5	49.1
7	SANDSTONE	49.1	89
7	SANDSTONE	49.1	89
8	SANDSTONE	89	91.4
8	SANDSTONE	89	91.4

Well ID: 7204663 **Easting:** 454826 **UTM Zone** 18
Construction Date: 2013-07-16 **Northing:** 5E+06 **Positional Accuracy:** margin of error : 30 m - 100 m

Well Depth: 61 **Water Kind** Untested **Pump Rate (LPM):** 55
Well Diameter (cm): 15.6 **Final Status** Water Supply **Recommended Pump Rate:** 55
Water First Found: 48.2 **Primary Water Use:** Domestic **Pumping Duration (h:m):** 1 : 0
Static Level: 10

Layer:	Driller's Description:	Top:	Bottom:

1	SAND	0	4.27
1	SAND	0	4.27
1	SAND	0	4.27
1	SAND	0	4.27
2	SILT	4.27	11.6
2	SILT	4.27	11.6
2	SILT	4.27	11.6
2	SILT	4.27	11.6
3	SAND	11.6	14.3
3	SAND	11.6	14.3
3	SAND	11.6	14.3
3	SAND	11.6	14.3
4	LIMESTONE	14.3	40.2
4	LIMESTONE	14.3	40.2
4	LIMESTONE	14.3	40.2
4	LIMESTONE	14.3	40.2
5	LIMESTONE	40.2	48.2
5	LIMESTONE	40.2	48.2
5	LIMESTONE	40.2	48.2
5	LIMESTONE	40.2	48.2
6	LIMESTONE	48.2	57.6
6	LIMESTONE	48.2	57.6
6	LIMESTONE	48.2	57.6
6	LIMESTONE	48.2	57.6
7	LIMESTONE	57.6	61
7	LIMESTONE	57.6	61
7	LIMESTONE	57.6	61
7	LIMESTONE	57.6	61

Well ID: 7209271
Construction Date: 2013-10-10

Easting: 454896
Northing: 5E+06

UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 54.9
Well Diameter (cm): 15.6
Water First Found: 21.0
Static Level: 4

Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic

Pump Rate (LPM): 91
Recommended Pump Rate: 91
Pumping Duration (h:m): 1 : 0

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	6.1
1	SAND	0	6.1
1	SAND	0	6.1
1	SAND	0	6.1
1	SAND	0	6.1
1	SAND	0	6.1
2	SAND	6.1	14.6

1	SAND	0	4.27
1	SAND	0	4.27
1	SAND	0	4.27
1	SAND	0	4.27
2	SILT	4.27	11.6
2	SILT	4.27	11.6
2	SILT	4.27	11.6
2	SILT	4.27	11.6
3	SAND	11.6	14.3
3	SAND	11.6	14.3
3	SAND	11.6	14.3
3	SAND	11.6	14.3
4	LIMESTONE	14.3	40.2
4	LIMESTONE	14.3	40.2
4	LIMESTONE	14.3	40.2
4	LIMESTONE	14.3	40.2
5	LIMESTONE	40.2	48.2
5	LIMESTONE	40.2	48.2
5	LIMESTONE	40.2	48.2
5	LIMESTONE	40.2	48.2
6	LIMESTONE	48.2	57.6
6	LIMESTONE	48.2	57.6
6	LIMESTONE	48.2	57.6
6	LIMESTONE	48.2	57.6
7	LIMESTONE	57.6	61
7	LIMESTONE	57.6	61
7	LIMESTONE	57.6	61
7	LIMESTONE	57.6	61

Well ID: 7209271
Construction Date: 2013-10-10

Easting: 454896
Northing: 5E+06

UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 54.9
Well Diameter (cm): 15.6
Water First Found: 21.0
Static Level: 4

Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic

Pump Rate (LPM): 91
Recommended Pump Rate: 91
Pumping Duration (h:m): 1 : 0

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	6.1
1	SAND	0	6.1
1	SAND	0	6.1
1	SAND	0	6.1
1	SAND	0	6.1
1	SAND	0	6.1
2	SAND	6.1	14.6

2	SAND	6.1	14.6
2	SAND	6.1	14.6
2	SAND	6.1	14.6
2	SAND	6.1	14.6
2	SAND	6.1	14.6
3	LIMESTONE	14.6	21.0
3	LIMESTONE	14.6	21.0
3	LIMESTONE	14.6	21.0
3	LIMESTONE	14.6	21.0
3	LIMESTONE	14.6	21.0
3	LIMESTONE	14.6	21.0
4	LIMESTONE	21.0	42.4
4	LIMESTONE	21.0	42.4
4	LIMESTONE	21.0	42.4
4	LIMESTONE	21.0	42.4
4	LIMESTONE	21.0	42.4
4	LIMESTONE	21.0	42.4
5	SANDSTONE	42.4	44.8
5	SANDSTONE	42.4	44.8
5	SANDSTONE	42.4	44.8
5	SANDSTONE	42.4	44.8
5	SANDSTONE	42.4	44.8
5	SANDSTONE	42.4	44.8
6	SANDSTONE	44.8	51.8
6	SANDSTONE	44.8	51.8
6	SANDSTONE	44.8	51.8
6	SANDSTONE	44.8	51.8
6	SANDSTONE	44.8	51.8
6	SANDSTONE	44.8	51.8
7	SANDSTONE	51.8	54.9
7	SANDSTONE	51.8	54.9
7	SANDSTONE	51.8	54.9
7	SANDSTONE	51.8	54.9
7	SANDSTONE	51.8	54.9
7	SANDSTONE	51.8	54.9

Well ID: 7217217
Construction Date: 2014-03-03

Easting: 455459
Northing: 5E+06

UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 32.3
Well Diameter (cm): 15.2
Water First Found: 30.2
Static Level: 6

Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic

Pump Rate (LPM): 91
Recommended Pump Rate: 91
Pumping Duration (h:m): 1 :

Layer:	Driller's Description:	Top:	Bottom:

1	SAND	0	14.3
1	SAND	0	14.3
2	LIMESTONE	14.3	30.2
2	LIMESTONE	14.3	30.2
3	LIMESTONE	30.2	32.3
3	LIMESTONE	30.2	32.3

Well ID: 7228009 **Easting:** 455435 **UTM Zone** 18
Construction Date: 2014-09-22 **Northing:** 5E+06 **Positional Accuracy:** margin of error : 30 m - 100 m

Well Depth: 61 **Water Kind** Untested **Pump Rate (LPM):** 91
Well Diameter (cm): 15.1 **Final Status** Water Supply **Recommended Pump Rate:** 91
Water First Found: 26.2 **Primary Water Use:** Domestic **Pumping Duration (h:m):** 1 :
Static Level: 8

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	15.2
1	SAND	0	15.2
1	SAND	0	15.2
1	SAND	0	15.2
1	SAND	0	15.2
1	SAND	0	15.2
2	LIMESTONE	15.2	26.2
2	LIMESTONE	15.2	26.2
2	LIMESTONE	15.2	26.2
2	LIMESTONE	15.2	26.2
2	LIMESTONE	15.2	26.2
2	LIMESTONE	15.2	26.2
3	LIMESTONE	26.2	40.8
3	LIMESTONE	26.2	40.8
3	LIMESTONE	26.2	40.8
3	LIMESTONE	26.2	40.8
3	LIMESTONE	26.2	40.8
3	LIMESTONE	26.2	40.8
4	LIMESTONE	40.8	54.9
4	LIMESTONE	40.8	54.9
4	LIMESTONE	40.8	54.9
4	LIMESTONE	40.8	54.9
4	LIMESTONE	40.8	54.9
4	LIMESTONE	40.8	54.9
5	SANDSTONE	54.9	59.1
5	SANDSTONE	54.9	59.1
5	SANDSTONE	54.9	59.1
5	SANDSTONE	54.9	59.1
5	SANDSTONE	54.9	59.1

5	SANDSTONE	54.9	59.1
6	SANDSTONE	59.1	61
6	SANDSTONE	59.1	61
6	SANDSTONE	59.1	61
6	SANDSTONE	59.1	61
6	SANDSTONE	59.1	61
6	SANDSTONE	59.1	61

Well ID: 7230319
Construction Date: 2014-10-29
Easting: 455162
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 90.5
Well Diameter (cm): 15.2
Water First Found: 88.7
Static Level: 10
Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 55
Recommended Pump Rate: 55
Pumping Duration (h:m): 1 :

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	9.14
1	SAND	0	9.14
2	GRAVEL	9.14	17.7
2	GRAVEL	9.14	17.7
3	LIMESTONE	17.7	43
3	LIMESTONE	17.7	43
4	LIMESTONE	43	48.2
4	LIMESTONE	43	48.2
5	SANDSTONE	48.2	88.7
5	SANDSTONE	48.2	88.7
6	SANDSTONE	88.7	90.5
6	SANDSTONE	88.7	90.5

Well ID: 7240506
Construction Date: 2015-04-24
Easting: 455080
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 61
Well Diameter (cm): 15.1
Water First Found: 41.8
Static Level: 4
Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 36
Recommended Pump Rate: 36
Pumping Duration (h:m): 1 : 0

Layer:	Driller's Description:	Top:	Bottom:
1	CLAY	0	16.8
1	CLAY	0	16.8
1	CLAY	0	16.8
1	CLAY	0	16.8
2	LIMESTONE	16.8	41.8
2	LIMESTONE	16.8	41.8
2	LIMESTONE	16.8	41.8
2	LIMESTONE	16.8	41.8
3	LIMESTONE	41.8	51.8
3	LIMESTONE	41.8	51.8

3	LIMESTONE	41.8	51.8
3	LIMESTONE	41.8	51.8
4	SANDSTONE	51.8	59.1
4	SANDSTONE	51.8	59.1
4	SANDSTONE	51.8	59.1
4	SANDSTONE	51.8	59.1
5	SANDSTONE	59.1	61
5	SANDSTONE	59.1	61
5	SANDSTONE	59.1	61
5	SANDSTONE	59.1	61

Well ID: 7243021
Construction Date: 2015-06-15
Easting: 455306
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 54.9
Well Diameter (cm): 15.2
Water First Found: 22.9
Static Level: 2
Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 91
Recommended Pump Rate: 91
Pumping Duration (h:m): 1 :

Layer:	Driller's Description:	Top:	Bottom:
1	CLAY	0	14.9
1	CLAY	0	14.9
1	CLAY	0	14.9
1	CLAY	0	14.9
2	LIMESTONE	14.9	22.9
2	LIMESTONE	14.9	22.9
2	LIMESTONE	14.9	22.9
2	LIMESTONE	14.9	22.9
3	LIMESTONE	22.9	52.4
3	LIMESTONE	22.9	52.4
3	LIMESTONE	22.9	52.4
3	LIMESTONE	22.9	52.4
4	LIMESTONE	52.4	54.9
4	LIMESTONE	52.4	54.9
4	LIMESTONE	52.4	54.9
4	LIMESTONE	52.4	54.9

Well ID: 7243032
Construction Date: 2015-06-15
Easting: 455258
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 48.8
Well Diameter (cm): 15.9
Water First Found: 46.9
Static Level: 3
Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 68
Recommended Pump Rate: 68
Pumping Duration (h:m): : 10

Layer:	Driller's Description:	Top:	Bottom:
1	CLAY	0	15.9
1	CLAY	0	15.9
1	CLAY	0	15.9

1	CLAY	0	15.9
2	LIMESTONE	15.9	26.2
2	LIMESTONE	15.9	26.2
2	LIMESTONE	15.9	26.2
2	LIMESTONE	15.9	26.2
3	LIMESTONE	26.2	46.9
3	LIMESTONE	26.2	46.9
3	LIMESTONE	26.2	46.9
3	LIMESTONE	26.2	46.9
4	LIMESTONE	46.9	48.8
4	LIMESTONE	46.9	48.8
4	LIMESTONE	46.9	48.8
4	LIMESTONE	46.9	48.8

Well ID: 7243033
Construction Date: 2015-06-15
Easting: 455335
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 65.5
Well Diameter (cm): 15.2
Water First Found: 57.9
Static Level: 9
Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 91
Recommended Pump Rate: 91
Pumping Duration (h:m): 1 :

Layer:	Driller's Description:	Top:	Bottom:
1	CLAY	0	14.6
1	CLAY	0	14.6
1	CLAY	0	14.6
1	CLAY	0	14.6
2	LIMESTONE	14.6	48.8
2	LIMESTONE	14.6	48.8
2	LIMESTONE	14.6	48.8
2	LIMESTONE	14.6	48.8
3	LIMESTONE	48.8	57.9
3	LIMESTONE	48.8	57.9
3	LIMESTONE	48.8	57.9
3	LIMESTONE	48.8	57.9
4	LIMESTONE	57.9	63.4
4	LIMESTONE	57.9	63.4
4	LIMESTONE	57.9	63.4
4	LIMESTONE	57.9	63.4
5	LIMESTONE	63.4	65.5
5	LIMESTONE	63.4	65.5
5	LIMESTONE	63.4	65.5
5	LIMESTONE	63.4	65.5

Well ID: 7252399
Construction Date: 2015-11-17
Easting: 455519
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 25
Well Diameter (cm): 15.2
Water First Found: 17.7
Static Level: 3
Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 91
Recommended Pump Rate: 91
Pumping Duration (h:m): 1 : 0

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	9.14
1	SAND	0	9.14
1	SAND	0	9.14
1	SAND	0	9.14
2	LIMESTONE	9.14	17.7
2	LIMESTONE	9.14	17.7
2	LIMESTONE	9.14	17.7
2	LIMESTONE	9.14	17.7
3	LIMESTONE	17.7	22.9
3	LIMESTONE	17.7	22.9
3	LIMESTONE	17.7	22.9
3	LIMESTONE	17.7	22.9
4	LIMESTONE	22.9	25
4	LIMESTONE	22.9	25
4	LIMESTONE	22.9	25
4	LIMESTONE	22.9	25

Well ID: 7252400
Construction Date: 2015-11-17
Easting: 455399
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 48.8
Well Diameter (cm): 15.9
Water First Found: 44.2
Static Level: 2
Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 91
Recommended Pump Rate: 91
Pumping Duration (h:m): 1 :

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	8.84
1	SAND	0	8.84
1	SAND	0	8.84
1	SAND	0	8.84
2	LIMESTONE	8.84	44.2
2	LIMESTONE	8.84	44.2
2	LIMESTONE	8.84	44.2
2	LIMESTONE	8.84	44.2
3	LIMESTONE	44.2	46.9
3	LIMESTONE	44.2	46.9
3	LIMESTONE	44.2	46.9
3	LIMESTONE	44.2	46.9
4	LIMESTONE	46.9	48.8

4	LIMESTONE	46.9	48.8
4	LIMESTONE	46.9	48.8
4	LIMESTONE	46.9	48.8

Well ID: 7255451
Construction Date: 2016-01-06
Easting: 455289
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 64.0
Well Diameter (cm): 15.6
Water First Found: 62.5
Static Level: 8
Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 91
Recommended Pump Rate: 91
Pumping Duration (h:m): 1 : 0

Layer:	Driller's Description:	Top:	Bottom:
1	CLAY	0	15.9
1	CLAY	0	15.9
1	CLAY	0	15.9
1	CLAY	0	15.9
2	LIMESTONE	15.9	48.8
2	LIMESTONE	15.9	48.8
2	LIMESTONE	15.9	48.8
2	LIMESTONE	15.9	48.8
3	LIMESTONE	48.8	49.1
3	LIMESTONE	48.8	49.1
3	LIMESTONE	48.8	49.1
3	LIMESTONE	48.8	49.1
4	LIMESTONE	49.1	62.5
4	LIMESTONE	49.1	62.5
4	LIMESTONE	49.1	62.5
4	LIMESTONE	49.1	62.5
5	LIMESTONE	62.5	64.0
5	LIMESTONE	62.5	64.0
5	LIMESTONE	62.5	64.0
5	LIMESTONE	62.5	64.0

Well ID: 7265398
Construction Date: 2016-06-21
Easting: 455315
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 73.2
Well Diameter (cm): 15.9
Water First Found: 70.7
Static Level: 8
Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 91
Recommended Pump Rate: 91
Pumping Duration (h:m): 1 : 0

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	15.9
1	SAND	0	15.9
1	SAND	0	15.9
1	SAND	0	15.9
1	SAND	0	15.9
1	SAND	0	15.9

2	LIMESTONE	15.9	48.8
2	LIMESTONE	15.9	48.8
2	LIMESTONE	15.9	48.8
2	LIMESTONE	15.9	48.8
2	LIMESTONE	15.9	48.8
2	LIMESTONE	15.9	48.8
3	SANDSTONE	48.8	55.5
3	SANDSTONE	48.8	55.5
3	SANDSTONE	48.8	55.5
3	SANDSTONE	48.8	55.5
3	SANDSTONE	48.8	55.5
3	SANDSTONE	48.8	55.5
4	SANDSTONE	55.5	59.1
4	SANDSTONE	55.5	59.1
4	SANDSTONE	55.5	59.1
4	SANDSTONE	55.5	59.1
4	SANDSTONE	55.5	59.1
4	SANDSTONE	55.5	59.1
5	SANDSTONE	59.1	70.7
5	SANDSTONE	59.1	70.7
5	SANDSTONE	59.1	70.7
5	SANDSTONE	59.1	70.7
5	SANDSTONE	59.1	70.7
5	SANDSTONE	59.1	70.7
6	SANDSTONE	70.7	73.2
6	SANDSTONE	70.7	73.2
6	SANDSTONE	70.7	73.2
6	SANDSTONE	70.7	73.2
6	SANDSTONE	70.7	73.2
6	SANDSTONE	70.7	73.2

