



**re:** Excavation, Grading and Drainage Plan Review  
Proposed Residential Development  
100 Steacie Drive, Ottawa, Ontario

**to:** Brigil - James Battison - [jamesbattison@brigil.com](mailto:jamesbattison@brigil.com)

**date:** June 1, 2026

**file:** PG5788-MEMO.03 Revision 3

---

Further to your request and authorization, Paterson Group (Paterson) prepared the current memorandum to complete an excavation, grading and site servicing plan review for the proposed residential development at the aforementioned site. The following memorandum should be read in conjunction with the following reports:

- Paterson Group Report PG5788-1 Revision 2, dated September 17, 2025
- Project No. 160401570, Drawing No. GP-1 Revision 8 – Grading Plan, dated February 26, 2026, prepared by Stantec Consulting Ltd.
- Project No.160401570, Drawing No. SSP-1 Revision 8 – Site Servicing Plan, dated March 24, 2026, prepared by Stantec Consulting Ltd.
- Project No. 13264, Drawing No. A200 Revision 2 – Basement Floor Plan, dated December 2, 2024, prepared by NEUF Architects Inc.

## **1.0 Grading Plan Review**

The proposed building is expected to be constructed over conventional footings bearing on bedrock or on piles end bearing on bedrock where the bedrock is deeper, generally within the north and northeastern portions of the site.

Due to the silty clay deposit a permissible grade raise restriction of 2.2 m is applicable for site.

Based on our review of the site grading plans, the north and northeast portions of the site were noted to have grade raise exceedance above the permissible grade raise limits. Since the building's footings are expected to be founded on bedrock, the proposed grade raise exceedance will not negatively impact the building. However, some minor settlement can be expected within the surrounding landscaping areas – including any retaining walls and the patios.

It should also be noted that based on the proposed building elevations, pre-loading will also take place during construction, which will assist with pre-consolidating the underlying clay layer to some extent.

To mitigate the settlement in the landscaping areas, some lightweight (LWF) will be required surrounding the proposed retaining walls and the hardscaped areas located along the north and northeast side of the building. The LWF material should consist of EPS 15.





Reference should be made to the attached grading plan markup and LWF details attached below for the required LWF thickness within the respective areas.

It is not recommended to plant medium to large trees directly above the LWF EPS. The EPS should be covered with a polyethylene sheet and surrounded with a non-woven geotextile such as Terrafix 270R.

A minimum of 200 mm of OPSS Granular A should be placed above the LWF under the hardscaped areas and a minimum 200 mm of approved fill material and a minimum of 100 mm of topsoil should be placed under landscaped areas.

## 2.0 Site Servicing Plan Review

### 2.1 Frost Protection

Based on our review of the above reference site servicing drawings, it was noted that , the proposed watermain will have less than 2.4 m of soil cover available above the pipe at the following locations: **STN 1 + 138.2**, **STN 1 + 141.8** and **STN 0 + 222.8**. As per the City of Ottawa standards, all watermain pipes are recommended to be provided with minimum 2.4 m of soil cover.

Where insufficient soil cover (i.e.- less than 2.4 m) is available for the watermain, the following frost protection criteria outlined in Table 1 is applicable.

<b>Table 1 – Frost Protection for Watermain</b>	
<b>Soil Cover Provided (mm)</b>	<b>Insulation Thickness (mm)</b>
1800 to 2400	50
1500 to 1800	75
1200 to 1500	100

The rigid insulation should be installed in accordance with City of Ottawa Drawing W22. The rigid insulation should be placed 150 mm above the pipe on top of a compacted Granular A backfill and should have a minimum of 150 mm of Granular A or Granular B Type II backfill above the rigid insulation. The rigid insulation should extend a minimum of 150 mm horizontally beyond the edge of the pipe and vertically down to the bottom of the pipe bedding, or alternatively to extend minimum 1.2 m on each side of the pipe.

Further to our review, some of the stormwater services were noted to have less than 2.0 m of soil cover. All the storm and sanitary service pipes should be provided with a minimum of 2.0 m of soil cover for frost protection. Where insufficient soil cover (i.e.- less than 2.0 m) is available for the stormwater pipes, the following frost protection criteria outlined in Table 2 is applicable.





