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- Commercial & Institutional
- Environmental Restoration

2175 Prince of Wales Drive Ottawa, ON

Serviceability Report

**2175 PRINCE OF WALES DRIVE
OTTAWA, ONTARIO**

SERVICEABILITY REPORT

Prepared By:

NOVATECH

Suite 200, 240 Michael Cowpland Drive
Ottawa, Ontario
K2M 1P6

September 2015

Revised: July 17, 2024

Revised: March 13, 2026

Ref: R-2015-035

Novatech File: 117074 (formerly 107005)

March 13, 2026

City of Ottawa
Planning and Growth Management Department
4th Floor
110 Laurier Avenue West
Ottawa, Ontario
K1P 1J1

Attention: Kimberley Baldwin, Planner III

Dear Ms. Baldwin:

**Re: 2175 Prince of Wales Drive
Serviceability Report
Our File No.: 117074 (formerly 107005)**

Please find enclosed the Serviceability Report for the above noted property. This report has been revised per City comments and is submitted in support of a Zoning Amendment Application for the property at 2175 Prince of Wales Drive.

If you have any questions, please contact the undersigned.

Sincerely,

NOVATECH



Cara Ruddle, P.Eng.
Senior Project Manager
Land Development

cc:

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1.0. INTRODUCTION

Novatech has been retained by Zena Investment Corporation to prepare a Serviceability Report in support of a Zoning Amendment Application. The site is located at 2175 Prince of Wales Drive within the City of Ottawa. **Figure 1** is a Key Plan showing the site location.

2.0. EXISTING AND PROPOSED DEVELOPMENT

The property is approximately 3.22 hectares in size. The property is currently undeveloped vacant land with 2 existing accesses from Prince of Wales Drive and 2 existing accesses from Waterbend Lane. The topography of the site is relatively flat with a gentle slope towards the east side of the site. The slope increases along the east side of the property adjacent to the Rideau River, it also increases as it drains to an existing watercourse on the north side of the property outletting to the Rideau River. The site is bounded by West Hunt Club Rd and the existing watercourse to the north, Rideau River to the east, residences fronting on Waterbend Lane to the south, and Prince of Wales Drive to the West. **Figure 2** shows the existing conditions.

The proposed development is not defined at this time. It is anticipated that commercial or light industrial use buildings will be constructed. There are a number of setbacks or constraints which are outlined in the Environmental Impact Study for proposed development at 2175 Prince of Wales Drive, Nepean, Ontario, Prepared by Kilgour & Associates Ltd., dated October 17, 2023, Project #Myers 1534. Figure 5 Developable Land and Setback Requirements On-site from the EIS Report shows the various setbacks and the developable land for the subject site and is included for reference. The area of the developable land is 1.94 hectares and this area will be used in the calculations in this report since the remainder of the property will remain untouched.

The purpose of this report is to demonstrate that commercial and/or light industrial uses can be serviced with the existing Municipal infrastructure surrounding the subject site.

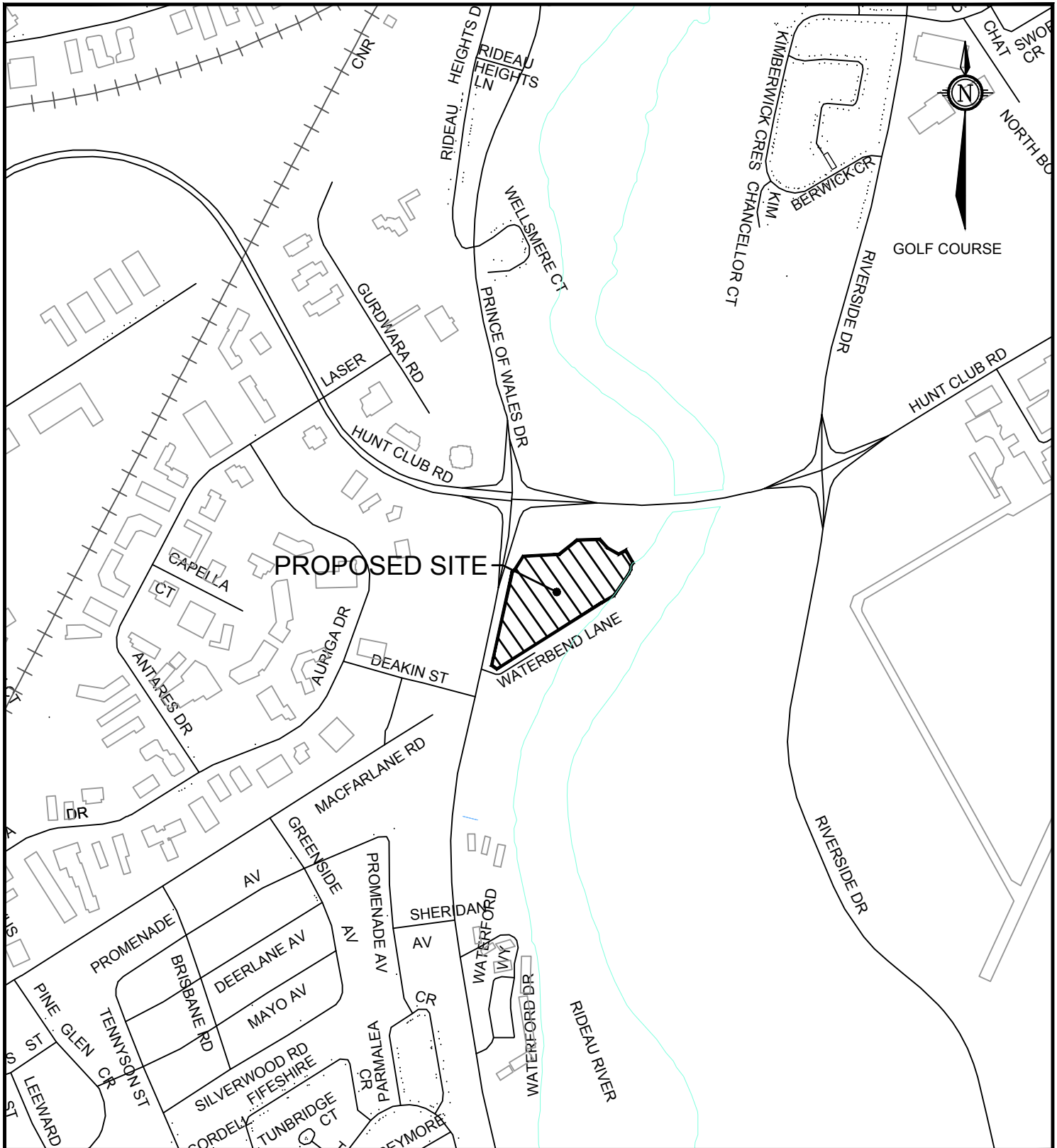
3.0. WATER SERVICING

There is an existing 400mm diameter watermain within the Prince of Wales right-of-way and a 150mm diameter watermain in Waterbend Lane. A portion of the City watermain mapping is provided in Appendix A for reference.

A water service connection could be made to either of the two existing watermains noted above to service the subject property. Calculations have been prepared for both industrial and commercial uses. Preliminary water demands and fire flows have been calculated in the following **Table 3.0**. Refer to **Figure 3** Conceptual Servicing Plan for the existing watermain information and proposed service locations.