Well ID: 7296379
Construction Date: 2017-10-03

Easting: 454770
Northing: 5E+06

UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 67.1
Well Diameter (cm): 15.9
Water First Found: 64.9
Static Level: 7

Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic

Pump Rate (LPM): 91
Recommended Pump Rate: 91
Pumping Duration (h:m): 1 : 0

Layer:	Driller's Description:	Top:	Bottom:
1	CLAY	0	3.05
1	CLAY	0	3.05
1	CLAY	0	3.05
1	CLAY	0	3.05
2	GRAVEL	3.05	17.7

2	GRAVEL	3.05	17.7
2	GRAVEL	3.05	17.7
2	GRAVEL	3.05	17.7
3	LIMESTONE	17.7	46.0
3	LIMESTONE	17.7	46.0
3	LIMESTONE	17.7	46.0
3	LIMESTONE	17.7	46.0
4	SANDSTONE	46.0	63.7
4	SANDSTONE	46.0	63.7
4	SANDSTONE	46.0	63.7
4	SANDSTONE	46.0	63.7
5	SANDSTONE	63.7	64.9
5	SANDSTONE	63.7	64.9
5	SANDSTONE	63.7	64.9
5	SANDSTONE	63.7	64.9
6	SANDSTONE	64.9	67.1
6	SANDSTONE	64.9	67.1
6	SANDSTONE	64.9	67.1
6	SANDSTONE	64.9	67.1

Well ID: 7301342
Construction Date: 2017-12-14
Easting: 455482
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 36.6
Well Diameter (cm): 15.9
Water First Found: 35.4
Static Level: 3
Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 91
Recommended Pump Rate: 91
Pumping Duration (h:m): 1 : 0

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	10.1
1	SAND	0	10.1
1	SAND	0	10.1
1	SAND	0	10.1
2	LIMESTONE	10.1	36.6
2	LIMESTONE	10.1	36.6
2	LIMESTONE	10.1	36.6
2	LIMESTONE	10.1	36.6

Well ID: 7318099
Construction Date: 2018-09-10
Easting: 455258
Northing: 5E+06
UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 61
Well Diameter (cm): 15.2
Water First Found: 57
Static Level: 8
Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic
Pump Rate (LPM): 91
Recommended Pump Rate: 91
Pumping Duration (h:m): 1 :

Layer:	Driller's Description:	Top:	Bottom:
1	CLAY	0	4.27
1	CLAY	0	4.27

2	SAND	4.27	15.2
2	SAND	4.27	15.2
3	LIMESTONE	15.2	41.8
3	LIMESTONE	15.2	41.8
4	SANDSTONE	41.8	45.7
4	SANDSTONE	41.8	45.7
5	SANDSTONE	45.7	57
5	SANDSTONE	45.7	57
6	SANDSTONE	57	61
6	SANDSTONE	57	61

Well ID: 7324334
Construction Date: 2018-12-11

Easting: 455498
Northing: 5E+06

UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 18.3
Well Diameter (cm): 15.9
Water First Found: 15.2
Static Level: 5

Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic

Pump Rate (LPM): 46
Recommended Pump Rate: 46
Pumping Duration (h:m): 1 :

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	7.31
1	SAND	0	7.31
2	SAND	7.31	11.3
2	SAND	7.31	11.3
3	LIMESTONE	11.3	15.2
3	LIMESTONE	11.3	15.2
4	LIMESTONE	15.2	18.3
4	LIMESTONE	15.2	18.3

Well ID: 7336839
Construction Date: 2019-07-10

Easting: 455307
Northing: 5E+06

UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 25
Well Diameter (cm): 15.9
Water First Found: 22
Static Level: 2

Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic

Pump Rate (LPM): 91
Recommended Pump Rate: 91
Pumping Duration (h:m): 1 : 0

Layer:	Driller's Description:	Top:	Bottom:
1	SAND	0	12.5
1	SAND	0	12.5
2	LIMESTONE	12.5	22
2	LIMESTONE	12.5	22
3	LIMESTONE	22	25
3	LIMESTONE	22	25

Well ID: 7341123
Construction Date: 2019-09-06

Easting: 455360
Northing: 5E+06

UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth:
Well Diameter (cm):
Water First Found:
Static Level:

Water Kind
Final Status Water Supply
Primary Water Use:

Pump Rate (LPM):
Recommended Pump Rate:
Pumping Duration (h:m): :

Layer:	Driller's Description:	Top:	Bottom:

Well ID: 7357357
Construction Date: 2020-04-28

Easting: 455292
Northing: 5E+06

UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth: 24.7
Well Diameter (cm): 15.6
Water First Found: 22.9
Static Level: 3

Water Kind Untested
Final Status Water Supply
Primary Water Use: Domestic

Pump Rate (LPM): 91
Recommended Pump Rate: 91
Pumping Duration (h:m): 1 :

Layer:	Driller's Description:	Top:	Bottom:
1	CLAY	0	15.2
1	CLAY	0	15.2
2	LIMESTONE	15.2	24.7
2	LIMESTONE	15.2	24.7

Well ID: 7364564
Construction Date: 2020-08-13

Easting: 455536
Northing: 5E+06

UTM Zone 18
Positional Accuracy: margin of error : 30 m - 100 m

Well Depth:
Well Diameter (cm):
Water First Found:
Static Level:

Water Kind
Final Status
Primary Water Use:

Pump Rate (LPM):
Recommended Pump Rate:
Pumping Duration (h:m):

Layer:	Driller's Description:	Top:	Bottom:

Well Use Survey Summary Report

Number	Street	Spoke to employee	Participated in program	In-person survey results	Paper survey results
1368	Greely Lane	Yes	No	Gave letter to frontdesk and they will pass it along to the owner	Survey not returned
1375	Greely Lane	Yes	No	Gave letter to frontdesk and they will pass it along to the owner	Returned survey indicates the well is approx 40ft deep and installed 31 years ago. No water quality or quanity issues are noted.
1380	Greely Lane	No	No	Left letter in mailbox	Returned survey indicates water is obtained from a well constructed in approx. 1989, and is used for shower, septic, and vehicle washing. A sulfur smell is noted for water quality.
6906	McKeown Drive	Yes	No	Gave letter to frontdesk and they will pass it along to the owner	Survey not returned
6876	McKeown Drive	Yes	No	Gave letter to frontdesk and they will pass it along to the owner	Survey not returned

CERTIFICATE OF WELL COMPLIANCE

I , Jeremy Hanna (License T3632), **AIR ROCK DRILLING CO. LTD.,** DO HEREBY CERTIFIY, that I am licensed to drill water wells in the Province of Ontario, and that I have supervised the drilling of a well on the

PROPERTY OF: DANDEX DEVELOPMENTS INC.

LOCATED AT : # 1386 GREELY LANE Greely

LOT # P/L 4&5 **CON #** 4 **PLAN #** 4M-351 **S/L #** Part Block 3

Geographical Township OSGOODE

of OTTAWA - CARLETON

I CERTIFY FURTHER that, I am aware of the well drilling requirements, the guidelines, recommendations and regulations of the Ministry of the Environment governing well installations in the Province of Ontario, and the standards specified in any subdivision agreement and hydrogeological report applicable to this site and City Standards.

AND DO HEREBY CERTIFY THAT the said well has been drilled, cased, grouted (cement or bentonite) as applicable and constructed in strict conformity with the standards required.

Signed this 21 ST day of MAY 2025,

Jeremy Hanna (T3632) Air Rock Drilling Co. Ltd. (C-7681)

The Engineer / Hydrologist on behalf of the Landowner set out above Certifies that He/She has Inspected the well and it was constructed in accordance with the specifications In Ministry of Environment Regulation 903

Signed this 11th day of June, 2025

Signed by:

A84A949C3B4C4B4...

HYDROLOGIST / ENGINEER
(Signature / STAMP)

Signed by:

PROFESSIONAL GEOSCIENTIST
KYLE N. HORNER
PRACTISING MEMBER
3066
ONTARIO

2025-06-11

2025225

TAG A 379053

Cassidy EW Construction

Ontario Ministry of the Environment, Conservation and Parks

Tag#:A379053 (Print Below)
A379053

Well Record

Regulation 903 Ontario Water Resources Act

Page ____ of ____

Measurements recorded in: ☐ Metric ☒ Imperial

Well Owner's Information

First Name	Last Name/Organization	E-mail Address	☐ Well Constructed by Well Owner
	Cassidy EW Construction		

Mailing Address (Street Number/Name)	Municipality	Province	Postal Code	Telephone No. (inc. area code)
1-1011 Thomas Spratt Place	Ottawa	ON	K1G 5L5	

Well Location

Address of Well Location (Street Number/Name)	Township	Lot	Concession
1386 Greely Lane	Osgoode	P/L 4&5	

County/District/Municipality	City/Town/Village	Province	Postal Code
Ottawa Carleton	Greely	Ontario	

UTM Coordinates	Zone	Easting	Northing	Municipal Plan and Sublot Number	Other
NAD 8 3	18	455205	5011824	4M-351	Part Block 3

Overburden and Bedrock Materials/Abandonment Sealing Record (see instructions on the back of this form)

General Colour	Most Common Material	Other Materials	General Description	Depth (m/ft) From	Depth (m/ft) To
	Clay			0	38
	Boulders	& Hard Pan		38	48
Grey & Black	Limestone			48	74
Grey & Black	Limestone			74	125
Grey & Black	Limestone			125	174
Grey & Black	Limestone			174	180

DAN DAX DEVELOPMENTS INC.

Annular Space

Depth Set at (m/ft) From	To	Type of Sealant Used (Material and Type)	Volume Placed (m³/ft³)
54	44	Neat cement	10.92
44	0	Bentonite slurry	25.20

Method of Construction

☐ Cable Tool ☐ Diamond
☐ Rotary (Conventional) ☐ Jetting
☐ Rotary (Reverse) ☐ Driving
☐ Boring ☐ Digging
☒ Air percussion
☐ Other, specify

Well Use

☐ Public ☐ Commercial ☐ Not used
☒ Domestic ☐ Municipal ☐ Dewatering
☐ Livestock ☐ Test Hole ☐ Monitoring
☐ Irrigation ☐ Cooling & Air Conditioning
☐ Industrial
☐ Other, specify

Construction Record - Casing

Inside Diameter (cm/in)	Open Hole OR Material (Galvanized, Fibreglass, Concrete, Plastic, Steel)	Wall Thickness (cm/in)	Depth (m/ft) From	Depth (m/ft) To
6 1/4"	Steel	.188"	+2	54
6 1/8"	Open Hole		54	180

Construction Record - Screen

Outside Diameter (cm/in)	Material (Plastic, Galvanized, Steel)	Slot No.	Depth (m/ft) From	Depth (m/ft) To

Status of Well

☒ Water Supply
☐ Replacement Well
☐ Test Hole
☐ Recharge Well
☐ Dewatering Well
☐ Observation and/or Monitoring Hole
☐ Alteration (Construction)
☐ Abandoned, Insufficient Supply
☐ Abandoned, Poor Water Quality
☐ Abandoned, other, specify
☐ Other, specify

Water Details

Water found at Depth (m/ft)	Kind of Water	Gas	Other, specify
74	☐ Fresh ☐ Untested	☐ Gas	
125	☐ Fresh ☐ Untested	☐ Gas	
174	☐ Fresh ☐ Untested	☐ Gas	

Hole Diameter

Depth (m/ft) From	To	Diameter (cm/in)
0	54	6 1/4"
54	180	6 1/8"

Results of Well Yield Testing

After test of well yield, water was:
☐ Clear and sand free
☐ Other, specify **Not tested**

If pumping discontinued, give reason:

Pump intake set at (m/ft): 170

Pumping rate (l/min / GPM): 20 +

Duration of pumping: 1 hrs + 0 min

Final water level end of pumping (m/ft): 8.6

If flowing give rate (l/min/GPM): X

Recommended pump depth (m/ft): 100

Recommended pump rate (l/min/GPM): 15

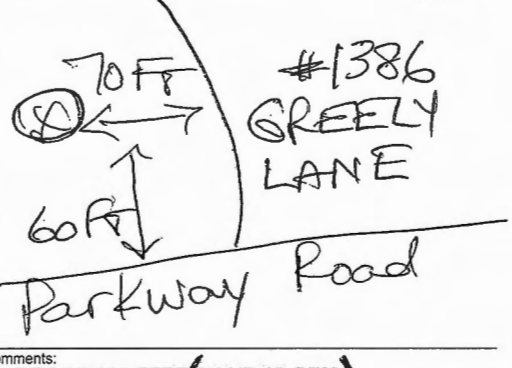
Well production (l/min/GPM): 20

Disinfected? ☒ Yes ☐ No

Draw Down Time (min)	Draw Down Water Level (m/ft)	Recovery Time (min)	Recovery Water Level (m/ft)
Static Level	7.1		8.6
1	8.3	1	7.9
2	8.3	2	7.7
3	8.4	3	7.7
4	8.4	4	7.7
5	8.4	5	7.7
10	8.5	10	7.7
15	8.5	15	7.7
20	8.6	20	7.7
25	8.6	25	7.7
30	8.6	30	7.7
40	8.6	40	7.7
50	8.6	50	7.7
60	8.6	60	7.7

Map of Well Location

Please provide a map below following instructions on the back.



Comments: SET AT 100 FEET 3/4 HP 15 GPM

Well Contractor and Well Technician Information

Business Name of Well Contractor	Well Contractor's Licence No.
Air Rock Drilling Co. Ltd.	7681

Business Address (Street Number/Name)	Municipality	Province	Postal Code	Business E-mail Address
6059 Franktown Road	Richmond	ON	K0A 2Z0	air-rock@sympatico.ca

Bus. Telephone No. (inc. area code)	Name of Well Technician (Last Name, First Name)
6138382170	Gauthier, Chris

Well Technician's Licence No.	Signature of Technician and/or Contractor	Date Submitted (Y Y Y Y M M D D)
T 5044		2025 06 30

Well owner's information package delivered	Date Package Delivered (Y Y Y Y M M D D)
☒ Yes ☐ No	2025 05 27

Ministry Use Only
Audit No. Z427114
Received 2025 05 27

Appendix E

Groundwater Quality Lab Results

CADUCEON ENVIRONMENTAL LABORATORIES
Client committed. Quality assured. Canadian owned.

CERTIFICATE OF ANALYSIS

Final Report

C.O.C.: G 107579 **REPORT No: 24-010898 - Rev. 1**

Report To:
Cambium Environmental - Kingston
625 Fortune Crescent
#1
Kingston, ON K7P 0L5

CADUCEON Environmental Laboratories
2378 Holly Lane
Ottawa, ON K1V 7P1

Attention: Kyle Horner

DATE RECEIVED: 2024-Apr-22
DATE REPORTED: 2024-Jul-30
SAMPLE MATRIX: Ground Water

CUSTOMER PROJECT: 17280-002
P.O. NUMBER:

Analyses	Qty	Site Analyzed	Authorized	Date Analyzed	Lab Method	Reference Method
Anions (Liquid)	1	OTTAWA	PCURIEL	2024-Apr-24	A-IC-01	SM 4110B
BOD5 (Liquid)	1	KINGSTON	JYEARWOOD	2024-Apr-24	BOD-001	SM 5210B
Cond/pH/Alk Auto (Liquid)	1	OTTAWA	SBOUDREAU	2024-Apr-22	COND-02/PH-02/A LK-02	SM 2510B/4500H/ 2320B
Cyanide Total (Liquid)	1	KINGSTON	JMACINNES	2024-Apr-23	CN-001	SM 4500-CN-E
Formaldehyde (Subcontracted)	1	TESTMARK	SISLAM	2024-Apr-26		Subcontracted
Ion Balance (Calc.)	1	OTTAWA	ASCHNEIDER		CP-028	MECP E3196
Chromium VI (Liquid)	1	OTTAWA	STAILLON	2024-Apr-25	D-CRVI-01	MECP E3056
ICP/MS Total (Liquid)	1	OTTAWA	AOZKAYMAK	2024-Apr-24	D-ICPMS-01	EPA 6020
ICP/OES Total (Liquid)	1	OTTAWA	APRUDYVUS	2024-Apr-29	D-ICP-01	SM 3120B
ICP/OES (Liquid)	1	OTTAWA	APRUDYVUS	2024-Apr-24	D-ICP-01	SM 3120B
Mercury (Liquid)	1	OTTAWA	TBENNETT	2024-Apr-24	D-HG-02	SM 3112B
NDMA Liquid (Subcontract)	1	SGS_LAKEFIELD	SISLAM	2024-May-30		Subcontracted
Ammonia (Liquid)	1	KINGSTON	JYEARWOOD	2024-Apr-24	NH3-001	SM 4500NH3
Nonylphenols (Subcontracted)	1	SGS_LAKEFIELD	SISLAM	2024-Apr-30		Subcontracted
OC Pesticides (Liquid)	1	KINGSTON	CSUMMERHAYS	2024-Apr-23	PESTCL-001	EPA 8081
Oil & Grease (Liquid)	1	KINGSTON	MLANE	2024-Apr-25	O&G-001	SM 5520
Phenols (Liquid)	1	KINGSTON	JMACINNES	2024-Apr-25	PHEN-01	MECP E3179
Sulphide (Liquid)	1	KINGSTON	EHINCH	2024-Apr-23	H2S-001	SM 4500-S2
SVOC - Semi-Volatiles (Liquid)	1	KINGSTON	EASIEDU	2024-Apr-24	NAB-W-001	EPA 8270D
TP & TKN (Liquid)	1	KINGSTON	KDIBBITS	2024-Apr-29	TPTKN-001	MECP E3516.2
TSS (Liquid)	1	KINGSTON	MCLOSS	2024-Apr-23	TSS-001	SM 2540D
Turbidity (Liquid)	1	OTTAWA	STAILLON	2024-Apr-23	A-TURB-01	SM 2130B
VOC-Volatiles Full (Water)	1	RICHMOND_HILL	FLENA	2024-Apr-24	C-VOC-02	EPA 8260