<b>Soil Cover Provided D (mm)</b>	<b>Insulation Dimensions (mm)</b>	
	<b>Thickness (mm)</b>	<b>Length (mm)</b>
800 to 1,100	100	Extend 1,200 mm horizontally beyond the edge face of the pipe
1,100 to 1,400	75	Extend 900 mm horizontally beyond the edge face of the pipe
1,400 to 1,700	50	Extend 600 mm horizontally beyond the edge face of the pipe
1,700 to 2,000	50	Extend 300 mm horizontally beyond the edge face of the pipe

The rigid insulation should be placed 150 mm above the pipe on top of a compacted Granular A backfill and should have a minimum of 150 mm of Granular A or Granular B Type II backfill above the rigid insulation. Rigid insulation placed underneath roadways should consist of minimum HI-40 or SR.P400 equivalent.

## **2.2 Stormwater Pipe Protection**

Based on our review of the Site Servicing plan it was noted that the invert of the stormwater building service connection from STM 1 along the east side of the building, was lower than the proposed USF elevation of the footings. Based on the proposed building loading, it is recommended that the stormwater connection pipe to the building be sleeved and surrounded by minimum 10 MPa lean concrete extending from the bedrock surface to the USF elevation of the building, to transfer the building load to the bedrock.

## **3.0 Proposed Berm**

Based on our review of the above-referenced grading plans a proposed crash berm will be constructed along the north and northeastern portions of the site that is located approximately within 30 m from the existing railway line. It is understood that the proposed berm will be constructed to a height to 2.5 m above ground surface using a brown silty clay fill material. It is expected the maximum slope of the berm will be 40% or approximately 2.5H to 1V. Furthermore, within the northwest portion of the berm, a proposed retaining wall – approximately 1.5 m high, is located above the berm.

Based on the above-noted information, Paterson completed a slope stability analysis, based on one (1) critical cross-section being considered as the worst-case scenario for slope stability.



The slope stability analysis was modelled in SLIDE, a computer program which permits a two-dimensional slope stability analysis calculating several methods including the Bishop's method and Morgenstern-Price Method, which are widely accepted slope analysis methods. The program calculates a factor of safety, which represents the ratio of the forces resisting failure to forces favouring failure. Theoretically, a factor of safety of 1.0 represents a condition where the slope is stable.

However, due to intrinsic limitations of the calculation methods and the variability of the subsurface soil and groundwater conditions, a factor of safety greater than 1.0 is generally required for the failure risk to be considered acceptable. A minimum factor of safety of 1.5 is generally recommended for conditions where the slope failure would comprise permanent structures.

An analysis considering seismic loading was also completed. A horizontal acceleration of 0.15 g was considered for the sections for the seismic loading condition in accordance with OBC 2024. A factor of safety of 1.1 is considered to be satisfactory for stability analyses including seismic loading.

The effective strength soil parameters used for static analysis were chosen based on the subsoil information recovered during the site visit, and our general knowledge of the geology in the area. The effective strength soil parameters used for static condition are presented in Table 1 below.

<b>Table 1 - Effective Strength Soil and Material Parameters (Static Analysis)</b>			
<b>Soil Layer</b>	<b>Unit Weight (kN/m<sup>3</sup>)</b>	<b>Friction Angle (degrees)</b>	<b>Cohesion (kPa)</b>
Topsoil	15	28	3
Brown Silty Clay Crust	18	32	5
Brown Silty Clay Fill	18	29	5
Grey Silty Clay	16	30	5
Glacial Till	18	35	2
Bedrock	22	-	-
Engineered Fill	22	34	0
Lightweight Fill	0.3	-	10

The total strength soil parameters used for seismic analysis were chosen based on the hand excavated holes completed at the time of our site visit, and our general knowledge of the geology in the area. The strength parameters used for seismic condition at the slope cross-sections are presented in Table 2 below:



Table 2 - Effective Strength Soil and Material Parameters (Seismic Analysis)			
Soil Layer	Unit Weight (kN/m <sup>3</sup> )	Friction Angle (degrees)	Cohesion (kPa)
Topsoil	15	28	3
Brown Silty Clay Crust	18	-	100
Brown Silty Clay Fill	18	-	20
Grey Silty Clay	16	-	50
Glacial Till	18	35	2
Bedrock	22	-	-
Engineered Fill	22	34	0
Lightweight Fill	0.3	-	10

The static and seismic analysis results for slope cross-section A are presented in the figures attached below. In summary, the factor of safety for the slope was more than 1.5 under static conditions and more than 1.1 under seismic conditions.

## 4.0 Summary

Provided the above recommendations are taken into consideration, the proposed grading and site servicing plan is acceptable from a geotechnical perspective.

We trust that the current submission meets your immediate requirements.