Table 3.0 Water Demand Summary

| Use | Ave. Daily Demand (L/s) | Max. Daily Demand (L/s) | Peak Hour Demand (L/s) | Fire Flow (L/s) |
|-----------------------|-------------------------|-------------------------|------------------------|-----------------|
| Industrial Use | 2.05 | 3.08 | 5.54 | 250 |
| Commercial Use | 1.04 | 1.56 | 2.81 | 217 |



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 Facsimile (613) 254-5867
 Website www.novatech-eng.com

2175 PRINCE OF WALES
 DRIVE

KEY PLAN

SCALE

NTS

DATE

JUL 2024

JOB

117074

FIGURE

1

M:\2017\117074\CAD\Design\Report\Figures\117074-FIG2.dwg, EXCOND, Jul 12, 2024 - 2:26pm, macedo



AURIGA DRIVE

DEAKEN STREET
MCFARLAND ROAD

PRINCE OF WALES

WATER BEND LANE

HUNT CLUB DRIVE

RIVERSIDE DRIVE

PROPOSED SITE

NOVATECH

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2175 PRINCE OF WALES
DRIVE

EXISTING CONDITIONS

SCALE 1 : 2500'

DATE JUL 2024 JOB 117074 FIGURE 2



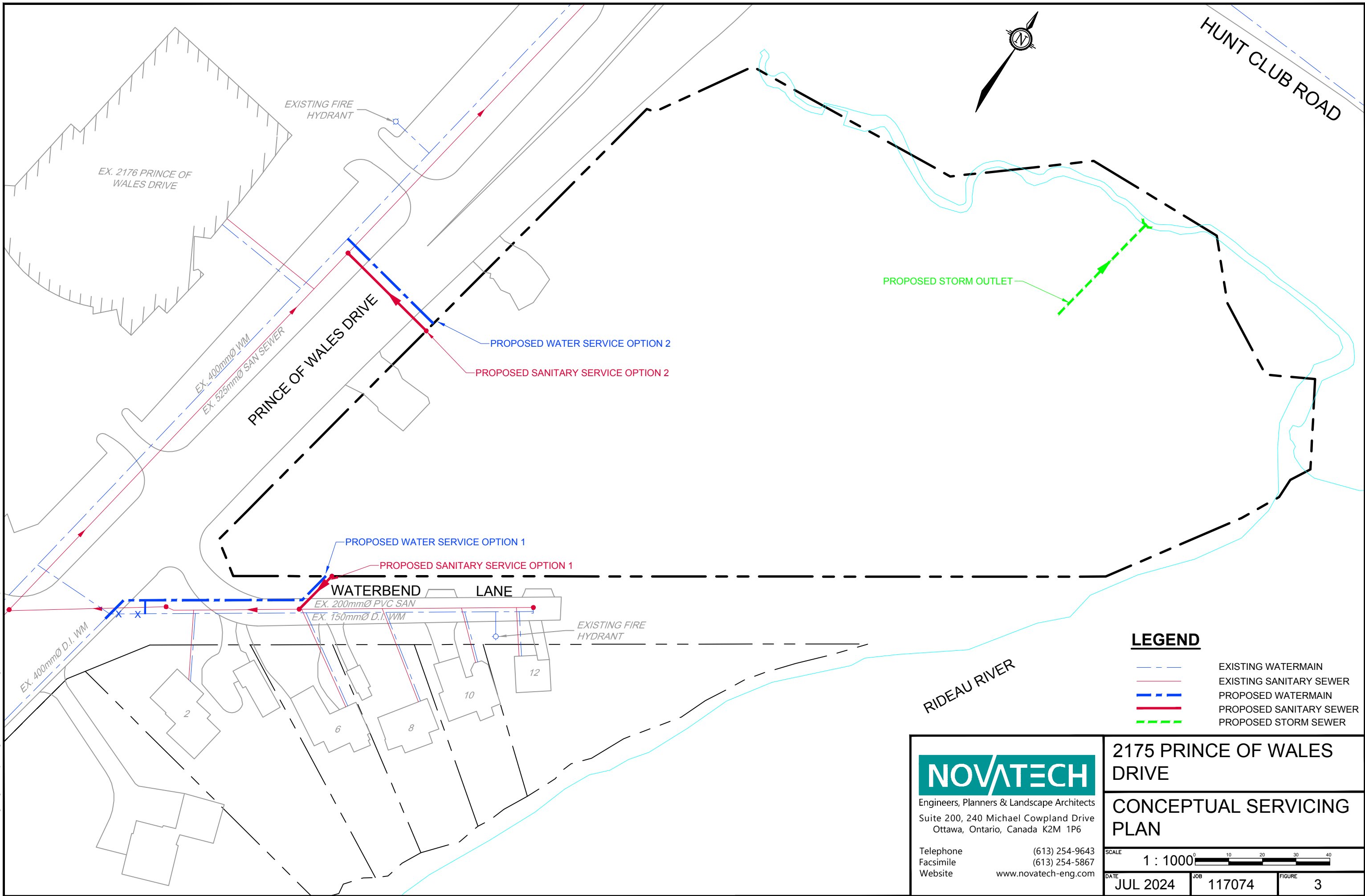
Figure 5 Developable Land and Setback Requirements on Site

REFERENCE: Environmental Impact Study for proposed development at 2175 Prince of Wales Drive, Nepean, Ontario

BY: KILGOUR & ASSOCIATES LTD.

DATED: 2023-10-17

M:\2017\117074\CAD\Design\Figures\Report Figures\117074-FIG4.dwg, GS, Jul 12, 2024 - 2:38pm, madeoif



LEGEND

- EXISTING WATERMAIN
- EXISTING SANITARY SEWER
- PROPOSED WATERMAIN
- PROPOSED SANITARY SEWER
- PROPOSED STORM SEWER



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2175 PRINCE OF WALES DRIVE

CONCEPTUAL SERVICING PLAN

SCALE 1 : 1000

DATE JUL 2024 JOB 117074 FIGURE 3

The maximum water demands were provided to the City and the boundary conditions found in **Table 3.1** below were provided for both the 150mm watermain in Waterbend Lane and the 400mm watermain in Prince of Wales Drive. Correspondence is provided in **Appendix A** for reference.

Table 3.1 Boundary Conditions

| Criteria | Head (m) |
|--|----------|
| Connection 1 to Existing 150mm Watermain in Waterbend Lane. | |
| Minimum HGL | 124.2 |
| Maximum HGL | 132.0 |
| Max Day + Fire Flow (89L/s) | 101.1m |
| Max Day + Fire Flow (91L/s) | 101.1m |
| Connection 2 to Existing 400mm Watermain in Prince of Wales Dr. | |
| Minimum HGL | 124.2m |
| Maximum HGL | 132.0m |
| Max Day + Fire Flow (250L/s) | 126.9m |
| Max Day + Fire Flow (217L/s) | 126.5m |

Preliminary calculations were performed to establish the approximate limits of the design operating pressures. Refer to **Appendix A** for calculation details. A summary of the water results is presented in **Table 3.2** below.

Table 3.2 Water Analysis Results Summary

| Condition | Street | Demand (L/s) | Min/Max Allowable Operating Pressures (psi) | Limits of Design Operating Pressures (psi) |
|--|-----------------|--------------|---|--|
| High Pressure | Waterbend | 2.05 | 80psi (Max) | 63.9 |
| | Prince of Wales | | | 63.9 |
| Maximum Daily Demand and <i>Fire Flow</i> Industrial / Commercial | Waterbend | 89 / 91 | 20psi (Min) | 20.0 / 20.0 |
| | Prince of Wales | 250 / 217 | | 56.7 / 56.1 |
| Peak Hour | Waterbend | 5.54 | 40psi (Min) | 52.9 |
| | Prince of Wales | | | 52.9 |

Based on the boundary conditions provided by the City, the watermain along Prince of Wales could provide adequate water supply for domestic water and fire flow demands while the watermain along Waterbend Lane could only provide adequate water supply for domestic water demand. If a connection is made to the existing watermain along Waterbend Lane, a dry hydrant or a fire pump and an onsite fire suppression water storage tank will be needed to meet the fire flow pressure demand. There is also the option to upsize a portion of the existing watermain along Waterbend Lane to a 200mm or 300mm diameter watermain to service the property.