R.L. = Reporting Limit
NC = Not Calculated
Test methods may be modified from specified reference method unless indicated by an *

Michelle Dubien
Data Specialist

				Client I.D.	BH106
				Sample I.D.	24-010898-1
				Date Collected	2024-Apr-19
Parameter	**Units**	**R.L.**	**Limits**		-
Alkalinity(CaCO3) to pH4.5	mg/L	5			283
pH @25°C	pH units	-	11.0, 9.0	SAN, STORM	7.85
Turbidity	NTU	0.1			7070
Fluoride	mg/L	0.1	10	SAN	<0.1
Sulphate	mg/L	1	1500	SAN	84
BOD5	mg/L	3	300, 25.0	SAN, STORM	3
Total Suspended Solids	mg/L	3	350, 15.0	SAN, STORM	**9480**
Phosphorus (Total)	mg/L	0.01	10, 0.4	SAN, STORM	**8.72**
Total Kjeldahl Nitrogen	mg/L	0.1	100	SAN	6.3
Ammonia (N)-Total (NH3+NH4)	mg/L	0.05			0.15
Ammonia (N)-unionized	mg/L	0.01			<0.01
Sulphide	mg/L	0.01	2	SAN	0.01
Cyanide (Total)	mg/L	0.005	2, 0.02	SAN, STORM	<0.005
Phenolics	mg/L	0.001	1, 0.008	SAN, STORM	<0.001
Hardness (as CaCO3)	mg/L	0.02			368
Aluminum	mg/L	0.01			0.07
Barium	mg/L	0.001			0.165
Calcium	mg/L	0.02			105
Iron	mg/L	0.005			0.020
Magnesium	mg/L	0.02			25.6
Tungsten	mg/L	0.01			<0.01

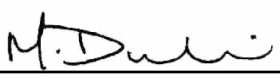
Michelle Dubien
Data Specialist

CADUCEON Environmental Laboratories Certificate of Analysis

Final Report

REPORT No: 24-010898 - Rev. 1

				Client I.D.	BH106
				Sample I.D.	24-010898-1
				Date Collected	2024-Apr-19
Parameter	**Units**	**R.L.**	**Limits**		-
Zinc	mg/L	0.005			<0.005
Zirconium	mg/L	0.003			<0.003
Hardness (as CaCO3)	mg/L	-			789
Aluminum (Total)	mg/L	0.01	50	SAN	0.03
Bismuth (Total)	mg/L	0.02	5	SAN	<0.02
Boron (Total)	mg/L	0.005	25	SAN	0.028
Cadmium (Total)	mg/L	0.005	0.02, 0.008	SAN, STORM	<0.005
Calcium (Total)	mg/L	0.02			97.7
Chromium (Total)	mg/L	0.002	5, 0.08	SAN, STORM	<0.002
Cobalt (Total)	mg/L	0.005	5	SAN	<0.005
Copper (Total)	mg/L	0.002	3, 0.04	SAN, STORM	0.008
Iron (Total)	mg/L	0.005			<0.005
Lead (Total)	mg/L	0.02	5, 0.12	SAN, STORM	<0.02
Magnesium (Total)	mg/L	0.02			27.3
Manganese (Total)	mg/L	0.001	0.05, 5	STORM, SAN	0.003
Molybdenum (Total)	mg/L	0.01	5	SAN	<0.01
Nickel (Total)	mg/L	0.01	3, 0.08	SAN, STORM	<0.01
Silver (Total)	mg/L	0.005	5, 0.12	SAN, STORM	<0.005
Tin (Total)	mg/L	0.05	5	SAN	<0.05
Titanium (Total)	mg/L	0.005	5	SAN	<0.005
Tungsten (Total)	mg/L	0.01			<0.01



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Final Report

REPORT No: 24-010898 - Rev. 1

				Client I.D.	BH106
				Sample I.D.	24-010898-1
				Date Collected	2024-Apr-19
Parameter	**Units**	**R.L.**	**Limits**		-
Vanadium (Total)	mg/L	0.005	5	SAN	<0.005
Zinc (Total)	mg/L	0.005	3, 0.04	SAN, STORM	<0.005
Zirconium (Total)	mg/L	0.003			<0.003
Antimony (Total)	mg/L	0.0001	5	SAN	0.0007
Arsenic (Total)	mg/L	0.0001	0.02, 1	STORM, SAN	**0.0275**
Beryllium (Total)	mg/L	0.0001			0.0032
Cadmium (Total)	mg/L	0.000015	0.008	STORM	0.00112
Chromium (Total)	mg/L	0.001	0.08	STORM	**0.249**
Cobalt (Total)	mg/L	0.0001			0.103
Copper (Total)	mg/L	0.0001	0.04	STORM	**0.301**
Lead (Total)	mg/L	0.00002	0.12	STORM	0.0768
Molybdenum (Total)	mg/L	0.0001			0.0076
Nickel (Total)	mg/L	0.0002	0.08	STORM	**0.189**
Selenium (Total)	mg/L	0.001	0.02, 5	STORM, SAN	<0.001
Silver (Total)	mg/L	0.0001	0.12	STORM	0.0011
Thallium (Total)	mg/L	0.00005			0.00182
Uranium (Total)	mg/L	0.00005			0.0114
Vanadium (Total)	mg/L	0.0001			0.327
Chromium (VI)	mg/L	0.01			<0.01
Mercury	mg/L	0.00002	0.001, 0.0004	SAN, STORM	<0.00002
Anion Sum	meq/L	-			16.6

Michelle Dubien
Data Specialist

CADUCEON Environmental Laboratories Certificate of Analysis

Final Report

REPORT No: 24-010898 - Rev. 1

				Client I.D.	BH106
				Sample I.D.	24-010898-1
				Date Collected	2024-Apr-19
Parameter	**Units**	**R.L.**	**Limits**		-
Cation Sum	meq/L	-			15.3
% Difference	%	-			4.03
Ion Ratio	-	-			1.08
Sodium Adsorption Ratio	-	-			4.28
TDS (Ion Sum Calc)	mg/L	1			893
TDS(calc.)/EC(actual)	-	-			0.540
Conductivity Calc	µmho/cm	-			1590
Conductivity Calc / Conductivity	-	-			0.959
Langelier Index(25°C)	-	-			0.800
Saturation pH (25°C)	-	-			7.05
pH (Client Data)	pH units	-			6.97
Temperature (Client Data)	°C	-			9.9

Michelle Dubien
Data Specialist

CADUCEON Environmental Laboratories Certificate of Analysis

Final Report

REPORT No: 24-010898 - Rev. 1

				Client I.D.	BH106
				Sample I.D.	24-010898-1
				Date Collected	2024-Apr-19
Parameter	**Units**	**R.L.**	**Limits**		-
Benzene	mg/L	0.0005	0.01, 0.002	SAN, STORM	<0.0005
Bromodichloromethane	mg/L	0.002	0.35	SAN	<0.002
Bromoform	mg/L	0.005	0.63	SAN	<0.005
Bromomethane	mg/L	0.0005	0.11	SAN	<0.0005
Carbon Tetrachloride	mg/L	0.0002	0.057	SAN	<0.0002
Chlorobenzene	mg/L	0.0005	0.057	SAN	<0.0005
Chloroethane	mg/L	0.003	0.27	SAN	<0.003
Chloroform	mg/L	0.001	0.08, 0.002	SAN, STORM	<0.001
Chloromethane (Methyl Chloride)	mg/L	0.002	0.19	SAN	<0.002
Dibromochloromethane	mg/L	0.002	0.057	SAN	<0.002
Ethylene Dibromide	mg/L	0.0002	0.028	SAN	<0.0002
Dichlorobenzene,1,2-	mg/L	0.0005	0.088, 0.0056	SAN, STORM	<0.0005
Dichlorobenzene,1,3-	mg/L	0.0005	0.036	SAN	<0.0005
Dichlorobenzene,1,4-	mg/L	0.0005	0.017, 0.0068	SAN, STORM	<0.0005
Dichloroethane,1,1-	mg/L	0.0005	0.2	SAN	<0.0005
Dichloroethane,1,2-	mg/L	0.0005	0.21	SAN	0.0007
Dichloroethylene,1,1-	mg/L	0.0005	0.04	SAN	<0.0005
Dichloroethylene,1,2-cis-	mg/L	0.0005	0.2, 0.0056	SAN, STORM	<0.0005
Dichloroethylene,1,2-trans-	mg/L	0.0005	0.2	SAN	<0.0005
Dichloropropane,1,2-	mg/L	0.0005	0.85	SAN	<0.0005
Dichloropropene,1,3-cis-	mg/L	0.0005	0.07	SAN	<0.0005

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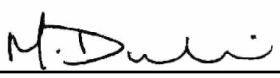
Final Report

REPORT No: 24-010898 - Rev. 1

				Client I.D.	BH106
				Sample I.D.	24-010898-1
				Date Collected	2024-Apr-19
Parameter	**Units**	**R.L.**	**Limits**		-
Dichloropropene,1,3-trans-	mg/L	0.0005	0.07, 0.0056	SAN, STORM	<0.0005
Ethylbenzene	mg/L	0.0005	0.057, 0.002	SAN, STORM	<0.0005
Dichloromethane (Methylene Chloride)	mg/L	0.005	0.211, 0.0052	SAN, STORM	<0.005
Styrene	mg/L	0.0005	0.04	SAN	<0.0005
Tetrachloroethane,1,1,2,2-	mg/L	0.0005	0.04, 0.017	SAN, STORM	<0.0005
Tetrachloroethylene	mg/L	0.0005	0.05, 0.0044	SAN, STORM	<0.0005
Toluene	mg/L	0.0005	0.08, 0.002	SAN, STORM	<0.0005
Trichloroethane,1,1,1-	mg/L	0.0005	0.054	SAN	<0.0005
Trichloroethane,1,1,2-	mg/L	0.0005	0.8	SAN	<0.0005
Trichloroethylene	mg/L	0.0005	0.054, 0.0076	SAN, STORM	<0.0005
Trichlorofluoromethane (Freon 11)	mg/L	0.005	0.02	SAN	<0.005
Trimethylbenzene,1,3,5-	mg/L	0.0001	0.003	SAN	<0.0001
Vinyl Chloride	mg/L	0.0002	0.4	SAN	<0.0002
Xylene, m,p-	µg/L	1			<1
Xylene, m,p,o-	mg/L	0.0011	0.32, 0.0044	SAN, STORM	<0.0011
Xylene, o-	µg/L	0.5			<0.5
Oil & Grease (Total)	mg/L	1.0			1.7
Oil and Grease (Mineral)	mg/L	1.0	15	SAN	<1.0
Oil and Grease (Anim/Veg)	mg/L	1.0	150	SAN	1.4



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REPORT No: 24-010898 - Rev. 1

				Client I.D.	BH106
				Sample I.D.	24-010898-1
				Date Collected	2024-Apr-19
Parameter	**Units**	**R.L.**	**Limits**		-
Acenaphthene	µg/L	0.05			<0.05
Acenaphthylene	µg/L	0.05			<0.05
Anthracene	µg/L	0.05			<0.05
Benzo[a]anthracene	µg/L	0.05			<0.05
Benzo(a)pyrene	µg/L	0.01			<0.01
Benzo(b)fluoranthene	µg/L	0.05			<0.05
Benzo(b+k)fluoranthene	µg/L	0.1			<0.1
Benzo(g,h,i)perylene	µg/L	0.05			<0.05
Benzo(k)fluoranthene	µg/L	0.05			<0.05
Butyl Benzyl Phthalate	mg/L	0.001	0.017	SAN	<0.001
Bis(2-Chloroethoxy)methane	mg/L	0.002	0.036	SAN	<0.002
Bis(2-ethylhexyl) Phthalate	mg/L	0.005	0.28	SAN	<0.005
Chrysene	µg/L	0.05			<0.05
Dibenzo(a,h)anthracene	µg/L	0.05			<0.05
Di-n-Butyl Phthalate	mg/L	0.0010	0.057	SAN	<0.0010
Dichlorophenol,2,4-	mg/L	0.0002	0.044	SAN	<0.0002
Diethyl Phthalate	mg/L	0.0010	0.2	SAN	<0.0010
Di-n-Octyl Phthalate	mg/L	0.0010	0.03	SAN	<0.0010
Fluoranthene	mg/L	0.00005	0.059	SAN	<0.00005
Fluorene	µg/L	0.05			<0.05
Indeno(1,2,3,-cd)Pyrene	µg/L	0.05			<0.05

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Parameter	Units	R.L.	Limits		Client I.D.: BH106 Sample I.D.: 24-010898-1 Date Collected: 2024-Apr-19
					-
Indole	mg/L	0.002	0.05	SAN	<0.002
Methylnaphthalene,1-	mg/L	0.00005	0.032	SAN	<0.00005
Methylnaphthalene,2-(1-)	µg/L	1			<1
Methylnaphthalene,2-	mg/L	0.00005	0.022	SAN	<0.00005
Naphthalene	mg/L	0.00005	0.059, 0.064	SAN, STORM	<0.00005
Phenanthrene	µg/L	0.05			<0.05
Pyrene	µg/L	0.05			<0.05
Total PAH	mg/L	0.0001	0.015, 0.006	SAN, STORM	<0.0001

Parameter	Units	R.L.	Limits		Client I.D.: BH106 Sample I.D.: 24-010898-1 Date Collected: 2024-Apr-19
					-
Hexachlorobenzene	mg/L	0.00001	0.00004	STORM	<0.00001

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REPORT No: 24-010898 - Rev. 1

Subcontracted Analyses

				Client I.D.	BH106
				Sample I.D.	24-010898-1
				Date Collected	2024-Apr-19
Parameter	**Units**	**R.L.**	**Limits**		-
Formaldehyde	mg/L	-	0.3	SAN	<0.008
Nitrosodimethylamine (NDMA)	mg/L	-	0.4	SAN	<0.0004
Nonylphenol Monoethoxylate	mg/L	-			<0.01
Nonylphenol Diethoxylate	mg/L	-			<0.01
Nonylphenols	mg/L	-	0.0025, 0.001	SAN, STORM	<0.001
Nonylphenol Ethoxylates	mg/L	-	0.025, 0.01	SAN, STORM	<0.01

Revised to include additional dissolved metals at clients request

: City of Ottawa
SAN: Sanitary Sewer By Law
STORM: Storm Sewer By Law

Summary of Exceedances		
Sanitary Sewer By Law		
BH106	**Found Value**	**Limit**
Total Suspended Solids	9480	350
Storm Sewer By Law		
BH106	**Found Value**	**Limit**
Total Suspended Solids	9480	15.0
Phosphorus (Total)	8.72	0.4
Arsenic (Total)	0.0275	0.02
Chromium (Total)	0.249	0.08
Copper (Total)	0.301	0.04
Nickel (Total)	0.189	0.08

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CERTIFICATE OF ANALYSIS

Final Report

C.O.C.: G 107579 **REPORT No: 24-010898 - Rev. 2**

Report To:
Cambium Environmental - Kingston
625 Fortune Crescent
#1
Kingston, ON K7P 0L5

CADUCEON Environmental Laboratories
2378 Holly Lane
Ottawa, ON K1V 7P1

Attention: Kyle Horner

DATE RECEIVED: 2024-Apr-22
DATE REPORTED: 2024-Aug-07
SAMPLE MATRIX: Ground Water

CUSTOMER PROJECT: 17280-002
P.O. NUMBER:

Analyses	Qty	Site Analyzed	Authorized	Date Analyzed	Lab Method	Reference Method
Anions (Liquid)	1	OTTAWA	PCURIEL	2024-Apr-24	A-IC-01	SM 4110B
BOD5 (Liquid)	1	KINGSTON	JYEARWOOD	2024-Apr-24	BOD-001	SM 5210B
Cond/pH/Alk Auto (Liquid)	1	OTTAWA	SBOUDREAU	2024-Apr-22	COND-02/PH-02/A LK-02	SM 2510B/4500H/ 2320B
Cyanide Total (Liquid)	1	KINGSTON	JMACINNES	2024-Apr-23	CN-001	SM 4500-CN-E
Formaldehyde (Subcontracted)	1	TESTMARK	SISLAM	2024-Apr-26		Subcontracted
Ion Balance (Calc.)	1	OTTAWA	ASCHNEIDER		CP-028	MECP E3196
Chromium VI (Liquid)	1	OTTAWA	STAILLON	2024-Apr-25	D-CRVI-01	MECP E3056
ICP/MS Total (Liquid)	1	OTTAWA	AOZKAYMAK	2024-Apr-24	D-ICPMS-01	EPA 6020
ICP/OES Total (Liquid)	1	OTTAWA	APRUDYVUS	2024-Apr-29	D-ICP-01	SM 3120B
ICP/OES (Liquid)	1	OTTAWA	APRUDYVUS	2024-Apr-24	D-ICP-01	SM 3120B
Mercury (Liquid)	1	OTTAWA	TBENNETT	2024-Apr-24	D-HG-02	SM 3112B
NDMA Liquid (Subcontract)	1	SGS_LAKEFIELD	SISLAM	2024-May-30		Subcontracted
Ammonia (Liquid)	1	KINGSTON	JYEARWOOD	2024-Apr-24	NH3-001	SM 4500NH3
Nonylphenols (Subcontracted)	1	SGS_LAKEFIELD	SISLAM	2024-Apr-30		Subcontracted
OC Pesticides (Liquid)	1	KINGSTON	CSUMMERHAYS	2024-Apr-23	PESTCL-001	EPA 8081
Oil & Grease (Liquid)	1	KINGSTON	MLANE	2024-Apr-25	O&G-001	SM 5520
Phenols (Liquid)	1	KINGSTON	JMACINNES	2024-Apr-25	PHEN-01	MECP E3179
Sulphide (Liquid)	1	KINGSTON	EHINCH	2024-Apr-23	H2S-001	SM 4500-S2
SVOC - Semi-Volatiles (Liquid)	1	KINGSTON	EASIEDU	2024-Apr-24	NAB-W-001	EPA 8270D
TP & TKN (Liquid)	1	KINGSTON	KDIBBITS	2024-Apr-29	TPTKN-001	MECP E3516.2
TSS (Liquid)	1	KINGSTON	MCLOSS	2024-Apr-23	TSS-001	SM 2540D
Turbidity (Liquid)	1	OTTAWA	STAILLON	2024-Apr-23	A-TURB-01	SM 2130B
VOC-Volatiles Full (Water)	1	RICHMOND_HILL	FLENA	2024-Apr-24	C-VOC-02	EPA 8260