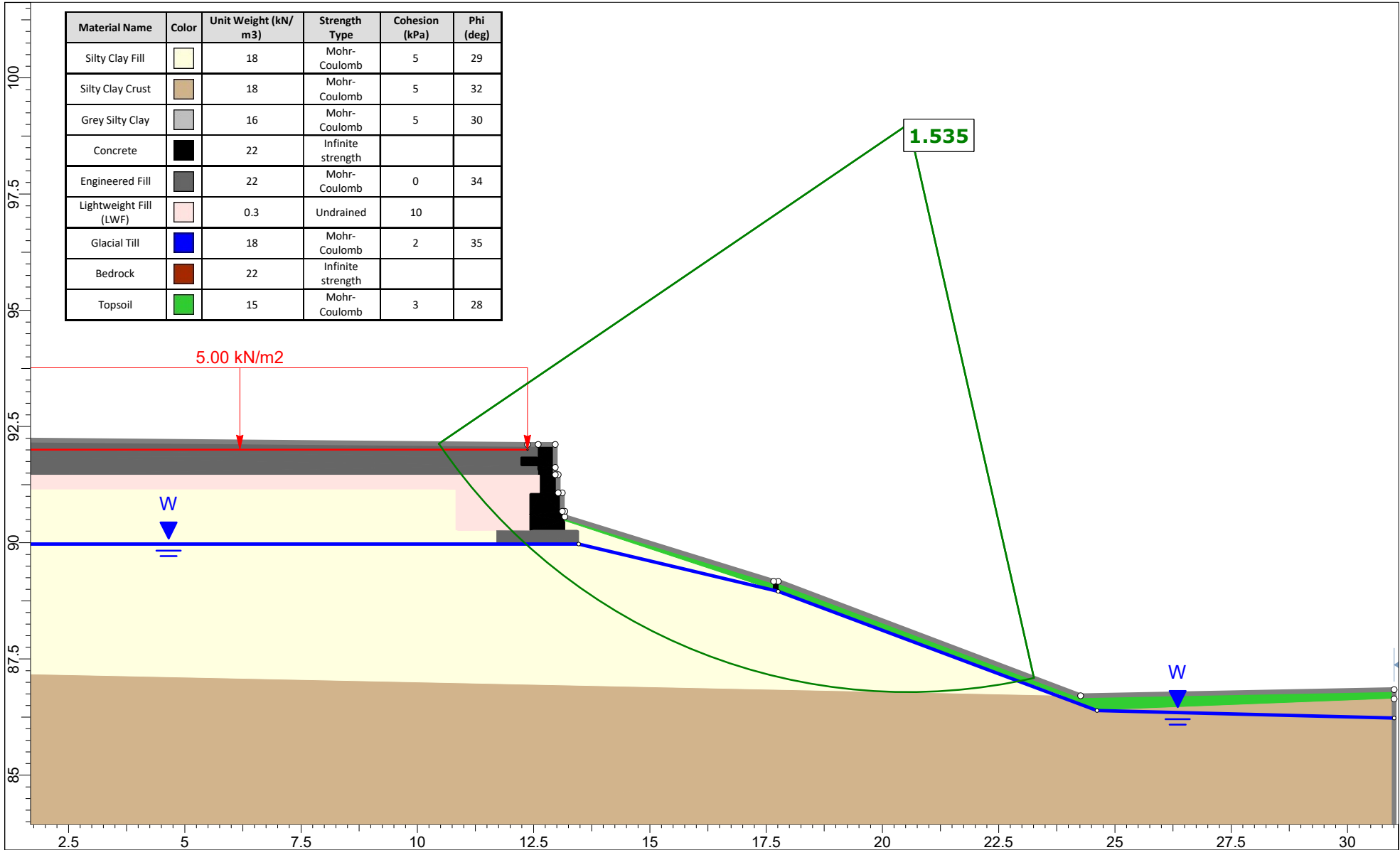
Best Regards,


**Paterson Group Inc.**