Additionally, a multi-hydrate approach to firefighting has been reviewed to provide fire flow to the proposed development. There are currently 2 Class AA (blue bonnet) municipal fire hydrants within 150m of the site. And private hydrant will be required to service the development. Refer to **Figure 4** Hydrant Coverage Plan in **Appendix A** for the hydrant locations and assumed distances from site. This would be reviewed in more detail when a defined development concept has been prepared during the Site Plan Approval process.

The site is serviceable from the existing municipal watermain system.

4.0. SANITARY SERVICING

There is an existing 525mm diameter sanitary sewer within the Prince of Wales right-of-way and an existing 200mm diameter sanitary sewer within the Waterbend Lane right-of-way. A portion of the City sewer mapping is included in Appendix B for reference.

Flows for the proposed development have been estimated for both industrial and commercial uses and are 11.0 L/s and 2.0 L/s respectively. Sanitary flow calculations are also included in Appendix B for reference.

The subject property can be serviced with a connection to either of the two existing sanitary sewers noted above. **Figure 3** Conceptual Servicing Plan shows the two servicing options. The existing sanitary sewer along Waterbend Lane only services the 5 existing residential dwellings along this road and has excess capacity to service the subject property. This existing sewer drains into the existing 525mm diameter sanitary sewer along Prince of Wales Drive. The 525mm diameter sanitary sewer along Prince of Wales Drive is considered a collector sewer and a direct service connection will likely not be the City's preferred alternative.

The site is serviceable from the existing municipal sanitary sewer system and the recommended option is to connect to the existing sanitary sewer along Waterbend Lane.

5.0. STORM SERVICING

As indicated previously the site is currently undeveloped vacant land. Stormwater currently sheet drains to the Rideau River along the east side of the property and to the existing watercourse along the north side of the property that drains into the Rideau River. The existing watercourse along the north property limit is an outlet for an existing 1650mm diameter trunk storm sewer servicing an area west of Prince of Wales Drive. A portion of the City sewer mapping is included in Appendix C for reference.

Development of the subject property would cause an increase in the existing storm runoff from the property. Therefore, stormwater management is required including both quality and quantity control. However, due to the close proximity to the Rideau River, it is recommended that storm flows be released directly from the site without quantity control. Storm flows from the site would outlet into the Rideau River well before the natural peak occurs in the river from the upstream watershed. Infiltration and LID measures could be included in the site design depending on the suitability of the existing soils and elevation of the ground water table. This would be reviewed in more detail during the site plan application process.

The site will continue to outlet to the Rideau River in post-development conditions. During detailed design, the velocity of the stormwater being released will need to be reviewed to ensure there are no negative impacts or erosive effects to the receiving watercourse. Refer to **Figure 3** Conceptual Servicing Plan for a proposed storm outlet location.

Quality control of stormwater is also required for 80% removal of total suspended solids (TSS). Quality control of stormwater can be provided through a combination of methods including an oil grit separator (OGS) unit, best management practices and low impact development techniques.

More details on these measures would be provided during the detailed design and site plan application process.

Preliminary stormwater calculations have been prepared. The pre-development release rate is calculated to be 112.4 L/s and 240.8 L/s for the 5 and 100 year storm events respectively. Post-development flows are estimated to be 477.6 L/s and 914.9 L/s for the 5 and 100 year storm events respectively. A drainage area plan and stormwater calculations are provided in **Appendix C** for reference.

The 100-year flood elevation for the Rideau River by the site is 77.05m (taken from Figure 9a in the RVCA Rideau River Flood Risk Mapping (Hogs Back to Kars)). The top of slope along the north side of the site by the watercourse is approximately 86.00m (9m from top of slope to 100 year water elevation). Therefore, it is anticipated that the proposed development will have minimal impact on the capacity of the outlet water course. The impacts of flooding and downstream erosion will be reviewed in more detail during the site plan application process and if required mitigative measures can be proposed.

During storms, in excess of the 100 year storm event, the existing overland flow route is to the Rideau River. This overland flow route would be maintained and incorporated into the detailed design of the subject property.

Therefore, the site is serviceable in terms of storm servicing and stormwater management. Once a defined development plan has been prepared, detailed stormwater calculations can be provided.

6.0. EROSION AND SEDIMENT CONTROL MEASURES

Temporary erosion and sediment control measures will be implemented on-site during construction in accordance with the Best Management Practices for Erosion and Sediment Control. This includes the following temporary measures:

- Filter bags will be placed under the grates of nearby catchbasins and manholes, and will remain in place until vegetation has been established and construction is completed;
- Silt fencing will be placed along the surrounding construction limits;
- Mud mats will be installed at the site entrances;
- The contractor will be required to perform regular street sweeping and cleaning as required, to suppress dust and to provide safe and clean roadways adjacent to the construction site;

The erosion and sediment control measures will be implemented prior to construction and will remain in place during all phases of construction. Regular inspection and maintenance of the erosion control measures will be undertaken

7.0. CONCLUSIONS AND RECOMMENDATIONS

The conclusions of this report are as follows:

- Water servicing, including both domestic and fire protection, can be provided by connection to the existing watermain infrastructure within Prince of Wales Drive. The watermain within Waterbend Lane can provide servicing for domestic flows and some additional measures may be required for fire protection depending on the type of development.
- Sanitary servicing can be provided from the existing sanitary sewer within Waterbend Lane or Prince of Wales Drive.
- Stormwater can outlet, as per existing conditions, to the Rideau River or the existing watercourse along the north property limit.
- Stormwater management is required for the development however, quantity control is not recommended given the close proximity of the site to the Rideau River. The velocity of stormwater will be reviewed as part of the detailed design to ensure there are no negative impacts to the receiving watercourse.
- Quality control of stormwater can be provided through the installation of an oil grit separator unit prior to release of stormwater from the site.
- An overland flow route will be provided.
- Erosion and sediment control measures will be implemented during construction.

NOVATECH

Prepared by:

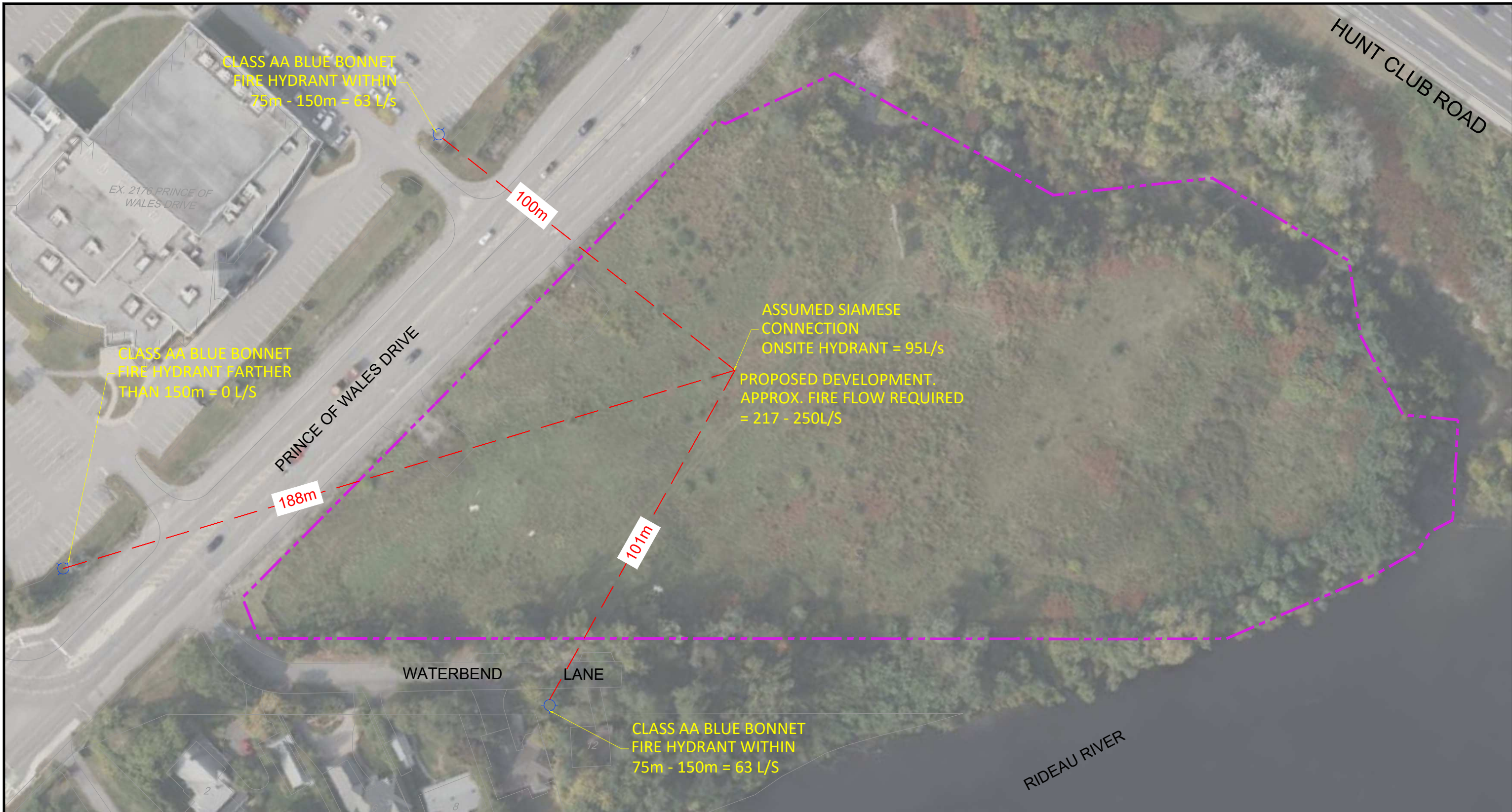


Cara Ruddle, P.Eng.
Project Manager

Reviewed by:

J. Lee Sheets, CET
Sr. Project Manager

APPENDIX A
Watermain Information



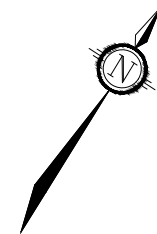
LEGEND

--- DISTANCE FROM HYDRANT TO SIAMESE CONNECTION

--- PROPERTY LINE



EXISTING CLASS AA HYDRANT



NOVATECH

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Website www.novatech-eng.com

2175 PRINCE OF WALES DRIVE

HYDRANT COVERAGE PLAN

SCALE 1 : 1000

| | | |
|----------|--------|--------|
| DATE | JOB | FIGURE |
| MAR 2026 | 117074 | 4 |

FUS - Fire Flow Calculations



Novatech Project #: 117074
Project Name: 2175 Prince of Wales Drive
Date: 5/31/2024
Input By: Leonel Perez
Reviewed By: Cara Ruddle
Drawing Reference:

Legend: Input by User
 No Input Required
Reference: Fire Underwriter's Survey Guideline (2020)
 Formula Method

Building Description: Multi-Storey Industrial Building
Type II - Non-combustible construction

| Step | | Choose | | Value Used | Total Fire Flow (L/min) | |
|---|--|---|--------------------|----------------------------|-------------------------|-------|
| Base Fire Flow | | | | | | |
| 1 | Construction Material | | Multiplier | | 0.8 | |
| | Coefficient related to type of construction C | Type V - Wood frame | | 1.5 | | |
| | | Type IV - Mass Timber | | Varies | | |
| | | Type III - Ordinary construction | | 1 | | |
| | | Type II - Non-combustible construction | Yes | 0.8 | | |
| Type I - Fire resistive construction (2 hrs) | | | 0.6 | | | |
| 2 | Floor Area | | | | 25,000 | |
| | A | Building Footprint (m ²) | 8000 | | | |
| | | Number of Floors/Storeys | 3 | | | |
| | | Protected Openings (1 hr) if C<1.0 | No | | | |
| | | Area of structure considered (m ²) | | 20,000 | | |
| F | Base fire flow without reductions | | | | | |
| Reductions or Surcharges | | | | | | |
| 3 | Occupancy hazard reduction or surcharge | | FUS Table 3 | Reduction/Surcharge | 25,000 | |
| | (1) | Non-combustible | | -25% | | |
| | | Limited combustible | | -15% | | |
| | | Combustible | Yes | 0% | | |
| | | Free burning | | 15% | | |
| Rapid burning | | | 25% | | | |
| 4 | Sprinkler Reduction | | FUS Table 4 | Reduction | -10,000 | |
| | (2) | Adequately Designed System (NFPA 13) | Yes | -30% | | |
| | | Standard Water Supply | Yes | -10% | | |
| | | Fully Supervised System | No | -10% | | |
| | | Cumulative Sub-Total | | | | -40% |
| Area of Sprinklered Coverage (m²) | | 24000 | 100% | | | |
| Cumulative Total | | | -40% | | | |
| 5 | Exposure Surcharge | | FUS Table 5 | Surcharge | 0 | |
| | (3) | North Side | >30m | 0% | | |
| | | East Side | >30m | 0% | | |
| | | South Side | >30m | 0% | | |
| | | West Side | >30m | 0% | | |
| Cumulative Total | | | 0% | | | |
| Results | | | | | | |
| 6 | (1) + (2) + (3) | Total Required Fire Flow, rounded to nearest 1000L/min | | L/min | 15,000 | |
| | | (2,000 L/min < Fire Flow < 45,000 L/min) | | or | L/s | 250 |
| | | | | or | USGPM | 3,963 |

FUS - Fire Flow Calculations



Novatech Project #: 117074
Project Name: 2175 Prince of Wales Drive
Date: 5/31/2024
Input By: Leonel Perez
Reviewed By: Cara Ruddle
Drawing Reference:

Legend: Input by User
 No Input Required
Reference: Fire Underwriter's Survey Guideline (2020)
 Formula Method