R.L. = Reporting Limit
NC = Not Calculated
Test methods may be modified from specified reference method unless indicated by an *

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				Client I.D.	BH106
				Sample I.D.	24-010898-1
				Date Collected	2024-Apr-19
Parameter	**Units**	**R.L.**	**Limits**		-
Alkalinity(CaCO3) to pH4.5	mg/L	5			283
pH @25°C	pH units	-	8.5	PWQO	7.85
Turbidity	NTU	0.1			7070
Fluoride	mg/L	0.1			<0.1
Sulphate	mg/L	1			84
BOD5	mg/L	3			3
Total Suspended Solids	mg/L	3			9480
Phosphorus (Total)	µg/L	10	10	INTERIM	8720
Total Kjeldahl Nitrogen	mg/L	0.1			6.3
Ammonia (N)-Total (NH3+NH4)	mg/L	0.05			0.15
Ammonia (N)-unionized	µg/L	10.0	20	PWQO	<10.0
Sulphide	mg/L	0.01			0.01
Cyanide (Total)	mg/L	0.005			<0.005
Phenolics	µg/L	1	1	PWQO	<1
Hardness (as CaCO3)	mg/L as CaCO3	0			368
Aluminum	µg/L	10	75	INTERIM	70
Barium	µg/L	1			165
Calcium	µg/L	20			105000
Iron	µg/L	5	300	PWQO	20
Magnesium	µg/L	20			25600
Tungsten	µg/L	10			<10

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				Client I.D.	BH106
				Sample I.D.	24-010898-1
				Date Collected	2024-Apr-19
Parameter	**Units**	**R.L.**	**Limits**		-
Zinc	µg/L	5	30	PWQO	<5
Zirconium	µg/L	3			<3
Hardness (as CaCO3)	mg/L as CaCO3	-			789
Aluminum (Total)	µg/L	10			30
Bismuth (Total)	µg/L	20			<20
Boron (Total)	µg/L	5	200	INTERIM	28
Cadmium (Total)	µg/L	5	0.1, 0.2	INTERIM, PWQO	<5
Calcium (Total)	µg/L	20			97700
Chromium (Total)	µg/L	2			<2
Cobalt (Total)	µg/L	5	0.9, 0.0	INTERIM, PWQO	<5
Copper (Total)	µg/L	2	5, 0.0	INTERIM, PWQO	8
Iron (Total)	µg/L	5	300	PWQO	<5
Lead (Total)	µg/L	20	1, 0.0	INTERIM, PWQO	<20
Magnesium (Total)	µg/L	20			27300
Manganese (Total)	µg/L	1			3
Molybdenum (Total)	µg/L	10	40, 0.0	INTERIM, PWQO	<10
Nickel (Total)	µg/L	10	25	PWQO	<10
Silver (Total)	µg/L	5	0.1	PWQO	<5
Tin (Total)	µg/L	50			<50
Titanium (Total)	µg/L	5			<5
Tungsten (Total)	µg/L	10	30	INTERIM	<10

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REPORT No: 24-010898 - Rev. 2

				Client I.D.	BH106
				Sample I.D.	24-010898-1
				Date Collected	2024-Apr-19
Parameter	**Units**	**R.L.**	**Limits**		-
Vanadium (Total)	µg/L	5			<5
Zinc (Total)	µg/L	5	20, 30	INTERIM, PWQO	<5
Zirconium (Total)	µg/L	3	4	INTERIM	<3
Antimony (Total)	µg/L	0.1	20	INTERIM	0.7
Arsenic (Total)	µg/L	0.1	5, 5	INTERIM, PWQO	27.5
Beryllium (Total)	µg/L	0.1	11	PWQO	3.2
Cadmium (Total)	µg/L	0.015	0.1, 0.2	INTERIM, PWQO	1.12
Chromium (Total)	µg/L	1			249
Cobalt (Total)	µg/L	0.1	0.9	INTERIM	103
Copper (Total)	µg/L	0.1	5	INTERIM	301
Lead (Total)	µg/L	0.02	1, 5	INTERIM, PWQO	76.8
Molybdenum (Total)	µg/L	0.1	40	INTERIM	7.6
Nickel (Total)	µg/L	0.2	25	PWQO	189
Selenium (Total)	µg/L	1	100	PWQO	<1
Silver (Total)	µg/L	0.1	0.1	PWQO	1.1
Thallium (Total)	µg/L	0.05	0.3, 0.3	INTERIM, PWQO	1.82
Uranium (Total)	µg/L	0.05	5	INTERIM	11.4
Vanadium (Total)	µg/L	0.1	6	INTERIM	327
Chromium (VI)	µg/L	10	1	PWQO	<10
Mercury	µg/L	0.02	0.2	PWQO	<0.02
Anion Sum	meq/L	-			16.6

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				Client I.D.	BH106
				Sample I.D.	24-010898-1
				Date Collected	2024-Apr-19
Parameter	**Units**	**R.L.**	**Limits**		-
Cation Sum	meq/L	-			15.3
% Difference	%	-			4.03
Ion Ratio	-	-			1.08
Sodium Adsorption Ratio	-	-			4.28
TDS (Ion Sum Calc)	mg/L	1			893
TDS(calc.)/EC(actual)	-	-			0.540
Conductivity Calc	µmho/cm	-			1590
Conductivity Calc / Conductivity	-	-			0.959
Langelier Index(25°C)	-	-			0.800
Saturation pH (25°C)	-	-			7.05
pH (Client Data)	pH units	-			6.97
Temperature (Client Data)	°C	-			9.9

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REPORT No: 24-010898 - Rev. 2

				Client I.D.	BH106
				Sample I.D.	24-010898-1
				Date Collected	2024-Apr-19
Parameter	**Units**	**R.L.**	**Limits**		-
Benzene	µg/L	0.5	100	INTERIM	<0.5
Bromodichloromethane	µg/L	2	200	INTERIM	<2
Bromoform	µg/L	5	60	INTERIM	<5
Bromomethane	µg/L	0.5	0.9	INTERIM	<0.5
Carbon Tetrachloride	µg/L	0.2			<0.2
Chlorobenzene	µg/L	0.5	15	PWQO	<0.5
Chloroethane	µg/L	3			<3
Chloroform	µg/L	1			<1
Chloromethane (Methyl Chloride)	µg/L	2	700	INTERIM	<2
Dibromochloromethane	µg/L	2	40	INTERIM	<2
Ethylene Dibromide	µg/L	0.2	5, 5	INTERIM, PWQO	<0.2
Dichlorobenzene,1,2-	µg/L	0.5	2.5	PWQO	<0.5
Dichlorobenzene,1,3-	µg/L	0.5	2.5	PWQO	<0.5
Dichlorobenzene,1,4-	µg/L	0.5	4	PWQO	<0.5
Dichloroethane,1,1-	µg/L	0.5	200	INTERIM	<0.5
Dichloroethane,1,2-	µg/L	0.5	100	INTERIM	0.7
Dichloroethylene,1,1-	µg/L	0.5	40	INTERIM	<0.5
Dichloroethylene,1,2-cis-	µg/L	0.5	200	INTERIM	<0.5
Dichloroethylene,1,2-trans-	µg/L	0.5	200	INTERIM	<0.5
Dichloropropane,1,2-	µg/L	0.5	0.7	INTERIM	<0.5
Dichloropropene,1,3-cis-	µg/L	0.5			<0.5

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REPORT No: 24-010898 - Rev. 2

				Client I.D.	BH106
				Sample I.D.	24-010898-1
				Date Collected	2024-Apr-19
Parameter	**Units**	**R.L.**	**Limits**		-
Dichloropropene,1,3-trans-	µg/L	0.5	7	INTERIM	<0.5
Ethylbenzene	µg/L	0.5	8	INTERIM	<0.5
Dichloromethane (Methylene Chloride)	µg/L	5	100	INTERIM	<5
Styrene	µg/L	0.5	4	INTERIM	<0.5
Tetrachloroethane,1,1,2,2-	µg/L	0.5	70	INTERIM	<0.5
Tetrachloroethylene	µg/L	0.5	50	INTERIM	<0.5
Toluene	µg/L	0.5	0.8, 0.8	INTERIM, PWQO	<0.5
Trichloroethane,1,1,1-	µg/L	0.5	10	INTERIM	<0.5
Trichloroethane,1,1,2-	µg/L	0.5	800	INTERIM	<0.5
Trichloroethylene	µg/L	0.5	20	INTERIM	<0.5
Trichlorofluoromethane (Freon 11)	µg/L	5			<5
Trimethylbenzene,1,3,5-	µg/L	0.1	3	INTERIM	<0.1
Vinyl Chloride	µg/L	0.2	600	INTERIM	<0.2
Xylene, m,p-	µg/L	1			<1
Xylene, m,p,o-	µg/L	1.1			<1.1
Xylene, o-	µg/L	0.5	40	INTERIM	<0.5
Oil & Grease (Total)	mg/L	1.0			1.7
Oil and Grease (Mineral)	mg/L	1.0			<1.0
Oil and Grease (Anim/Veg)	mg/L	1.0			1.4

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REPORT No: 24-010898 - Rev. 2

				Client I.D.	BH106
				Sample I.D.	24-010898-1
				Date Collected	2024-Apr-19
Parameter	**Units**	**R.L.**	**Limits**		-
Acenaphthene	µg/L	0.05			<0.05
Acenaphthylene	µg/L	0.05			<0.05
Anthracene	µg/L	0.05	0.0008	PWQO	**<0.05**
Benzo[a]anthracene	µg/L	0.05	0.0004	INTERIM	**<0.05**
Benzo(a)pyrene	µg/L	0.01			<0.01
Benzo(b)fluoranthene	µg/L	0.05			<0.05
Benzo(b+k)fluoranthene	µg/L	0.1			<0.1
Benzo(g,h,i)perylene	µg/L	0.05	0.00002	INTERIM	**<0.05**
Benzo(k)fluoranthene	µg/L	0.05			<0.05
Butyl Benzyl Phthalate	µg/L	1	0.2	INTERIM	**<1**
Bis(2-Chloroethoxy)methane	µg/L	2			<2
Bis(2-ethylhexyl) Phthalate	µg/L	5			<5
Chrysene	µg/L	0.05	0.0001	INTERIM	**<0.05**
Dibenzo(a,h)anthracene	µg/L	0.05	0.002	INTERIM	**<0.05**
Di-n-Butyl Phthalate	µg/L	1	4	PWQO	<1
Dichlorophenol,2,4-	µg/L	0.2	0.2	PWQO	<0.2
Diethyl Phthalate	µg/L	1			<1
Di-n-Octyl Phthalate	µg/L	1	0.6	PWQO	**<1**
Fluoranthene	µg/L	0.05	0.0008	INTERIM	**<0.05**
Fluorene	µg/L	0.05	0.2	INTERIM	<0.05
Indeno(1,2,3,-cd)Pyrene	µg/L	0.05			<0.05

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Final Report

REPORT No: 24-010898 - Rev. 2

Parameter	Units	R.L.	Limits		Client I.D.: BH106 Sample I.D.: 24-010898-1 Date Collected: 2024-Apr-19
					-
Indole	µg/L	2			<2
Methylnaphthalene,1-	µg/L	0.05	2	INTERIM	<0.05
Methylnaphthalene,2-(1-)	µg/L	1			<1
Methylnaphthalene,2-	µg/L	0.05	2	INTERIM	<0.05
Naphthalene	µg/L	0.05	7	INTERIM	<0.05
Phenanthrene	µg/L	0.05	0.03	INTERIM	**<0.05**
Pyrene	µg/L	0.05			<0.05
Total PAH	µg/L	0.1			<0.1

Parameter	Units	R.L.	Limits		Client I.D.: BH106 Sample I.D.: 24-010898-1 Date Collected: 2024-Apr-19
					-
Hexachlorobenzene	µg/L	0.01			<0.01

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REPORT No: 24-010898 - Rev. 2

Subcontracted Analyses

Parameter	Units	R.L.	Limits		Client I.D.: BH106 Sample I.D.: 24-010898-1 Date Collected: 2024-Apr-19
					-
Formaldehyde	µg/L	-	0.8	INTERIM	<8
Nitrosodimethylamine (NDMA)	µg/L	-	15	INTERIM	<0.4
Nonylphenol Monoethoxylate	µg/L	-			<10
Nonylphenol Diethoxylate	µg/L	-			<10
Nonylphenols	µg/L	-	0.04	INTERIM	<1
Nonylphenol Ethoxylates	µg/L	-			<10

Revised to change guideline to PWQO

: PWQO Limits
INTERIM: Interim PWQO
PWQO: PWQO

Michelle Dubien
Data Specialist

Summary of Exceedances

Interim PWQO

BH106	Found Value	Limit
Phosphorus (Total)	8720	10
Cadmium (Total)	<5	0.1
Cobalt (Total)	<5	0.9
Copper (Total)	8	5
Lead (Total)	<20	1
Arsenic (Total)	27.5	5
Cadmium (Total)	1.12	0.1
Cobalt (Total)	103	0.9
Copper (Total)	301	5
Lead (Total)	76.8	1
Thallium (Total)	1.82	0.3
Uranium (Total)	11.4	5
Vanadium (Total)	327	6
Benzo[a]anthracene	<0.05	0.0004
Benzo(g,h,i)perylene	<0.05	0.00002
Butyl Benzyl Phthalate	<1	0.2
Chrysene	<0.05	0.0001
Dibenzo(a,h)anthracene	<0.05	0.002
Fluoranthene	<0.05	0.0008
Phenanthrene	<0.05	0.03
Formaldehyde	<8	0.8
Nonylphenols	<1	0.04

PWQO

BH106	Found Value	Limit
Cadmium (Total)	<5	0.2
Silver (Total)	<5	0.1
Arsenic (Total)	27.5	5
Cadmium (Total)	1.12	0.2
Lead (Total)	76.8	5
Nickel (Total)	189	25
Silver (Total)	1.1	0.1
Thallium (Total)	1.82	0.3
Chromium (VI)	<10	1
Anthracene	<0.05	0.0008
Di-n-Octyl Phthalate	<1	0.6

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CERTIFICATE OF ANALYSIS

Final Report

C.O.C.: G 106721 **REPORT No: 24-024417 - Rev. 0**

Report To:
Cambium Environmental - Kingston
31 Hyperion Crt
Suite 102
Kingston, ON K7K 7G3

Attention: Natasha Augustine

CADUCEON Environmental Laboratories
285 Dalton Ave
Kingston, ON K7K 6Z1

DATE RECEIVED: 2024-Aug-10
DATE REPORTED: 2024-Aug-16
SAMPLE MATRIX: Ground Water

CUSTOMER PROJECT: 17281-002
P.O. NUMBER:

Analyses	Qty	Site Analyzed	Authorized	Date Analyzed	Lab Method	Reference Method
ICP/MS (Liquid)	1	OTTAWA	AOZKAYMAK	2024-Aug-13	D-ICPMS-01	EPA 200.8
ICP/OES (Liquid)	1	OTTAWA	APRUDYVUS	2024-Aug-14	D-ICP-01	SM 3120B
TSS (Liquid)	1	KINGSTON	DCASSIDY	2024-Aug-15	TSS-001	SM 2540D

R.L. = Reporting Limit
NC = Not Calculated
Test methods may be modified from specified reference method unless indicated by an *

				Client I.D.	BH106
				Sample I.D.	24-024417-1
				Date Collected	2024-Aug-08
Parameter	**Units**	**R.L.**	**Limits**		-
Total Suspended Solids	mg/L	3			<3
Hardness (as CaCO3)	mg/L as CaCO3	0			380
Aluminum	µg/L	10	75	INTERIM	20
Boron	µg/L	5	200	INTERIM	62
Calcium	µg/L	20			107000
Iron	µg/L	5	300	PWQO	334
Magnesium	µg/L	20			27400
Tungsten	µg/L	10			<10
Zinc	µg/L	5	30	PWQO	<5
Zirconium	µg/L	3			<3

Michelle Dubien
Data Specialist

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REPORT No: 24-024417 - Rev. 0