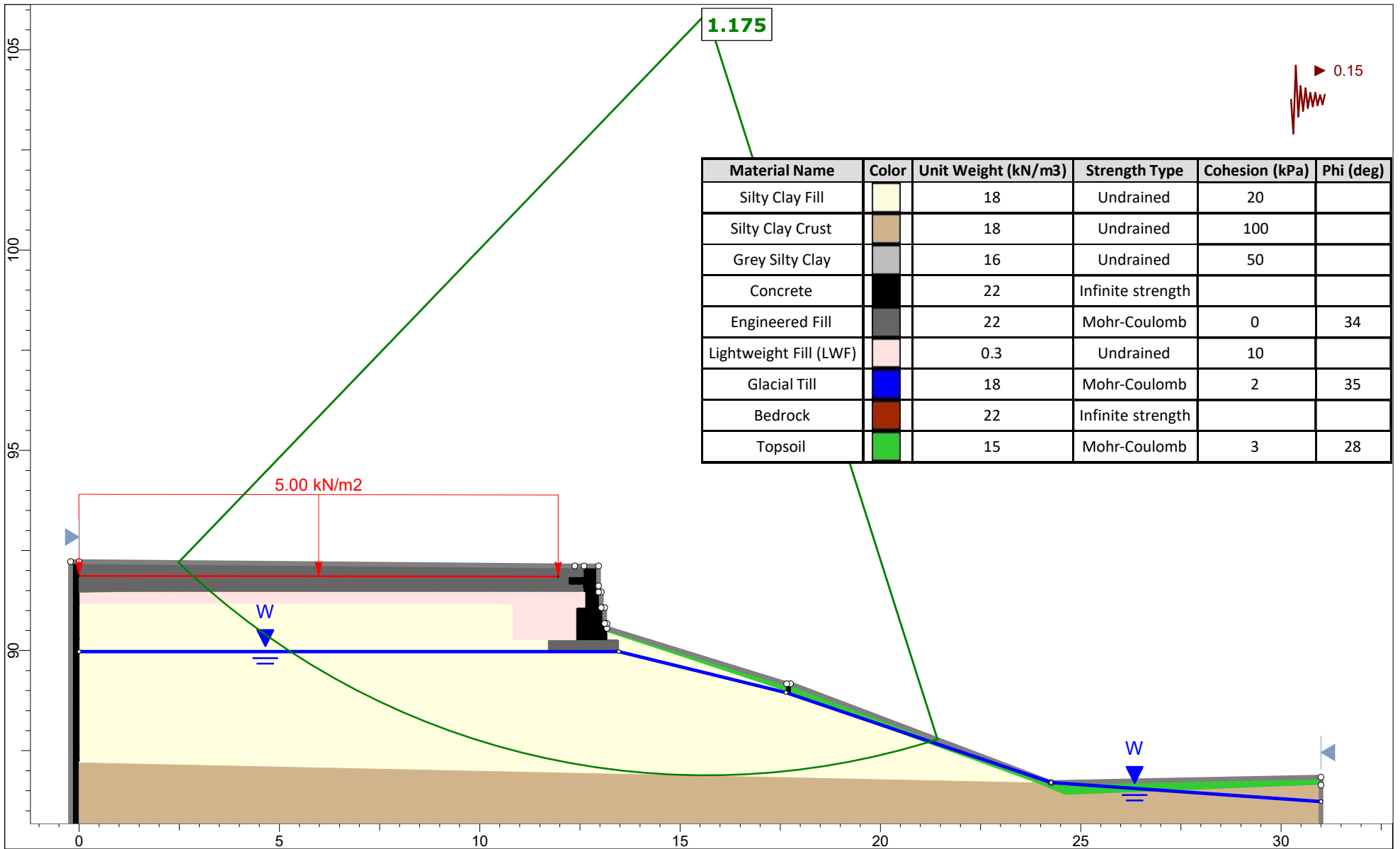
Pratheep Thirumoolan, M.Eng., P.Eng.



Joey R. Villeneuve, M.A.Sc., P.Eng.