Building Description: Multi-Storey Commercial Building
Type I - Fire resistive construction (2 hrs)


| Step | | Choose | | Value Used | Total Fire Flow (L/min) | |
|--|--|---|--------------------|----------------------------|-------------------------|-------|
| Base Fire Flow | | | | | | |
| 1 | Construction Material | | Multiplier | | 0.6 | |
| | Coefficient related to type of construction C | Type V - Wood frame | | 1.5 | | |
| | | Type IV - Mass Timber | | Varies | | |
| | | Type III - Ordinary construction | | 1 | | |
| | | Type II - Non-combustible construction | | 0.8 | | |
| Type I - Fire resistive construction (2 hrs) | | Yes | 0.6 | | | |
| 2 | Floor Area | | | | 20,000 | |
| | A | Building Footprint (m ²) | 8000 | | | |
| | | Number of Floors/Storeys | 4 | | | |
| | | Protected Openings (1 hr) if C<1.0 | No | | | |
| | | Area of structure considered (m ²) | | 24,000 | | |
| F | Base fire flow without reductions | | | | | |
| Reductions or Surcharges | | | | | | |
| 3 | Occupancy hazard reduction or surcharge | | FUS Table 3 | Reduction/Surcharge | 20,000 | |
| | (1) | Non-combustible | | -25% | | |
| | | Limited combustible | | -15% | | |
| | | Combustible | Yes | 0% | | |
| | | Free burning | | 15% | | |
| Rapid burning | | | 25% | | | |
| 4 | Sprinkler Reduction | | FUS Table 4 | Reduction | -7,500 | |
| | (2) | Adequately Designed System (NFPA 13) | Yes | -30% | | |
| | | Standard Water Supply | Yes | -10% | | |
| | | Fully Supervised System | Yes | -10% | | |
| | | Cumulative Sub-Total | | | | -50% |
| | Area of Sprinklered Coverage (m²) | 24000 | 75% | | | |
| | | Cumulative Total | | -38% | | |
| 5 | Exposure Surcharge | | FUS Table 5 | Surcharge | 0 | |
| | (3) | North Side | >30m | 0% | | |
| | | East Side | >30m | 0% | | |
| | | South Side | >30m | 0% | | |
| | | West Side | >30m | 0% | | |
| | | Cumulative Total | | 0% | | |
| Results | | | | | | |
| 6 | (1) + (2) + (3) | Total Required Fire Flow, rounded to nearest 1000L/min | | L/min | 13,000 | |
| | | (2,000 L/min < Fire Flow < 45,000 L/min) | | or | L/s | 217 |
| | | | | or | USGPM | 3,435 |

FW: 2175 Prince of Wales Drive (107005) - Water boundary conditions request

Cara Ruddle <c.ruddle@novatech-eng.com>

Mon 7/8/2024 3:41 PM

To: Micheal Adeoti <m.adeoti@novatech-eng.com>; Jesse Appiah-Kubi <j.appiah-kubi@novatech-eng.com>

 1 attachments (644 KB)

2175 Prince of Wales Drive June 2024.pdf;

Please update the calculations and Serviceability Report based on the below water boundary conditions.

Thanks.

Cara Ruddle, P.Eng., Senior Project Manager | Land Development Engineering**NOVATECH**

Engineers, Planners & Landscape Architects

240 Michael Cowpland Drive, Suite 200, Ottawa, ON, K2M 1P6 | Tel: 613.254.9643 Ext: 220 | Cell: 613.261.7719

The information contained in this email message is confidential and is for exclusive use of the addressee.

From: Brault, Ryan <ryan.brault@ottawa.ca>**Sent:** Wednesday, July 3, 2024 5:29 PM**To:** Cara Ruddle <c.ruddle@novatech-eng.com>**Subject:** RE: 2175 Prince of Wales Drive (107005) - Water boundary conditions request

Hi Cara,

Please find the below boundary conditions for the 2175 Prince of Wales Drive site:

The following are boundary conditions, HGL, for hydraulic analysis at 2175 Prince of Wales Drive (zone 2W2C) assumed to be connected to the 406mm watermain on Prince of Wales Drive or the 152mm watermain on Waterbend Lane (see attached PDF for location).

Scenario 1 (Industrial):

Minimum HGL = 124.2 m

Maximum HGL = 132.0 m

Max Day + Fire Flow (250 L/s) at Prince of Wales Drive connection = 126.9 m

Max Day + Available Fire Flow at 152mm on Waterbend Lane is 89 L/s with a residual pressure of 20 psi

Scenario 2 (Commercial):

Minimum HGL = 124.2 m

Maximum HGL = 132.0 m

200Max Day + Fire Flow (217 L/s) at Prince of Wales Drive connection = 126.5 m

Max Day + Available Fire Flow at 152mm on Waterbend Lane is 91 L/s with a residual pressure of 20 psi

These are for current conditions and are based on computer model simulation.

Disclaimer: The boundary condition information is based on current operation of the city water distribution system. The computer model simulation is based on the best information available at the time. The operation of the water distribution system can change on a regular basis, resulting in a variation in boundary conditions. The physical properties of watermains deteriorate over time, as such must be assumed in the absence of actual field test data. The variation in physical watermain properties can therefore alter the results of the computer model simulation. Fire Flow analysis is a reflection of available flow in the watermain; there may be additional restrictions that occur between the watermain and the hydrant that the model cannot take into account.

Please note that the Fire Flow is not met at connection 2 (Waterbend Lane).

Please let me know if you have any questions.

Regards,

Ryan Brault, M.Eng., P.Eng

Project Manager - Infrastructure Approvals

City of Ottawa
Development Review - West Branch
Planning, Development, and Building Services
110 Laurier Ave West, 4th Floor East;
Ottawa ON K1P 1J1
Tel: 613-580-2424 x 32540

From: Cara Ruddle <c.ruddle@novatech-eng.com>

Sent: July 03, 2024 10:41 AM

To: Brault, Ryan <ryan.brault@ottawa.ca>

Subject: RE: 2175 Prince of Wales Drive (107005) - Water boundary conditions request

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ATTENTION : Ce courriel provient d'un expéditeur externe. Ne cliquez sur aucun lien et n'ouvrez pas de pièce jointe, excepté si vous connaissez l'expéditeur.

That is great news.

Thanks.

Cara Ruddle, P.Eng., Senior Project Manager | Land Development Engineering

NOVATECH

Engineers, Planners & Landscape Architects

240 Michael Cowpland Drive, Suite 200, Ottawa, ON, K2M 1P6 | Tel: 613.254.9643 Ext: 220 | Cell: 613.261.7719

The information contained in this email message is confidential and is for exclusive use of the addressee.

From: Brault, Ryan <ryan.brault@ottawa.ca>
Sent: Wednesday, July 3, 2024 10:38 AM
To: Cara Ruddle <c.ruddle@novatech-eng.com>
Subject: RE: 2175 Prince of Wales Drive (107005) - Water boundary conditions request

Hi Cara,

I actually received them yesterday but sent a question back to my colleagues to confirm one of the notes they provided.

You should have them by end of day today.

Regards,

Ryan Brault, M.Eng., P.Eng
Project Manager - Infrastructure Approvals

City of Ottawa
Development Review - West Branch
Planning, Development, and Building Services
110 Laurier Ave West, 4th Floor East;
Ottawa ON K1P 1J1
Tel: 613-580-2424 x 32540

From: Cara Ruddle <c.ruddle@novatech-eng.com>
Sent: July 03, 2024 10:01 AM
To: Brault, Ryan <ryan.brault@ottawa.ca>
Subject: RE: 2175 Prince of Wales Drive (107005) - Water boundary conditions request

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ATTENTION : Ce courriel provient d'un expéditeur externe. Ne cliquez sur aucun lien et n'ouvrez pas de pièce jointe, excepté si vous connaissez l'expéditeur.

Ryan:
Just thought I would try to get an update on the timing for these boundary conditions so I can give an update to my client.

Thanks.

Cara Ruddle, P.Eng., Senior Project Manager | Land Development Engineering

NOVATECH

Engineers, Planners & Landscape Architects
240 Michael Cowpland Drive, Suite 200, Ottawa, ON, K2M 1P6 | Tel: 613.254.9643 Ext: 220 | Cell: 613.261.7719
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From: Schaeffer, Gabrielle <gabrielle.schaeffer@Ottawa.ca>
Sent: Friday, June 14, 2024 11:04 AM
To: Cara Ruddle <c.ruddle@novatech-eng.com>; Brault, Ryan <ryan.brault@ottawa.ca>
Subject: RE: 2175 Prince of Wales Drive (107005) - Water boundary conditions request

Hi Cara,

Ryan Brault will be helping me with this file and will review your BC request. Once he's done his review, he will send it to the City's water modelling team to get you the BC information. This part may take up to 2 weeks after Ryan's review. If he has any questions, he'll reach out to you.