				Client I.D.	BH106
				Sample I.D.	24-024417-1
				Date Collected	2024-Aug-08
Parameter	**Units**	**R.L.**	**Limits**		-
Antimony	µg/L	0.1	20, 5	INTERIM, PWQO	0.3
Arsenic	µg/L	0.1	5, 0.0	INTERIM, PWQO	1.0
Beryllium	µg/L	0.1	0.0, 11	INTERIM, PWQO	<0.1
Cadmium	µg/L	0.015	0.1, 0.2	INTERIM, PWQO	**0.211**
Chromium	µg/L	1.0			<1.0
Cobalt	µg/L	0.1			1.1
Copper	µg/L	0.1	5	INTERIM	**5.4**
Lead	µg/L	0.02	1, 5	INTERIM, PWQO	0.08
Molybdenum	µg/L	0.1	40	INTERIM	5.0
Nickel	µg/L	0.2	25	PWQO	3.8
Selenium	µg/L	1.00	100	PWQO	<1.00
Silver	µg/L	0.1	0.1	PWQO	<0.1
Thallium	µg/L	0.05	0.3, 0.3	INTERIM, PWQO	<0.05
Uranium	µg/L	0.05	5	INTERIM	4.68
Vanadium	µg/L	0.1	6	INTERIM	0.3

: PWQO Limits
INTERIM: Interim PWQO
PWQO: PWQO

Michelle Dubien
Data Specialist

Summary of Exceedances		
Interim PWQO		
BH106	**Found Value**	**Limit**
Cadmium	0.211	0.1
Copper	5.4	5
PWQO		
BH106	**Found Value**	**Limit**
Iron	334	300
Cadmium	0.211	0.2

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Data Specialist

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Final Report

C.O.C.: G 106721

REPORT No: 24-024417 - Rev. 2

Report To:
Cambium Environmental - Kingston
31 Hyperion Crt
Suite 102
Kingston, ON K7K 7G3

Attention: Natasha Augustine

CADUCEON Environmental Laboratories
285 Dalton Ave
Kingston, ON K7K 6Z1

DATE RECEIVED: 2024-Aug-10
DATE REPORTED: 2024-Sep-05
SAMPLE MATRIX: Ground Water

CUSTOMER PROJECT: 17281-002
P.O. NUMBER:

Analyses	Qty	Site Analyzed	Authorized	Date Analyzed	Lab Method	Reference Method
ICP/MS (Liquid)	1	OTTAWA	AOZKAYMAK	2024-Aug-13	D-ICPMS-01	EPA 200.8
ICP/OES (Liquid)	1	OTTAWA	APRUDYVUS	2024-Aug-14	D-ICP-01	SM 3120B
TSS (Liquid)	1	KINGSTON	DCASSIDY	2024-Aug-15	TSS-001	SM 2540D

R.L. = Reporting Limit
NC = Not Calculated
Test methods may be modified from specified reference method unless indicated by an *

Parameter	Units	R.L.	Limits		Client I.D.: BH106 Sample I.D.: 24-024417-1 Date Collected: 2024-Aug-08
					-
Total Suspended Solids	mg/L	3	350, 15.0	SAN, STORM	<3
Hardness (as CaCO3)	mg/L as CaCO3	0.02			380
Aluminum	mg/L	0.01	50	SAN	0.02
Boron	mg/L	0.005	25	SAN	0.062
Calcium	mg/L	0.02			107
Iron	mg/L	0.005			0.334
Magnesium	mg/L	0.02			27.4
Phosphorus	mg/L	0.1			<0.1
Tungsten	mg/L	0.01			<0.01
Zinc	mg/L	0.005	3, 0.04	SAN, STORM	<0.005

Steve Garrett
Director of Laboratory Services

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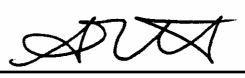
REPORT No: 24-024417 - Rev. 2

				Client I.D.	BH106
				Sample I.D.	24-024417-1
				Date Collected	2024-Aug-08
Parameter	**Units**	**R.L.**	**Limits**		-
Zirconium	mg/L	0.003			<0.003
Antimony	mg/L	0.0001	5	SAN	0.0003
Arsenic	mg/L	0.0001	1, 0.02	SAN, STORM	0.0010
Beryllium	mg/L	0.0001			<0.0001
Cadmium	mg/L	0.000015	0.02, 0.008	SAN, STORM	0.000211
Chromium	mg/L	0.001	5, 0.08	SAN, STORM	<0.001
Cobalt	mg/L	0.0001	5	SAN	0.0011
Copper	mg/L	0.0001	3, 0.04	SAN, STORM	0.0054
Lead	mg/L	0.00002	5, 0.12	SAN, STORM	0.00008
Molybdenum	mg/L	0.0001	5	SAN	0.0050
Nickel	mg/L	0.0002	3, 0.08	SAN, STORM	0.0038
Selenium	mg/L	0.001	5, 0.02	SAN, STORM	<0.001
Silver	mg/L	0.0001	5, 0.12	SAN, STORM	<0.0001
Thallium	mg/L	0.00005			<0.00005
Uranium	mg/L	0.00005			0.00468
Vanadium	mg/L	0.0001	5	SAN	0.0003

Revised to add Phosphorous result by ICP

: City of Ottawa
SAN: Sanitary Sewer By Law
STORM: Storm Sewer By Law



Steve Garrett
Director of Laboratory Services

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CERTIFICATE OF ANALYSIS

Final Report

C.O.C.: G 112298

REPORT No: 24-027621 - Rev. 0

Report To:
Cambium Environmental - Kingston
31 Hyperion Crt
Suite 102
Kingston, ON K7K 7G3

CADUCEON Environmental Laboratories
285 Dalton Ave
Kingston, ON K7K 6Z1

Attention: Natasha Augustine

DATE RECEIVED: 2024-Sep-06
DATE REPORTED: 2024-Sep-10
SAMPLE MATRIX: Ground Water

CUSTOMER PROJECT: 17281-002
P.O. NUMBER:

Analyses	Qty	Site Analyzed	Authorized	Date Analyzed	Lab Method	Reference Method
TP & TKN (Liquid)	1	KINGSTON	YLIEN	2024-Sep-10	TPTKN-001	MECP E3516.2

R.L. = Reporting Limit
NC = Not Calculated
Test methods may be modified from specified reference method unless indicated by an *

Parameter			Phosphorus (Total)
		Units	mg/L
		R.L.	0.01
Client I.D.	Sample I.D.	Date Collected	-
BH106	24-027621-1	2024-Sep-05	<0.01

Steve Garrett
Director of Laboratory Services

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CERTIFICATE OF ANALYSIS

Final Report

C.O.C.: G 132184

REPORT No: 25-015207 - Rev. 0

Report To:
Cambium Environmental - Kingston
31 Hyperion Crt
Suite 102
Kingston, ON K7K 7G3

CADUCEON Environmental Laboratories
2378 Holly Lane
Ottawa, ON K1V 7P1

Attention: Kyle Horner

DATE RECEIVED: 2025-May-30
DATE REPORTED: 2025-Jun-06
SAMPLE MATRIX: Ground Water

CUSTOMER PROJECT: 17281-001
P.O. NUMBER: 17281-001

Analyses	Qty	Site Analyzed	Authorized	Date Analyzed	Lab Method	Reference Method
Anions (Liquid)	2	OTTAWA	STAILLON	2025-Jun-02	A-IC-01	SM 4110B
Colour (Liquid)	2	OTTAWA	MMIRELLA	2025-Jun-04	A-COL-01	SM 2120C
Cond/pH/Alk Auto (Liquid)	2	OTTAWA	SBOUDREAU	2025-May-30	COND-02/PH-02/A LK-02	SM 2510B/4500H/ 2320B
Coliforms - DC Media (Liquid)	2	OTTAWA	AHIRSI	2025-May-30	ECTC-001	MECP E3407
DOC (Liquid)	2	OTTAWA	SLOZO	2025-Jun-02	C-OC-01	EPA 415.2
HPC Spread Plate (Liquid)	2	OTTAWA	SLOZO	2025-May-30	HPC-001	SM 9215D
Ion Balance (Calc)	2	OTTAWA	ASCHNEIDER		CP-028	MECP E3196
ICP/MS (Liquid)	2	OTTAWA	TPRICE	2025-Jun-03	D-ICPMS-01	EPA 200.8
ICP/OES (Liquid)	2	OTTAWA	GFENTON	2025-Jun-02	D-ICP-01	SM 3120B
Ammonia (Liquid)	2	KINGSTON	DCASSIDY	2025-Jun-06	NH3-001	SM 4500NH3
Phenols (Liquid)	2	KINGSTON	MCLOSS	2025-Jun-03	PHEN-01	MECP E3179
Sulphide (Liquid)	2	KINGSTON	MWILSON	2025-Jun-02	H2S-001	SM 4500-S2
Tannins (Liquid)	2	KINGSTON	MWILSON	2025-Jun-03	TAN-001	SM 5550
TP & TKN (Liquid)	2	KINGSTON	YLIEN	2025-Jun-06	TPTKN-001	MECP E3516.2
Turbidity (Liquid)	2	OTTAWA	MMIRELLA	2025-May-30	A-TURB-01	SM 2130B
VOC-Volatiles Full (Water)	2	RICHMOND_HILL	FLENA	2025-Jun-04	C-VOC-02	EPA 8260

R.L. = Reporting Limit
NC = Not Calculated
Test methods may be modified from specified reference method unless indicated by an *

Michelle Dubien
Data Specialist

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REPORT No: 25-015207 - Rev. 0

Parameter	Units	R.L.	Limits	DWG	Client I.D.	TW1-1	TW1-2
					Sample I.D.	25-015207-1	25-015207-2
					Date Collected	2025-May-29	2025-May-29
					DWG	-	-
Total Coliform (DC Media)	CFU/100mL	1	0	MAC		0	0
E coli (DC Media)	CFU/100mL	1	0	MAC		0	0
Background (DC Media)	CFU/100mL	1				55	37
Heterotrophic Plate Count	CFU/1mL	2				<2	<2
Alkalinity(CaCO3) to pH4.5	mg/L	5	500	OG		241	244
TDS (Calc. from Cond.)	mg/L	3	500	AO		495	503
Conductivity @25°C	uS/cm	1				932	946
pH @25°C	pH units	-	8.5	OG		7.98	7.95
Colour	TCU	2	5	AO		3	2
Turbidity	NTU	0.1	5	AO		8.4	3.4
Fluoride	mg/L	0.1	1.5	MAC		<0.1	<0.1
Chloride	mg/L	0.5	250	AO		125	132
Nitrate (N)	mg/L	0.05	10.0	MAC		<0.05	<0.05
Nitrite (N)	mg/L	0.05	1.0	MAC		<0.05	<0.05
Sulphate	mg/L	1	500	AO		64	65
Total Kjeldahl Nitrogen	mg/L	0.1				0.2	0.2
Ammonia (N)-Total (NH3+NH4)	mg/L	0.05				0.08	0.08
Dissolved Organic Carbon	mg/L	0.8	5	AO		1.3	1.2
Tannin & Lignin	mg/L	0.5				<0.5	<0.5
Sulphide	mg/L	0.01	0.05	AO		<0.01	<0.01
Phenolics	mg/L	0.001				<0.001	<0.001

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REPORT No: 25-015207 - Rev. 0

				Client I.D.	TW1-1	TW1-2
				Sample I.D.	25-015207-1	25-015207-2
				Date Collected	2025-May-29	2025-May-29
Parameter	**Units**	**R.L.**	**Limits**	**DWG**	-	-
Hardness (as CaCO3)	mg/L as CaCO3	0.02	100	OG	**389**	**394**
Aluminum	mg/L	0.01	0.1	OG	0.02	0.03
Barium	mg/L	0.001	1	MAC	0.825	0.839
Boron	mg/L	0.005	5	MAC	0.025	0.025
Calcium	mg/L	0.02			100	101
Iron	mg/L	0.005	0.3	AO	0.205	**0.326**
Magnesium	mg/L	0.02			33.8	34.2
Manganese	mg/L	0.001	0.05	AO	0.030	0.025
Potassium	mg/L	0.1			2.7	2.7
Sodium	mg/L	0.2	200, 20, 20	AO, WL, MAC	**38.9**	**40.0**
Strontium	mg/L	0.001			0.393	0.399
Zinc	mg/L	0.005	5	AO	<0.005	<0.005
Antimony	mg/L	0.0001	0.006	MAC	<0.0001	<0.0001
Arsenic	mg/L	0.0001	0.01	MAC	<0.0001	<0.0001
Beryllium	mg/L	0.0001			<0.0001	<0.0001
Cadmium	mg/L	0.000015	0.005	MAC	<0.000015	<0.000015
Chromium	mg/L	0.001	0.05	MAC	<0.001	<0.001
Cobalt	mg/L	0.0001			0.0001	0.0001
Copper	mg/L	0.0001	1	AO	0.0005	0.0008
Lead	mg/L	0.00002	0.010	MAC	0.00002	0.00002
Molybdenum	mg/L	0.0001			0.0008	0.0008

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Parameter	Units	R.L.	Limits	DWG	Client I.D.	TW1-1	TW1-2
					Sample I.D.	25-015207-1	25-015207-2
					Date Collected	2025-May-29	2025-May-29
						-	-
Nickel	mg/L	0.0002				0.0007	0.0007
Selenium	mg/L	0.001	0.05	MAC		<0.001	<0.001
Silver	mg/L	0.0001				<0.0001	<0.0001
Thallium	mg/L	0.00005				<0.00005	<0.00005
Uranium	mg/L	0.00005	0.02	MAC		0.00035	0.00036
Vanadium	mg/L	0.0001				<0.0001	<0.0001
Anion Sum	meq/L	-				9.70	9.96
Cation Sum	meq/L	-				9.56	9.71
% Difference	%	-				0.707	1.28
TDS (Ion Sum Calc)	mg/L	1	500	AO		510	522
Conductivity Calc	µmho/cm	-				944	964
pH (Client Data)	pH units	-				7.33	7.42
Temperature (Client Data)	°C	-				11.6	11.8

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Data Specialist

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Final Report

REPORT No: 25-015207 - Rev. 0

				Client I.D.	TW1-1	TW1-2
				Sample I.D.	25-015207-1	25-015207-2
				Date Collected	2025-May-29	2025-May-29
Parameter	**Units**	**R.L.**	**Limits**	**DWG**	-	-
Acetone	µg/L	30			<30	<30
Benzene	µg/L	0.5	1	MAC	<0.5	<0.5
Bromodichloromethane	µg/L	2			<2	<2
Bromoform	µg/L	5			<5	<5
Bromomethane	µg/L	0.5			<0.5	<0.5
Carbon Tetrachloride	µg/L	0.2	2	MAC	<0.2	<0.2
Chlorobenzene	µg/L	0.5	80, 30	MAC, AO	<0.5	<0.5
Chloroform	µg/L	1			<1	<1
Dibromochloromethane	µg/L	2			<2	<2
Ethylene Dibromide	µg/L	0.2			<0.2	<0.2
Dichlorobenzene,1,2-	µg/L	0.5	200, 3	MAC, AO	<0.5	<0.5
Dichlorobenzene,1,3-	µg/L	0.5			<0.5	<0.5
Dichlorobenzene,1,4-	µg/L	0.5	5, 1	MAC, AO	<0.5	<0.5
Dichlorodifluoromethane (Freon 12)	µg/L	2			<2	<2
Dichloroethane,1,1-	µg/L	0.5			<0.5	<0.5
Dichloroethane,1,2-	µg/L	0.5	5	MAC	<0.5	<0.5
Dichloroethylene,1,1-	µg/L	0.5	14	MAC	<0.5	<0.5
Dichloroethylene,1,2-cis-	µg/L	0.5			<0.5	<0.5
Dichloroethylene,1,2-cis+trans-	µg/L	0.7			<0.7	<0.7
Dichloroethylene,1,2-trans-	µg/L	0.5			<0.5	<0.5
Dichloropropane,1,2-	µg/L	0.5			<0.5	<0.5

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Data Specialist

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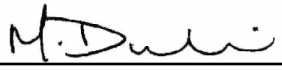
REPORT No: 25-015207 - Rev. 0

Parameter	Units	R.L.	Limits	DWG	Client I.D.	TW1-1	TW1-2
					Sample I.D.	25-015207-1	25-015207-2
					Date Collected	2025-May-29	2025-May-29
					DWG	-	-
Dichloropropene,1,3-cis-	µg/L	0.5				<0.5	<0.5
Dichloropropene,1,3-cis+trans-(Calculated)	µg/L	0.5				<0.5	<0.5
Dichloropropene,1,3-trans-	µg/L	0.5				<0.5	<0.5
Ethylbenzene	µg/L	0.5	140, 1.6	MAC, AO		<0.5	<0.5
Hexane	µg/L	5				<5	<5
Dichloromethane (Methylene Chloride)	µg/L	5	50	MAC		<5	<5
Methyl Ethyl Ketone	µg/L	2				<2	<2
Methyl Isobutyl Ketone	µg/L	20				<20	<20
Methyl tert-Butyl Ether (MTBE)	µg/L	2				<2	<2
Styrene	µg/L	0.5				<0.5	<0.5
Tetrachloroethane,1,1,1,2-	µg/L	0.5				<0.5	<0.5
Tetrachloroethane,1,1,2,2-	µg/L	0.5				<0.5	<0.5
Tetrachloroethylene	µg/L	0.5	10	MAC		<0.5	<0.5
Toluene	µg/L	0.5	60	MAC		<0.5	<0.5
Trichloroethane,1,1,1-	µg/L	0.5				<0.5	<0.5
Trichloroethane,1,1,2-	µg/L	0.5				<0.5	<0.5
Trichloroethylene	µg/L	0.5	5	MAC		<0.5	<0.5
Trichlorofluoromethane (Freon 11)	µg/L	5				<5	<5
Vinyl Chloride	µg/L	0.2	1	MAC		<0.2	<0.2
Xylene, m,p-	µg/L	1				<1	<1
Xylene, m,p,o-	µg/L	1.1	90, 20	MAC, AO		<1.1	<1.1