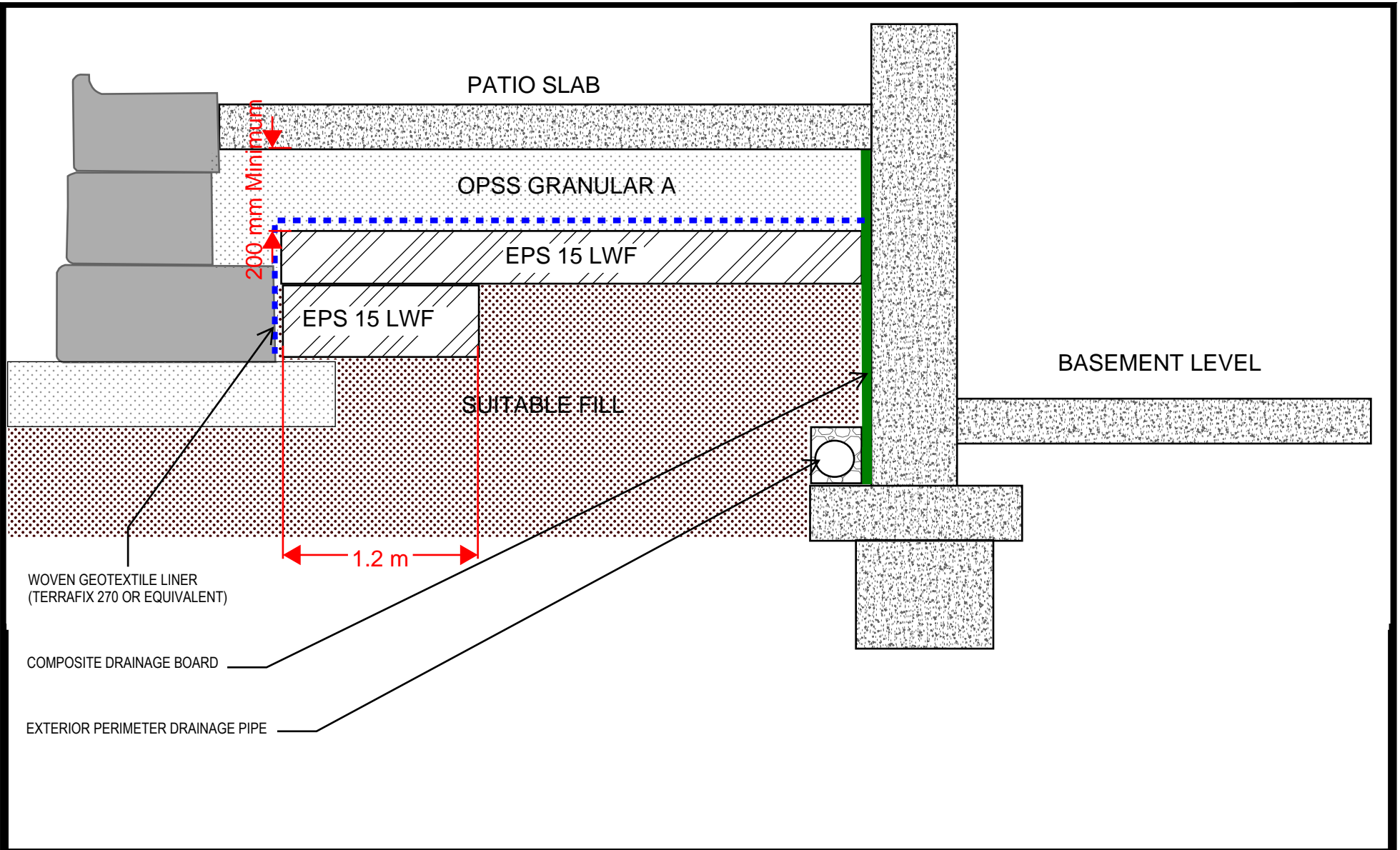
	Project		PG5788 - 100 Steacie Drive	
	Group		Section A - Static Analysis	
	Drawn By	PT	Company	Brigil
	Date	04/2026	File Name	Slope Stability Assessment



Material Name	Color	Unit Weight (kN/m <sup>3</sup> )	Strength Type	Cohesion (kPa)	Phi (deg)
Silty Clay Fill		18	Undrained	20	
Silty Clay Crust		18	Undrained	100	
Grey Silty Clay		16	Undrained	50	
Concrete		22	Infinite strength		
Engineered Fill		22	Mohr-Coulomb	0	34
Lightweight Fill (LWF)		0.3	Undrained	10	
Glacial Till		18	Mohr-Coulomb	2	35
Bedrock		22	Infinite strength		
Topsoil		15	Mohr-Coulomb	3	28



Project	PG5788 - 100 Steacie Drive		
Group	Section A - Seismic Analysis		
Drawn By	PT	Company	Brigil
Date	04/2026	File Name	Slope Stability Assessment



9 AURIGA DRIVE  
OTTAWA, ON  
K2E 7T9  
TEL: (613) 226-7381

**BRIGIL  
GRADING PLAN REVIEW  
100 STEACIE DRIVE**

OTTAWA, ONTARIO

**LIGHTWEIGHT FILL DETAIL**

Scale:	N.T.S.	Date:	09/2025
Drawn by:	PT	Report No.:	PG5788-MEMO.03
Checked by:	PT	Drawing No.:	<b>FIGURE 1</b>
Approved by:	JV	Revision No.:	