Regards,

Gabrielle (Gabi) Schaeffer, P.Eng
Senior Engineer - Infrastructure Applications

City of Ottawa
Development Review - West Branch
Planning, Development and Building Department
110 Laurier Ave West, 4th Floor East;
Ottawa ON K1P 1J1
Tel: 613-580-2424 x 22517

From: Cara Ruddle <c.ruddle@novatech-eng.com>
Sent: June 14, 2024 8:48 AM
To: Schaeffer, Gabrielle <gabrielle.schaeffer@Ottawa.ca>
Subject: 2175 Prince of Wales Drive (107005) - Water boundary conditions request

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Gabrielle:

Please find attached information for water demands for a proposed development at 2175 Prince of Wales. I am looking to get boundary conditions in order to update a Serviceability Report to support a Zoning Amendment Application. In addition to the water demand and FUS calculations, attached is a portion of the GeoOttawa mapping showing the existing watermain infrastructure surrounding the subject site. Please note that the property could be serviced with either commercial or light industrial use and an FUS calculation has been provided for both uses. Also, a water service could be connected to either the existing 400mm diameter watermain along Prince of Wales or the 150mm diameter watermain along Waterbend Lane. Please provide updated boundaries conditions as soon as possible so that our report can be updated and submitted to the City for review and approval.

Thanks.

Cara Ruddle, P.Eng., Senior Project Manager | Land Development Engineering

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Name : 2175 - Prince of Wales

Date : July 10/2024

Project : 117074

By : MA

400mm WM (Prince of Wales)

* Assumed Finished
grade = 87.0 ±

Min HGL - 124.2m

Max HGL - 132.0m

Max Day + Fire flow - 126.9m (Industrial)

Max Day + Fire flow - 126.5m (Commercial)

Max Day - 308 L/s

Fire flow - 250 L/s (Industrial)

Fire flow - 217 L/s (Commercial)

$$\text{Avg Day} = 132.0\text{m} - 87.0\text{m} = 45\text{m} \\ \approx 63.9\text{ psi}$$

$$\text{Peak Hour} = 124.2\text{m} - 87.0\text{m} = 37.2\text{m} \\ \approx 52.9\text{ psi}$$

$$\text{Max Day + FF (Industrial)} = 126.9\text{m} - 87.0\text{m} = 39.9\text{m} \\ \approx 56.7\text{ psi}$$

$$\text{Max Day + FF (Commercial)} = 126.5\text{m} - 87.0\text{m} = 39.5\text{m} \\ \approx 56.1\text{ psi}$$

150 mm WM (Water bend)

$$\text{Min HGL} = 124.2 \text{ m}$$

$$\text{Max HGL} = 132.0 \text{ m}$$

$$\text{(Industrial) Max Day + Fire flow} = 89 \text{ L/s (Residual 20 psi)}$$

$$\text{(Commercial) Max Day + Fire flow} = 91 \text{ L/s (Residual 20 psi)}$$

$$\begin{aligned} \text{Avg. Day} &= 132.0 \text{ m} - 87.0 \text{ m} = 45 \text{ m} \\ &\approx 63.9 \text{ psi} \end{aligned}$$

$$\begin{aligned} \text{Peak Hour} &= 124.2 \text{ m} - 87.0 \text{ m} = 37.2 \text{ m} \\ &\approx 52.9 \text{ psi} \end{aligned}$$

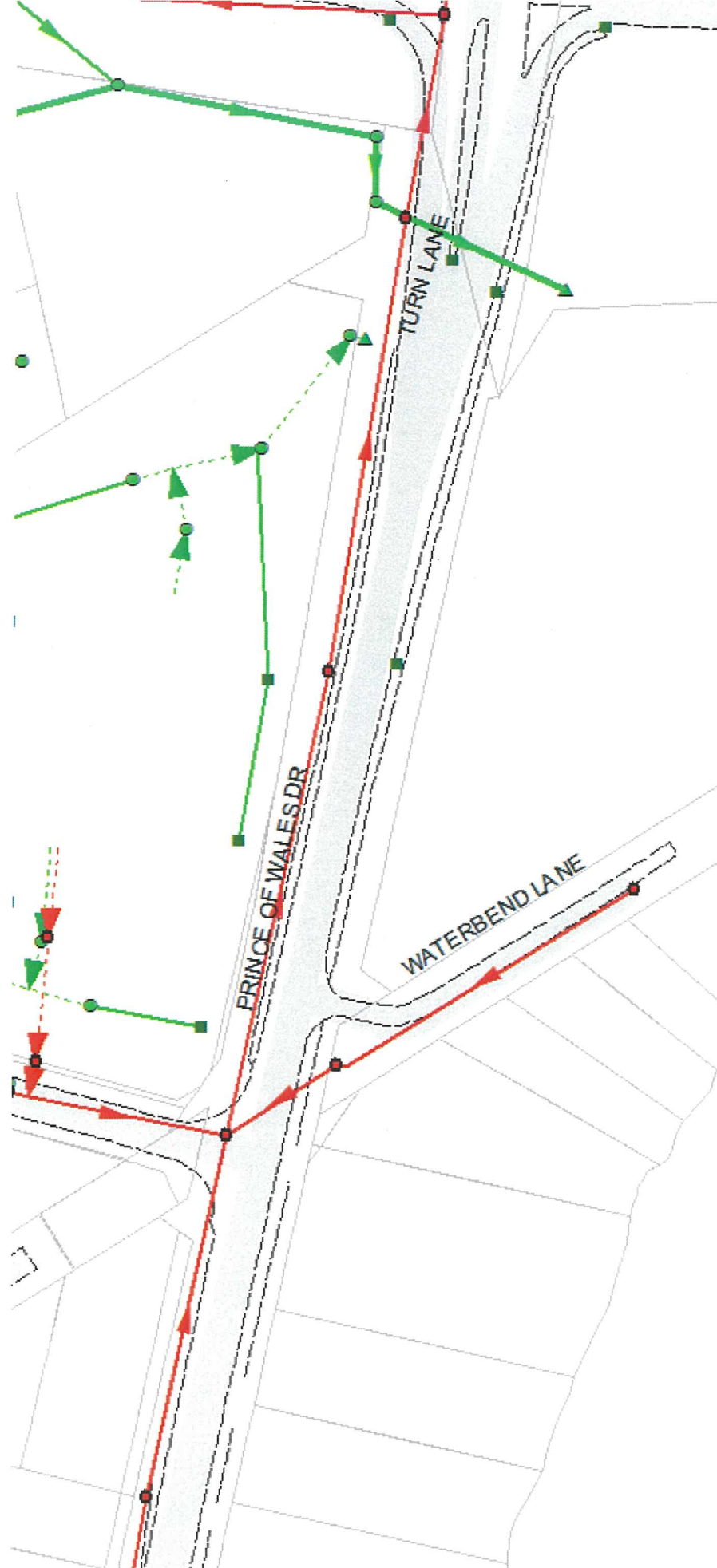
$$\begin{aligned} \text{Max Day + FF} &= 89 \text{ L/s @ 20 psi (Industrial)} \\ 20 \text{ psi} &= 14.1 \text{ m} \end{aligned}$$

$$\therefore \text{HGL} = 14.1 \text{ m} + 87.0 \text{ m} = 101.1 \text{ m @ 20 psi}$$

$$\begin{aligned} \text{Max Day + FF} &= 91 \text{ L/s @ 20 psi (Commercial)} \\ 20 \text{ psi} &= 14.1 \text{ m} \end{aligned}$$

$$\therefore \text{HGL} = 14.1 \text{ m} + 87.0 \text{ m} = 101.1 \text{ m @ 20 psi}$$