Michelle Dubien
Data Specialist

CADUCEON Environmental Laboratories Certificate of Analysis

Final Report

REPORT No: 25-015207 - Rev. 0

Parameter	Units	R.L.	Limits	DWG	Client I.D.	TW1-1	TW1-2
					Sample I.D.	25-015207-1	25-015207-2
					Date Collected	2025-May-29	2025-May-29
					-	-	-
Xylene, o-	µg/L	0.5				<0.5	<0.5

DWG - Drinking Water Guidelines
ODWS - Ontario Drinking Water Standards
AO - Aesthetic Objectives
IMAC - Interim Maximum Acceptable Concentration
MAC - Maximum Acceptable Concentration
ODWO - D-5-5 Objective
OG - Operational Guidelines
WL - Warning Level - Sodium Restricted Diets

Summary of Exceedances		
Aesthetic Objectives		
TW1-1	**Found Value**	**Limit**
Turbidity	8.4	5
TDS (Ion Sum Calc)	510	500
TW1-2	**Found Value**	**Limit**
TDS (Calc. from Cond.)	503	500
Iron	0.326	0.3
TDS (Ion Sum Calc)	522	500
Maximum Acceptable Concentration		
TW1-1	**Found Value**	**Limit**
Sodium	38.9	20
TW1-2	**Found Value**	**Limit**
Sodium	40.0	20
Operational Guidelines		
TW1-1	**Found Value**	**Limit**
Hardness (as CaCO3)	389	100
TW1-2	**Found Value**	**Limit**
Hardness (as CaCO3)	394	100
Warning Level - Sodium Restricted Diets		
TW1-1	**Found Value**	**Limit**
Sodium	38.9	20
TW1-2	**Found Value**	**Limit**
Sodium	40.0	20

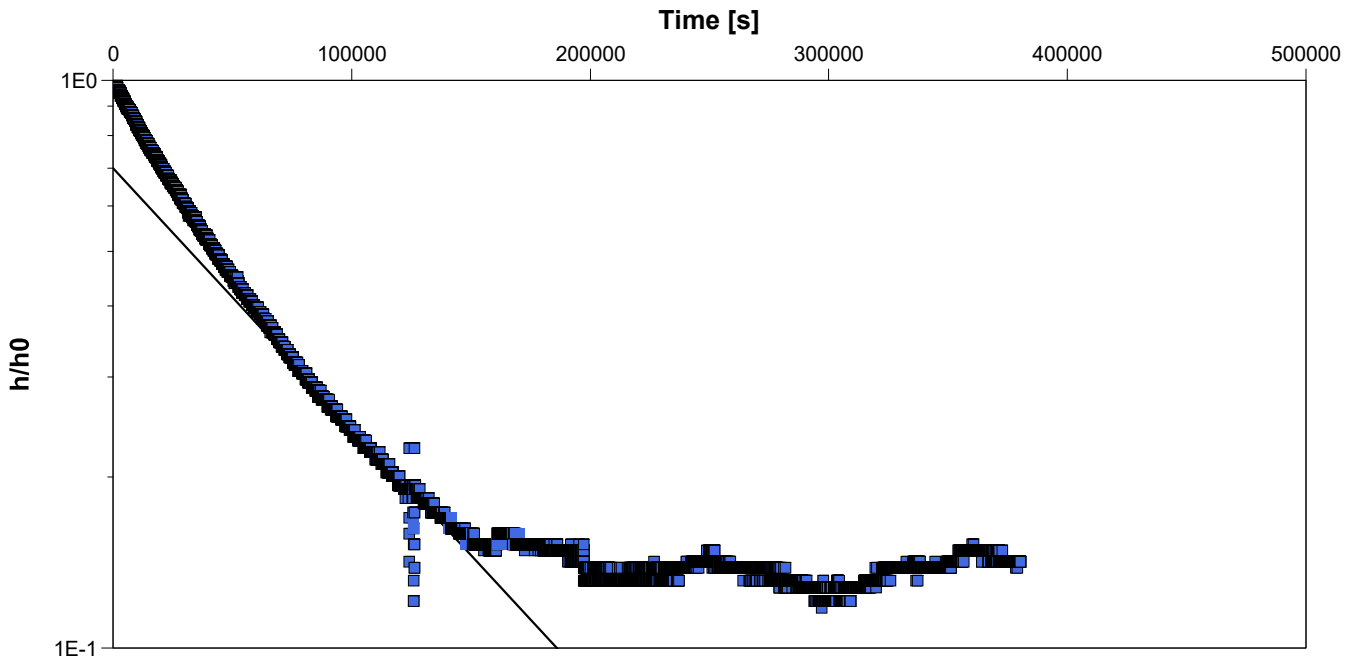
Michelle Dubien
Data Specialist

Appendix F

Single Well Hydraulic Test Results

CAMBIUM	**Slug Test Analysis Report**
	Project: Hydrogeological Assessment
	Number: 17281-002
	Client: Cassidy EW Construction Consultant Ltd.

Location: 1386 & 1394 Greely Lane	Slug Test: BH105 - Slug Test 1	Test Well: BH105-23
Test Conducted by: MC		Test Date: 4/19/2024
Analysis Performed by: NA	Hvorslev	Analysis Date: 7/11/2024
Aquifer Thickness: 2.62 m		

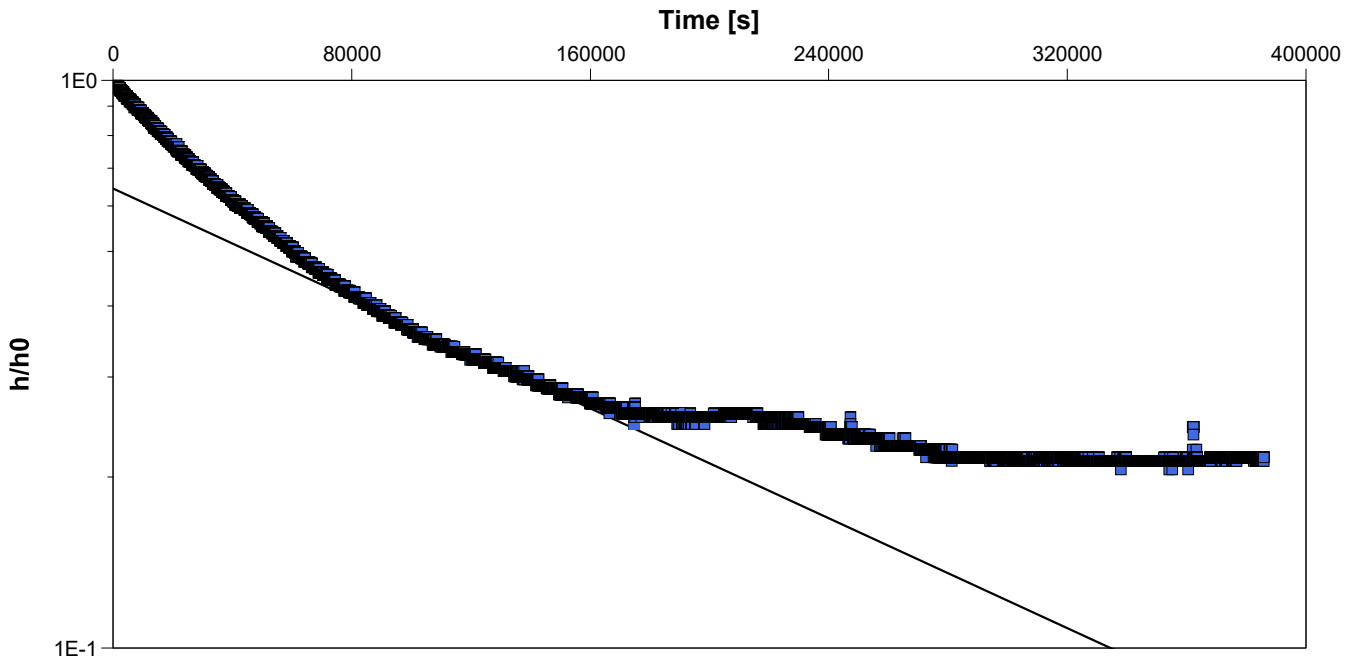


Calculation using Hvorslev

Observation Well	Hydraulic Conductivity [m/s]	
BH105-23	6.35×10^{-9}	

CAMBIUM	**Slug Test Analysis Report**
	Project: Hydrogeological Assessment
	Number: 17281-002
	Client: Cassidy EW Construction Consultant Ltd.

Location: 1386 & 1394 Greely Lane	Slug Test: BH105 - Slug Test 2	Test Well: BH105-23
Test Conducted by: MC		Test Date: 4/19/2024
Analysis Performed by: NA	Hvorslev	Analysis Date: 7/11/2024
Aquifer Thickness: 2.62 m		

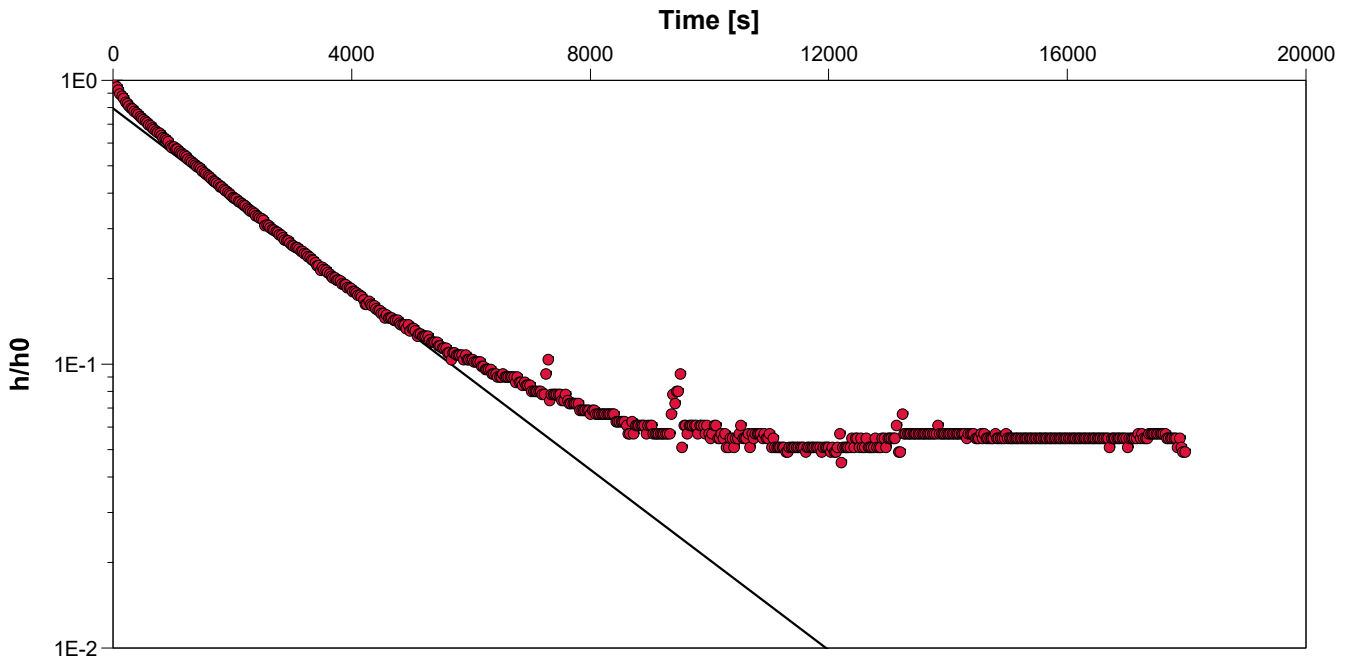


Calculation using Hvorslev

Observation Well	Hydraulic Conductivity [m/s]	
BH105-23	3.38×10^{-9}	

CAMBIUM	**Slug Test Analysis Report**
	Project: Hydrogeological Assessment
	Number: 17281-002
	Client: Cassidy EW Construction Consultant Ltd.

Location: 1386 & 1394 Greely Lane	Slug Test: BH106 - Slug Test 1	Test Well: BH106-23
Test Conducted by: MC		Test Date: 4/19/2024
Analysis Performed by: NA	Hvorslev	Analysis Date: 7/11/2024
Aquifer Thickness: 2.46 m		

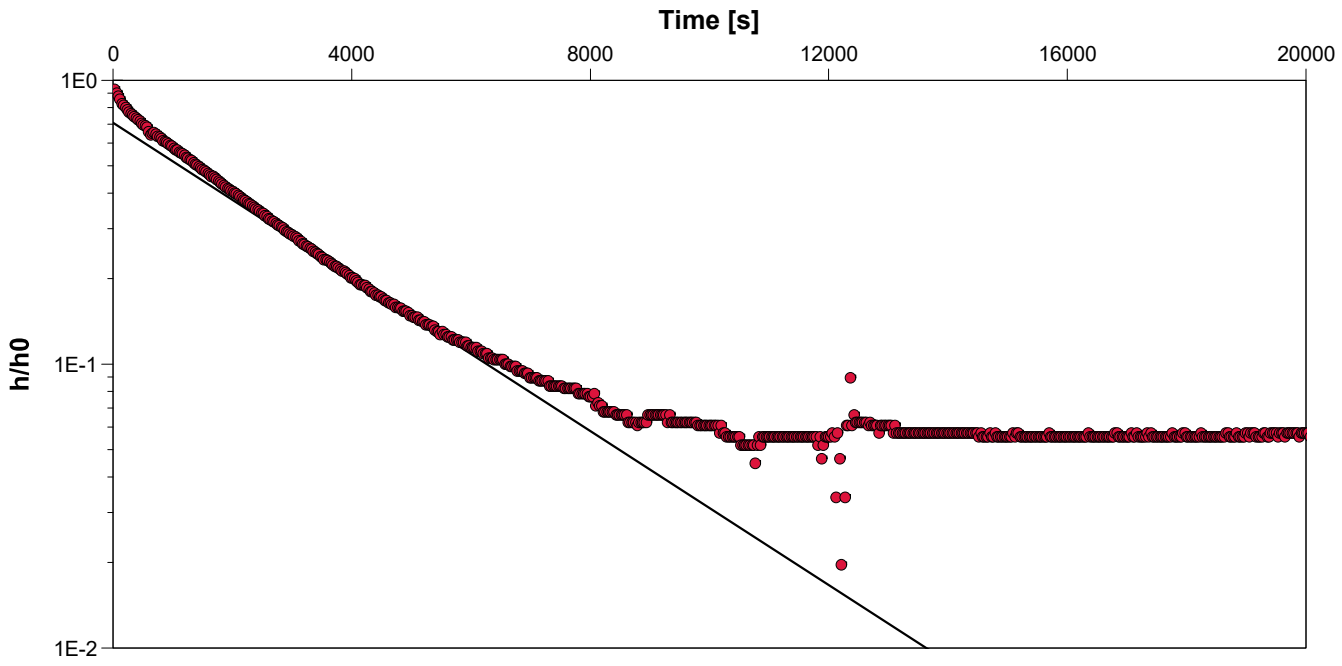


Calculation using Hvorslev

Observation Well	Hydraulic Conductivity [m/s]	
BH106-23	2.22×10^{-7}	

CAMBIUM	**Slug Test Analysis Report**
	Project: Hydrogeological Assessment
	Number: 17281-002
	Client: Cassidy EW Construction Consultant Ltd.

Location: 1386 & 1394 Greely Lane	Slug Test: BH106 - Slug Test 2	Test Well: BH106-23
Test Conducted by: MC		Test Date: 4/19/2024
Analysis Performed by: NA	Hvorslev	Analysis Date: 7/11/2024
Aquifer Thickness: 2.46 m		

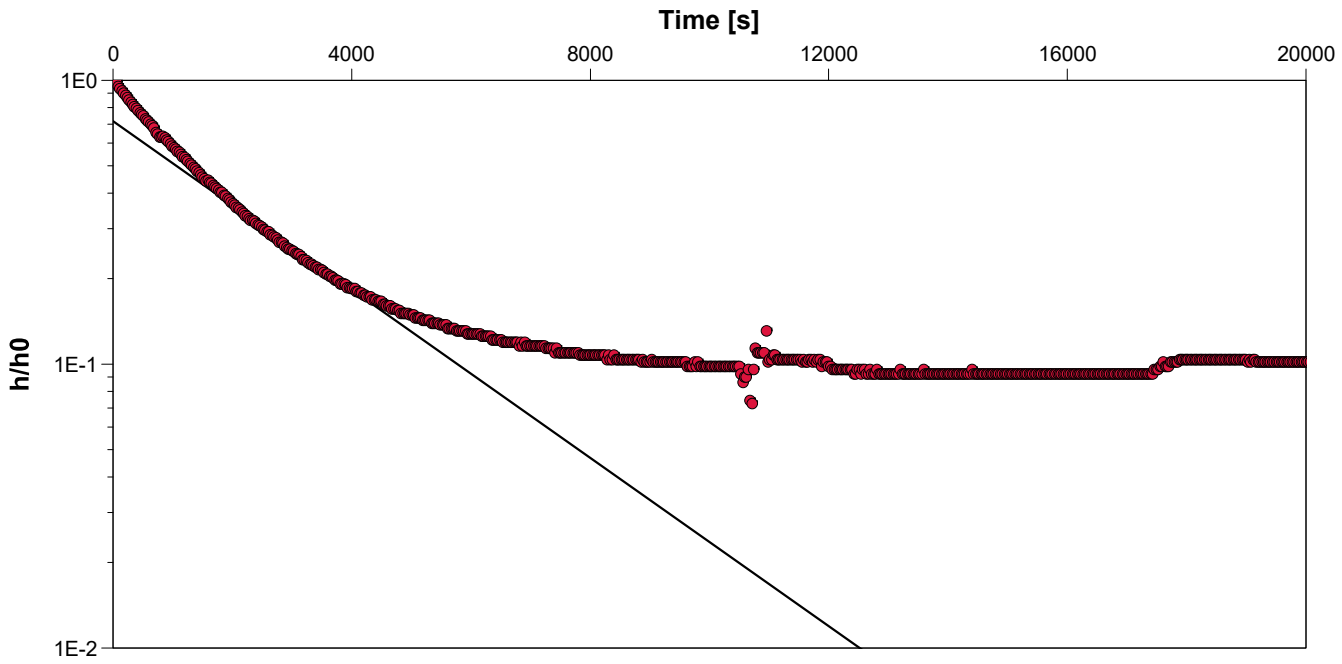


Calculation using Hvorslev

Observation Well	Hydraulic Conductivity [m/s]	
BH106-23	1.90×10^{-7}	

CAMBIUM	**Slug Test Analysis Report**
	Project: Hydrogeological Assessment
	Number: 17281-002
	Client: Cassidy EW Construction Consultant Ltd.

Location: 1386 & 1394 Greely Lane	Slug Test: BH106 - Slug Test 3	Test Well: BH106-23
Test Conducted by: MC		Test Date: 4/19/2024
Analysis Performed by: NA	Hvorslev	Analysis Date: 7/11/2024
Aquifer Thickness: 2.46 m		

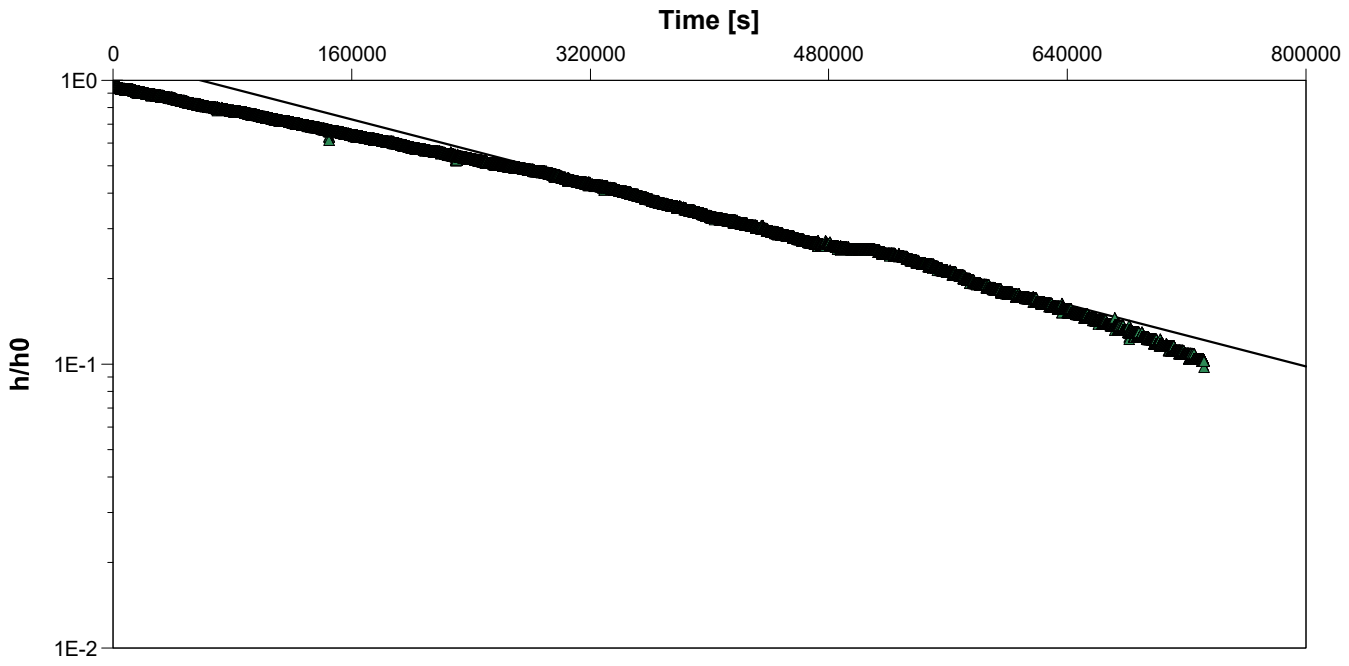


Calculation using Hvorslev

Observation Well	Hydraulic Conductivity [m/s]	
BH106-23	2.07×10^{-7}	

CAMBIUM	**Slug Test Analysis Report**
	Project: Hydrogeological Assessment
	Number: 17281-002
	Client: Cassidy EW Construction Consultant Ltd.

Location: 1386 & 1394 Greely Lane	Slug Test: BH107 - Slug Test 1	Test Well: BH107-23
Test Conducted by: MC		Test Date: 4/19/2024
Analysis Performed by: NA	Hvorslev	Analysis Date: 7/11/2024
Aquifer Thickness: 2.89 m		



Calculation using Hvorslev

Observation Well	Hydraulic Conductivity [m/s]	
BH107-23	1.90×10^{-9}	

Appendix G
Dewatering Calculations

DEWATERING CALCULATIONS - CONSTRUCTION PHASE

Modified Dupuit-Forchheimer Equation: unconfined flow into a linear excavation.
Calculations assume no flow boundary at aquifer base

Excavation Area		Initial Depth to Groundwater	Target Depth to Groundwater	Depth to Base of Aquifer*	Unit Length of Trench (a)	Width of Trench (b)	Hydraulic Conductivity (K)	Drawdown (s)	R	r_w = b/2	R_o	$\ln(R_o/r_w)$	L = R_o/2	H	h = H-s	Q_{ends}	Q_{trench}	Q_{total}		
		mbgs	mbgs	mbgs	m	m	m/s	m	m	m	m	-	m	m	m	m^3/s	m^3/s	m^3/s	L/s	L/d
Elongated Trench @ 50 m Increments	Minimum K	0.30	2.50	3.60	50	2	1.90E-09	2.20	0.29	1.00	1.29	0.25	0.64	3.30	1.10	0.000000	0.000001	**0.000002**	**0.002**	**143**
	Maximum K	0.30	2.50	3.60	50	2	2.06E-07	2.20	2.99	1.00	3.99	1.39	2.00	3.30	1.10	0.000005	0.000050	**0.000054**	**0.05**	**4,702**
	Geometric mean K	0.30	2.50	3.60	50	2	1.22E-08	2.20	0.73	1.00	1.73	0.55	0.86	3.30	1.10	0.000001	0.000007	**0.000008**	**0.01**	**648**

s = target drawdown (initial - target depth to groundwater) (m)
R_o = radius of influence of construction dewatering/pumping, from center of excavation (m)
L = distance to line source (m)
r_s = equivalent single well radius (m)
H = Initial hydraulic head in aquifer (m)
h = hydraulic head at radius of well (m)
Q = construction dewatering rate (m^3/s)
*For base of aquifer, use target depth to groundwater plus 50% of target drawdown (s), unless specific geological conditions dictate otherwise.
For practical use, R is presented as zone of influence for reporting purposes, with the distance defined from edge of excavation.

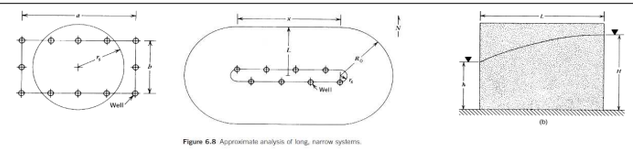


Figure 6.8 Approximate analysis of long, narrow systems.

Source: Powers, J. Patrick, et al. "Construction dewatering and groundwater control." (2007)

$$Q = \frac{\pi K(H^2 - h^2)}{\ln R_o/r_s} + 2\left[\frac{xK(H^2 - h^2)}{2L}\right] \quad (6.10b)$$

x = unit length of trench

R = 3000*s*sqrt(K)
Source: Kyrieleis, W. and Sichardt, W. "Grundwasserabsenkung bei Fundierungsarbeiten" Springer, Berlin, 1930

R_o = R, if R >> r_s ***(R >> rs when R/r s > 100)***
else, R_o = R + r_s

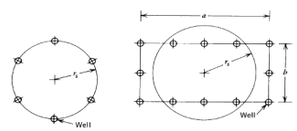
Source: Cashman and Preene. "Groundwater Lowering in Construction." (2013)

DEWATERING CALCULATIONS - OPERATIONAL PHASE

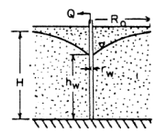
Modified Dupuit-Forchheimer Equation: unconfined flow into a rectangular excavation.
Calculations assume no flow boundary at aquifer base

Excavation Area		Initial Depth to Groundwater	Target Depth to Groundwater	Depth to Base of Aquifer*	Excavation Length (a)	Excavation Width (b)	Hydraulic Conductivity (K)	Drawdown (s)	R	$r_w = \sqrt{(ab/\pi)}$	R_o	$\ln(R_o/r_w)$	H	$h_w = H-s$	Q_{total}		
		mbgs	mbgs	mbgs	m	m	m/s	m	m	m	m	-	m	m	m^3/s	L/s	L/d
Rectangular excavation with dimensions axb	Minimum K	0.30	1.50	3.60	23	55	1.9E-09	1.20	0.16	20.07	20.22	0.01	3.30	2.10	**0.000005**	**0.005**	**429**
	Maximum K	0.30	1.50	3.60	23	55	2.1E-07	1.20	1.63	20.07	21.70	0.08	3.30	2.10	**0.000054**	**0.05**	**4,628**
	Geometric mean K	0.30	1.50	3.60	23	55	1.2E-08	1.20	0.40	20.07	20.46	0.02	3.30	2.10	**0.000013**	**0.01**	**1,093**

s = target drawdown (initial - target depth to groundwater) (m)
R_o = radius of influence of construction dewatering/pumping, from center of excavation (m)
r_s = equivalent single well radius (m)
H = Initial hydraulic head in aquifer (m)
h = hydraulic head at radius of well (m)
Q = construction dewatering rate (m^3/s)
*For base of aquifer, use target depth to groundwater plus 50% of target drawdown (s), unless specific geological conditions dictate otherwise.
For practical use, R is presented as zone of influence for reporting purposes, with the distance defined from edge of excavation.



$$r_s = \sqrt{\frac{ab}{\pi}}$$



Radial flow, water table aquifier

$$Q_w = \frac{\pi K(H^2 - h_w^2)}{\ln R_0/r_w}$$

(from Table 6.1, pg 67)

*Use $r_w = r_s$ for rectangular excavations

Source: Powers, J. Patrick, et al. "Construction dewatering and groundwater control." (2007)

R = 3000*s*sqrt(K)

Source: Kyrieleis, W. and Sichardt, W. "Grundwasserabsenkung bei Fundierungsarbeiten" Springer, Berlin, 1930

R_o = R, if R >> r_s *(R >> rs when R/r_s > 100)*
else, R_o = R + r_s

Source: Cashman and Preene. "Groundwater Lowering in Construction." (2013)

Appendix H

Hydraulic Pumping Test Results

PUMPING TEST DATA SHEET

CAMBIUM

Project Name: Greely Lane
Project Number: 17
Date: May 29. 2025

Staff: Marbett
Contractor: Air Rock Drilling
Weather: Raining. 15°C

Well Name: TW1
Depth of Pump: ~165-170ft
Distance to Pump Well: —
Static Level: 2.34 mbtop
Start Time: 8:03

Diameter: 6'
Stick up:
MP Elevation: —
Geological Unit: —
End Time: 16:03

Time	Elapsed Time	Water Level	Draw Down	Recovery	Discharge Volume	Rate Change	Comments & Observations
	0:30	2.50				~5 gallons/min	
	1:00	2.48					
	1:30	2.48					* talking to driller
	* 3:30	2.505					
	4:00	2.50					
	4:30	2.50					
	5:00	2.50					
	6:00	2.50					
	7:00	2.50					
	8:00	2.50					
	9:00	2.50					
	10:00	2.50					
	12:00	2.50					
	14:00	2.50					
	15:00	Rate change				10 g/min	
	16:00	2.54					
	16:30	2.55					
	17:00	2.56					
	17:30	2.55					
	18:00	2.54					
	19:00	2.535					
	19:30	2.53					
	20:00	2.53					
	26:00	2.56					
	31:00	2.54					
	35:00	2.52					
	40:00	2.52					
	45:00	2.52					
	50:00	2.51					
	55:00	2.51					
	1:00:00	2.51					
	1:30:00	2.51					
	2:00:00	2.50					
	2:30:00	2.49					
	3:00:00	2.49					
	3:30:00	2.45					
	4:00:00	2.49					
	4:30:00	2.49					
	5:00:00	2.45					

PUMPING TEST DATA SHEET

CAMBIUM

Time	Elapsed Time	Water Level	Draw Down	Recovery	Discharge Volume	Rate Change	Comments & Observations
	5:3000	2.41					
	6:00:00	2.39					
	6:30:00	2.37					
	7:00:00	2.36					
	7:30:00	2.35					
	8:00:00	2.34					
	8:04	no check	value so	attempted to create	a	lock	but do not believe I
	~~8:30~~	succeeded					
	1:00	2.17					
	1:30						
	2:00						
	2:30						
	3:00						
	3:30						
	4:00						
	4:30						
	5:00						
	6:00						
	7:00						
	8:00						
	9:00						
	10:00						
	15:00	2.14					

CAMBIUM
Guiding Good Decisions

Project: 17281-001 Date: May 29, 2025
Subject: Breedy Lane P-Test - MW1 and gen chem
Staff: M. Latt
Contact:

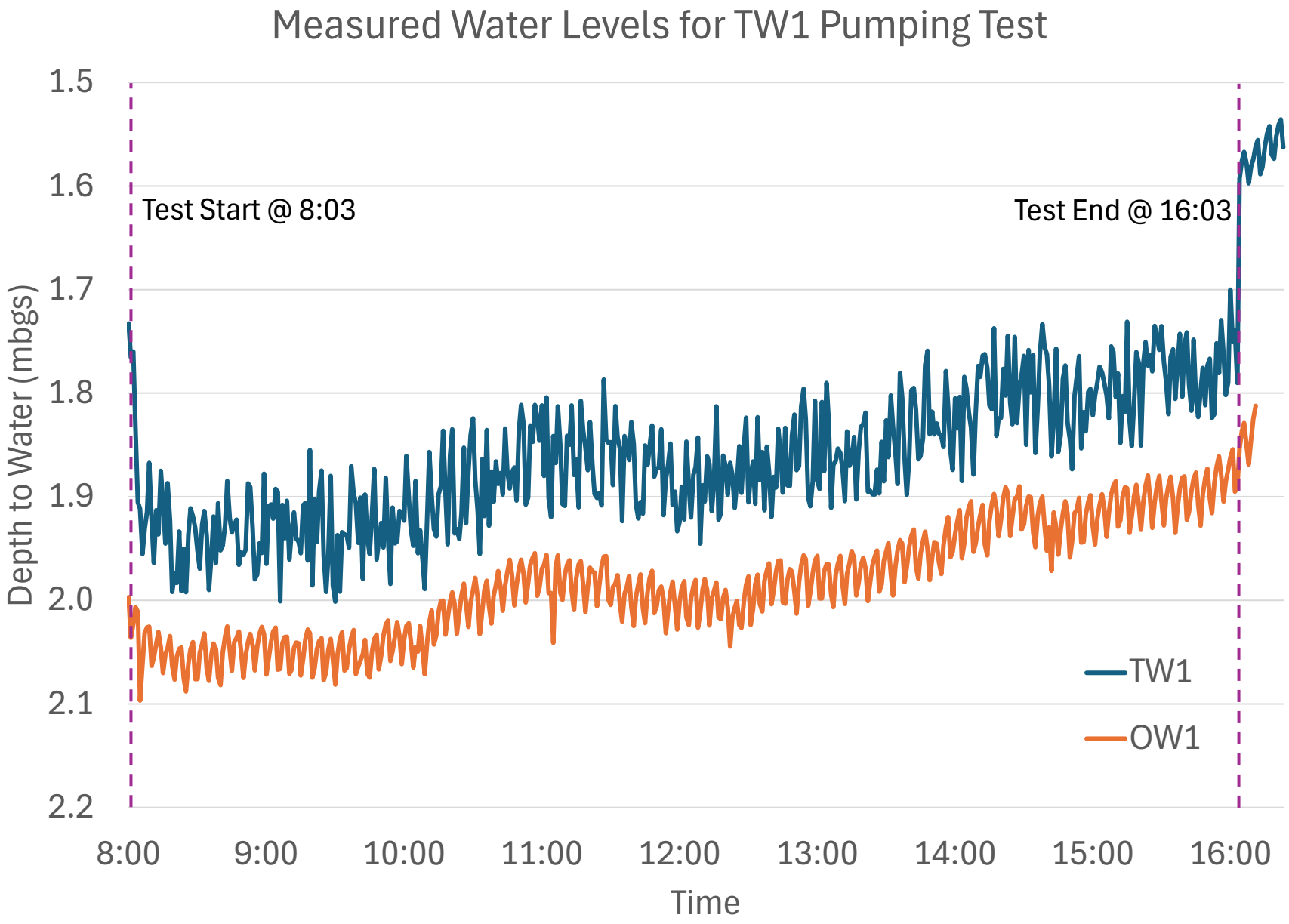
MW1 WL's

test hr	WL mbtoc
Pre	2.28
1	2.28
2	2.28
3	2.28
4	2.21
5	2.19
6	2.16
7	2.12
Post	2.09