APPENDIX B
Sanitary Calculations



Identify

Identify from: <All layers>

Location: 367,485.876 5,021,592.652 Met

| Field | Value |
|---------------------|-----------------------|
| Administrative_Area | NEPEAN |
| CCTV_REPORT_ID | <null> |
| CCTV_STATUS | <null> |
| CCTV_YEAR | 0 |
| COMMENTS | <null> |
| CREATED_BY | <null> |
| CREATED_DATE | <null> |
| District | <null> |
| Flow_Direction | <null> |
| Function | <null> |
| INSITU_SOIL | <null> |
| Install_Year | 1990 |
| Invert_Downstream | 83.28 |
| Invert_Upstream | 83.86 |
| LAST_ID | <null> |
| LENGTH_AS_BUILT | 105 |
| Life_Cycle_Status | In Service |
| Lining_Type | None |
| Lining_Year | 0 |
| MATERIAL | Poly Vinyl Chloride |
| MXASSETNUM | 3695787 |
| NODE1 | MHSA19103 |
| NODE1_TYPE | Sanitary_Manhole |
| NODE2 | MHSA19102 |
| NODE2_TYPE | Sanitary_Manhole |
| Ownership | Public |
| PIPE_CLASS | SDR26 |
| Pipe_Shape | Round |
| REFERENCE | 8180 |
| SHAPE_Length | 108 |
| SLOPE | 0.55 |
| Street | WATERBEND LANE NEP |
| STRUCT_ID | SAN19685 |
| STRUCT_TYPE | Sanitary_Pipe |
| SURFACE_TYPE | <null> |
| TEMP_NUMBER | <null> |
| Trunk | No |
| Ward | 09 Knoxdale-Merivale |
| Width | 200mm |
| XStreet | PRINCE OF WALES DR NI |

Identified 23 features

Analysis of Existing Sanitary Sewer on Waterbend Lane

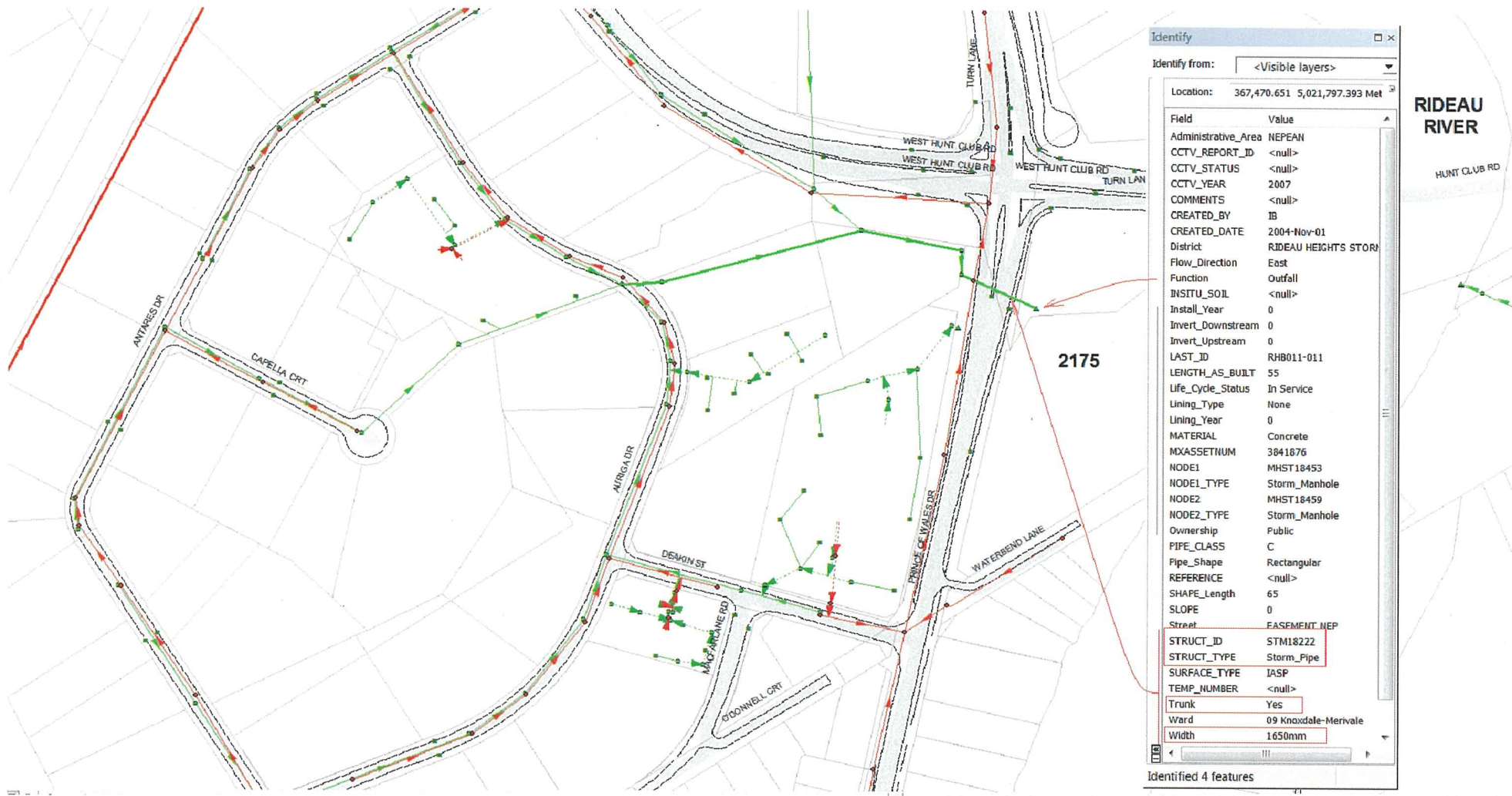
| LOCATION | | | EXISTING RESIDENTIAL | | | | | ICI | | | | INFILTRATION | | | Total Flow (l/s) | PIPE | | | | | |
|------------|------------|------------|----------------------|------|-------------|----------------------|----------------------|------------------|-----------------|-----------------|------------------|---------------------|-----------|-----------|---------------------|------------|----------------|----------------------|-------------------------|-------------|-----------------|
| Site Use | FROM | TO | SINGLES | | | Commercial Area (ha) | Industrial Area (ha) | Accum. Area (ha) | Peak Flow (l/s) | Total Area (ha) | Accum. Area (ha) | Infiltr. Flow (l/s) | Size (mm) | Slope (%) | | Length (m) | Capacity (l/s) | Full Flow Vel. (m/s) | Q/Q _{full} (%) | | |
| | | | Units | Pop. | Accum. Pop. | | | | | | | | | | | | | | | Peak Factor | Peak Flow (l/s) |
| Industrial | EX SANMH 2 | EX SANMH 1 | 5 | 16.5 | 16.5 | 3.7 | 0.2 | | 3.22 | 3.22 | 9.6 | 4.08 | 4.08 | 1.1 | 11.0 | 200 | 0.55 | 81 | 25.4 | 0.78 | 43.2% |
| Commercial | EX SANMH 2 | EX SANMH 1 | 5 | 16.5 | 16.5 | 3.7 | 0.2 | 3.22 | | 3.22 | 1.6 | 4.08 | 4.08 | 1.1 | 2.9 | 200 | 0.55 | 81 | 25.4 | 0.78 | 11.5% |