TW1 gen. Chem

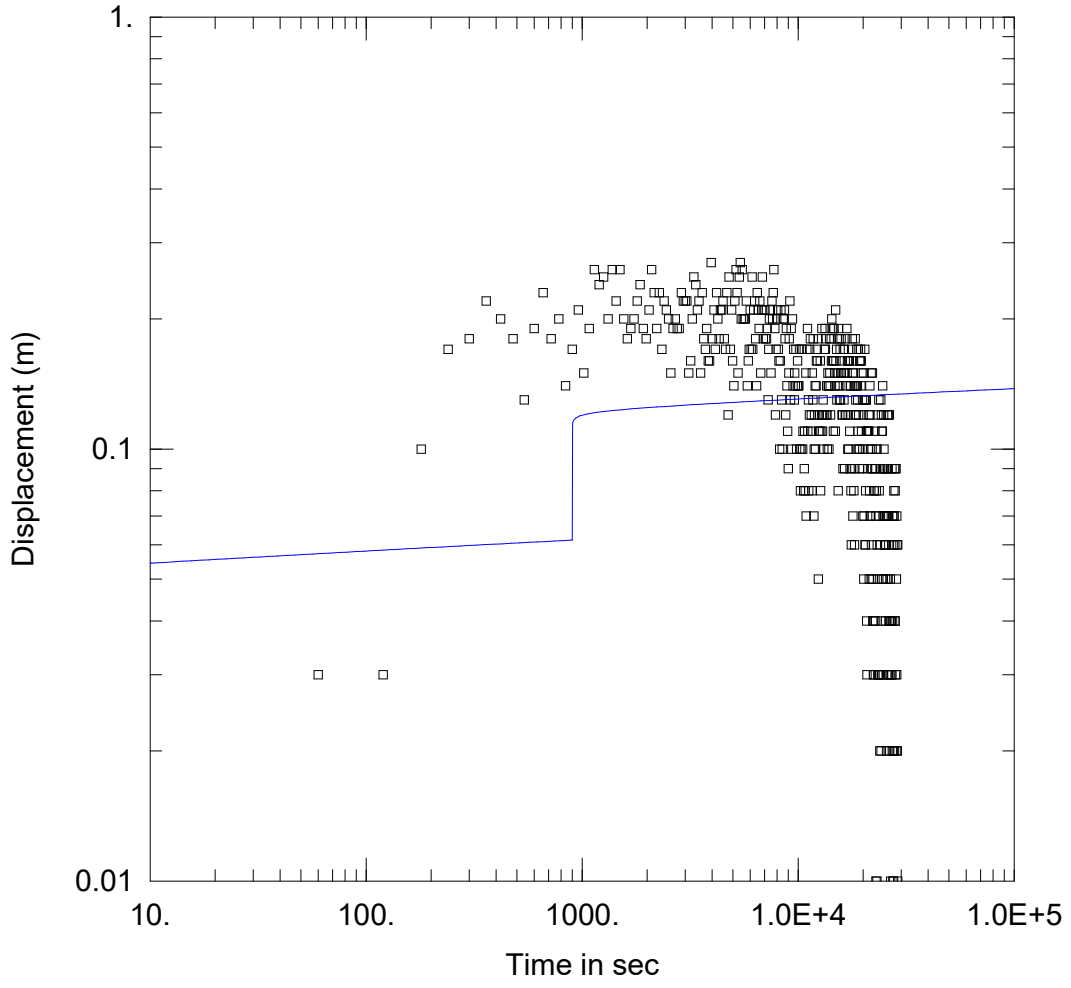
test hr	temp (C)	DO (mg/L)	Cond (µS/cm)	pH	ORP (mV)	Turb (NTU)	Chlor	Comments
1	11.2	1.69	663	7.37	-9.4	4.44	0.00	
2	11.5	1.82	669	7.34	-9.4	5.89	0.00	
3	11.6	1.99	682	7.33	-9.3	7.05	0.00	Sampled TW1-1
4	11.4	1.94	684	7.31	-9.6	5.21	0.00	
5	11.3	1.92	688	7.32	-12.8	3.95	0.00	
6	11.7	1.92	690	7.36	-28.2	2.83	0.00	
7	11.8	1.91	691	7.42	-46.1	2.62	0.00	Sampled TW1-2

test hr	Colour	Odour	Clarity	Sheen
1	none	None	Clear	None
2	↓	↓	↓	↓
3				
4				
5				
6				
7				



TW1 Pumping Test

Prepared By:	Prepared For:
Cambium Inc	Cassidy E.W. Construction
Project:	**Location:**
17281-002	Greely Lane, Ottawa



Data Set:
Date: 06/17/25 Time: 11:22:56

SOLUTION

Aquifer Model: Confined
Solution Method: Theis

T = 0.9563 m^2/sec
S = 1.0E-10
Kz/Kr = 1.
b = 10. m

WELL DATA

Pumping Wells

Well Name	X (m)	Y (m)
TW1	0	0

Observation Wells

Well Name	X (m)	Y (m)
▫ TW1	0	0

Appendix I

Water Balance Calculations and Nitrate Assessment

Water Balance Calculations

1386 and 1394 Greely Lane, Ottawa, Ontario

THORNTHWAITE-TYPE MONTHLY WATER-BALANCE MODEL

modified from Dingman 2015: Box 6-8 (pg 299) using ET model of Hamon (1963)

	Input Data					Computed Values							
											Surplus	397	mm/yr
Weather Station Location:	OTTAWA MACDONALD-CARTIER INT'L A *					**Latitude:**	45.3	degree					
Solar Declination (degree)	-20.6	-12.6	-1.5	10.0	19.0	23.1	21.0	13.4	2.6	-9.0	-18.5	-23.0	
DayLength (hr)*	9.0	10.3	11.8	13.4	14.7	15.4	15.0	13.9	12.4	10.8	9.4	8.6	

Available Water Storage Capacity	0.21	m/m	**Root Depth**	460	mm	**SOILmax** 96.6 mm

MONTHLY WATER BALANCE DATA

Temperatures in C, water-balance terms in mm.

Month:	J	F	M	A	M	J	J	A	S	O	N	D	Year
TEMPERATURE (T)	-10.3	-8.1	-2.3	6.3	13.3	18.5	21.0	19.8	15.0	8.0	1.5	-6.2	
PRECIPITATION (P)	65.4	54.3	64.4	74.5	80.3	92.8	91.9	85.5	90.1	86.1	81.9	76.4	944
RAIN	25.0	18.7	31.1	63.0	80.1	92.8	91.9	85.5	90.1	82.2	64.5	33.5	758
SNOW	40	36	33	12	0	0	0	0	0	4	17	43	185
MELT FACTOR (F)	0.00	0.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.25	0.00	
PACK	96	132	165	0	0	0	0	0	0	0	13	56	
MELT	0	0	0	177	0	0	0	0	0	4	4	0	185
INPUT (W)	25	19	31	240	80	93	92	86	90	86	69	34	944
POTENTIAL ET (PET)	0	0	0	41	73	101	118	101	65	38	21	0	557
NET INPUT (ΔW)	25	19	31	199	8	-8	-26	-16	25	48	48	34	
SOIL MOISTURE (SOIL)	97	97	97	97	97	89	68	58	82	97	97	97	
ΔSOIL	0	0	0	0	0	-8	-21	-10	25	14	0	0	0
ET	0	0	0	41	73	100	113	96	65	38	21	0	547
SURPLUS=W-ET-DSOIL	25	19	31	199	8	0	0	0	0	34	48	34	397

Notes:

Precipitation, Rain, Temperature, and Latitude are inputted parameters

SOILmax = available water storage capacity * root depth

m = month

D = Day length (hrs) $=2*\cos^{-1}(-\tan(\text{Latitude})*\tan(\text{Declination}))/0.2618$ [calculation is in radians]

$SNOW_m = P_m - RAIN_m$

$F_m = 0$ if $T_m <= 0^oC$; $F_m = 0.167*T_m$ if $0^oC<T_m<6^oC$; $F_m = 1$ if $T_m>=6^oC$

$PACK_m = (1-F_m)*(SNOW_m+PACK_{m-1})$

$MELT = F_m*(SNOW_m+PACK_{m-1})$

$W_m = RAIN_m+MELT_m$.

PET = 0 if $T_m<0$; otherwise $PET = 2.98*0.611*\exp(17.3*T_m/(T_m+237))/(T_m+237.2)*$Number of days in month [Hamon ET model (1963)]

$\Delta W_m = W_m - PET_m$

$SOIL = \min\{[\Delta W_m+SOIL_{m-1}], SOILmax\}$, if $\Delta Wm>0$; otherwise $SOIL = SOIL_{m-1} * \exp(\Delta W/SOILmax)$

$\Delta SOIL = SOIL_{m-1} - SOIL_m$

ET = PET if W_m > PET; otherwise, $ET=W_m-\Delta SOIL$

Pre- and Post-Development Water Balance Calculations

1386 AND 1394 Greely Lane, Ottawa, ON

1 Climate Information

Precipitation	944	mm/yr
Actual Evapotranspiration	547	mm/yr
Water Surplus	397	mm/yr

2 Infiltration Rates

Table 2 Approach - Infiltration factors	
Topography: Rolling to Hilly Land	0.15
Soil Type: Silty Loam	0.2
Cover: Cultivated land	0.1
Total Infiltration Factor	**0.45**

Infiltration (Water Surplus * Infiltration Factor)	**179**	mm/yr
Run-off (Water Surplus - Infiltration)	**218**	mm/yr

Table 3 Approach - Typical Recharge Rates		
Coarse Sand and Gravel	>250	mm/yr
Fine to medium sand	200-250	mm/yr
Silty sand to sandy silt	150-200	mm/yr
Silt	125-150	mm/yr
Clayey Silt	100- 125	mm/yr
Clay	<100	mm/yr

Site development area is underlain predominantly by silty sand to clayey silt.

Based on the above, the recharge rate is typically	125-150	mm/yr

3 Pre-Development Property Statistics	**ha**	**m^2**
Total Paved Area	0.08	855
Total Roof Area	0.04	353
Total Landscape Area	0.35	3,472
Total	**0.47**	**4,680**

4 Post-Development Property Statistics	**ha**	**m^2**
Total Paved Area	0.22	2,236
Total Roof Area	0.13	1,261
Total Landscape Area	0.12	1,183
Total	**0.47**	**4,680**

Pre- and Post-Development Water Balance Calculations

1386 AND 1394 Greely Lane, Ottawa, ON

5 Pre-Development Water Balance

Land Use		Area (m^2)	Precipitation (m^3)	Evapotranspiration (m^3)	Infiltration (m^3)	Run-off (m^3)
Impervious Areas	Paved Area	855	807	81	-	726
	Roof Area	353	333	33	-	300
Pervious Areas	Landscape Area	3,472	3,278	1,899	620	758
	Totals	4,680	4,418	2,013	620	1,784

Assuming no infiltration occurring in paved and roof areas, and 10% of precipitation to be evaporated from paved and roof areas.

6 Post-Development Water Balance

Land Use		Area (m^2)	Precipitation (m^3)	Evapotranspiration (m^3)	Infiltration (m^3)	Run-off (m^3)
Impervious Areas	Paved Area	2,236	2,111	211	-	1,900
	Roof Area	1,261	1,190	119	-	1,071
Pervious Areas	Landscape Area	1,183	1,117	647	211	258
	Totals	4,680	4,418	977	211	3,229

Assuming no infiltration occurring in paved and roof areas, and 10% of precipitation to be evaporated from paved and roof areas.

7 Comparision of Pre- and Post -Development

	Precipitation (m^3)	Evapotranspiration (m^3)	Infiltration (m^3)	Run-off (m^3)
Pre-Development	4,418	2,013	620	1,784
Post-Development	4,418	977	211	3,229
Change in Volume	-	- 1,036	- 409	1,445
Change in %	-	- 51	- 66	81

8 Requirement for Infiltration of Roof Run-off

Volume of Pre-Development Infiltration (m^3/yr)	620
Volume of Post-Development Infiltration (m^3/yr)	211
Deficit from Pre to Post Development Infiltration (m^3/yr)	409
Percentage of Roof Runoff required to match the pre-development infiltration (%)	38

Nitrate Attenuation

Input Data			Computed Values

Areas	LOT #1	Total
LOT AREA (m^2)	1183	1183

Surplus water

0.397	m/yr
1.0876712E-03	m/day
1.286715068	m^3/day

Infiltration Factor

Rolling to Hilly Land	0.15
Silty Loam	0.2
Cultivated land	0.1
Total	0.45

Infiltrated water

0.000489452	m/day
0.579021781	m^3/day

Runoff 0.707693288 m^3/day

Combined Concentrations at Property Boundaries

	Without Reduction	Waterloo Biofilter System (80.3% Nitrate Reduction)
Qe	1800	1800
Ce	40	7.88
Qi	579.0218	579.0217808
Ci	0.1	0.1
Qt	2379.022	2379.021781
mg/L	**30.29**	**5.99**

Appendix J

Waterloo Biofilter Supporting Documentation

WaterNOx-LS Third Party Testing Summary

In the fall of 2016, Waterloo Biofilter Systems Inc. installed their WaterNOx-LS™ denitrification unit at the Bureau de Normalisation du Quebec (BNQ) test site located in Quebec City. The system underwent BNQ 3680-600 test protocol which includes two parts - Period A and Period B. Period A is based on the methodology of NSF/ANSI Standards 40 and 245, containing the same flow patterns and stress tests. Period B provides for a further 6 months of seasonal reliability testing to ensure that the test includes cold weather results.

The WaterNOx-LS is a passive autotrophic denitrification process using sulphur-limestone minerals in a submerged, up-flow configuration. The WaterNOx-LS, which was sized for 1,600 L/day (350 gpd) followed a Waterloo Biofilter nitrifying treatment unit.

Period A Test Results

During Period A wastewater is dosed according to the hydraulic loading specified in NSF-40. Period A includes the wash-day, working-parent, power failure, and vacation period stress tests. All sample results taken during stress tests are included in the analysis. Influent wastewater temperature values ranged from 10.0 °C (50 °F) to 16.5 °C (62 °F) with an average value of 13.3 °C (56 °F). Influent pH averaged 7.9 and effluent pH averaged 7.2.

Table 1 – Period A Results for the WaterNOx-LS

Parameters	Influent	Effluent	Removal
(c)BOD_5	260	6	97.6%
TSS	312	3	99.2%
Fecal Coliforms	2,403,000	4,900	99.8%
$NO_{2,3}$	0.08	0.20	
TKN	57.1	4.6	92.0%
TN	57.1	4.8	91.6%

n = 123; n = 357 for fecals

All parameters in mg/L except Fecal Coliforms in cfu/100mL

All values arithmetic averages except Fecal Coliforms in geometric average

Weekly influent total nitrogen concentrations ranged from 43.0 mg/L to 68.8 mg/L with a six-month average concentration of 57.1 mg/L.

Weekly effluent $NO_{2,3}$ concentrations ranged from < 0.02 mg/L to 3.33 mg/L with a six-month average of 0.20 mg/L. Weekly effluent TKN concentrations ranged from 1.5 mg/L to 16.9 mg/L with a six-month average of 4.6 mg/L. Weekly effluent total nitrogen concentrations ranged from 1.7 mg/L to 17.1 mg/L with a six-month average of 4.8 mg/L. The total nitrogen reduction over the six-month period was 91.6%.

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Period B Test Results

Weekday hydraulic loading is modified during Period B to a strenuous 'working parent' schedule where 40% of the flow is delivered over three hours in the morning, and 60% is delivered over three hours in the evening. All samples taken during Period B are included in the analysis. Influent wastewater temperature values ranged from 10.1 °C (50 °F) to 15.8 °C (60 °F) with an average value of 12.3 °C (54 °F). Influent pH averaged 8.0 and effluent pH averaged 7.1.

Table 2 – Period B Results for the WaterNOx-LS

Parameters	Influent	Effluent	Removal
(c)BOD_5	248	4	98.2%
TSS	304	3	99.1%
Fecal Coliforms	2,142,000	2,800	99.9%
$NO_{2,3}$	0.17	3.38	
TKN	60.3	8.5	85.9%
TN	60.4	11.9	80.3%

n = 59 except Fecal Coliforms n = 118

All parameters in mg/L except Fecal Coliforms in cfu/100mL

All values arithmetic averages except Fecal Coliforms in geometric average

Weekly influent total nitrogen concentrations ranged from 21.2 mg/L to 85.6 mg/L with a six-month average concentration of 60.4 mg/L.

Weekly effluent $NO_{2,3}$ concentrations ranged from < 0.04 mg/L to 15.2 mg/L with a six-month average of 3.38 mg/L. Weekly effluent TKN concentrations ranged from 1.2 mg/L to 21.2 mg/L with a weekly average of 8.5 mg/L. Weekly effluent total nitrogen concentrations ranged from 3.7 mg/L to 22.2 mg/L with a six-month average of 11.9 mg/L. The total nitrogen reduction over the six-month period was 80.3%.

Conclusion

In summary, the WaterNOx-LS system can successfully remove very high levels of total nitrogen passively, while buffering pH to neutral and keeping $cBOD_5$ and TSS levels below 10 mg/L.