Design Parameters:

| | |
|---|---|
| Extraneous Flows = | 0.28 l/s/ha |
| Industrial Flow = | 55,000 l/ha/day |
| Commercial Flow = | 28,000 l/ha/day |
| Peaking Factor Commercial = | 1.5 |
| Peaking Factor Heavy Industrial = | 4.7 (Based on Appendix 4-B.1, <i>Ottawa Sewer Design Guidelines</i>) |
| Domestic Flow: | |
| Avg Flow/Person | 280 l/day |
| Population Density: | 3.3 people/unit |
| Residential Peaking Factor = Harmon Equation (max 4, min 2) | |
| Pipe Friction n = | 0.013 |





CHANGES AS PER CITY TECHNICAL BULLETING 2018-01

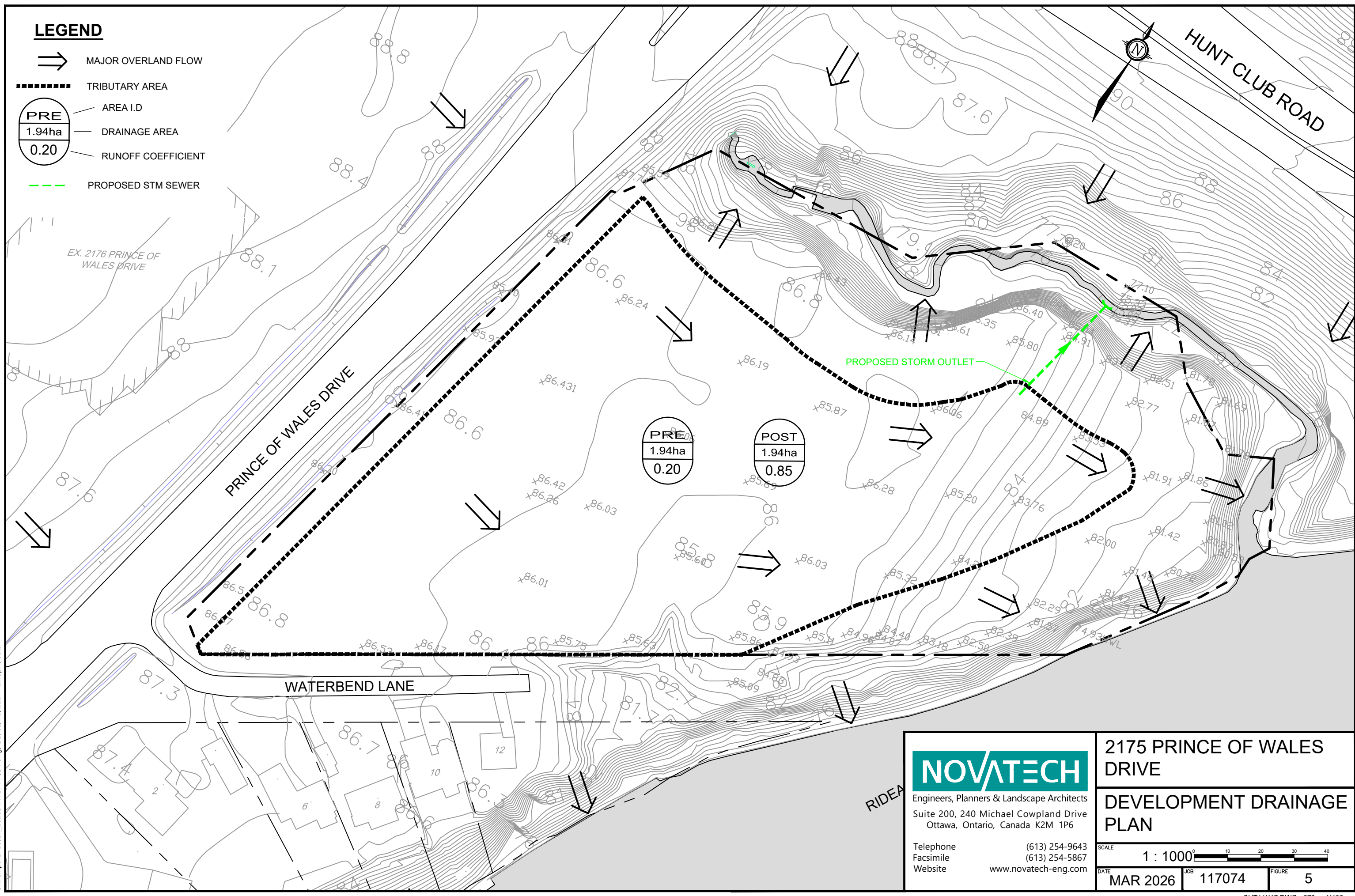
APPENDIX C
Stormwater Management Calculations




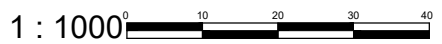
2175 Prince of Wales Dr – ArcMAP (2015) – Storm Sewer Data

LEGEND

-  MAJOR OVERLAND FLOW
-  TRIBUTARY AREA
-  AREA I.D.
DRAINAGE AREA
RUNOFF COEFFICIENT
-  PROPOSED STM SEWER



C:\Temp\AcPublish\25288\117074-FIG4.dwg, GS, Jul 12, 2024 - 2:49pm, abbi

| | | |
|---|-----------------------------------|------------|
|  Engineers, Planners & Landscape Architects Suite 200, 240 Michael Cowpland Drive Ottawa, Ontario, Canada K2M 1P6 Telephone (613) 254-9643 Facsimile (613) 254-5867 Website www.novatech-eng.com | 2175 PRINCE OF WALES DRIVE | |
| | DEVELOPMENT DRAINAGE PLAN | |
| SCALE 1 : 1000  | DATE MAR 2026 | JOB 117074 |
| | | FIGURE 5 |

CUT11V17 DWG 270mm X 420mm

PROJECT #: 117074

PROJECT NAME: 2175 Prince of Wales Dr
 LOCATION: Ottawa, ON
 DATE PREPARED: March 2015
 DATE REVISED: July 2024



TABLE C1: Free Flow Pre-Development

Pre-Development Runoff Coefficient "C"

| Area | Surface | Ha | 1:5 Year Event | | 1:100 Year Event | |
|-------|---------|-------|----------------|----------------|------------------|------------------|
| | | | C | C ₅ | C | C ₁₀₀ |
| Total | Hard | 0.000 | 0.90 | 0.20 | 1.00 | 0.25 |
| 1.940 | Soft | 1.940 | 0.20 | | 0.25 | |

Pre-Development Free Flows

| Outlet Options | Area (ha) | Q _{5 Year} (L/s) | Q _{100 Year} (L/s) |
|--------------------|-----------|---------------------------|-----------------------------|
| Overland Free flow | 1.940 | 112.4 | 240.8 |

Runoff Coefficient Equation

$$C_5 = (A_{\text{hard}} \times 0.9 + A_{\text{soft}} \times 0.2) / A_{\text{Tot}}$$

$$C_{100} = (A_{\text{hard}} \times 1.0 + A_{\text{soft}} \times 0.25) / A_{\text{Tot}}$$

* Runoff Coefficient increases by 25% up to a maximum value of 1.00 for the 100-Year event

Time of Concentration T_c= 10 min
 Rainfall Intensity (5 Year Event) I₅= 104.19 mm/hr
 Rainfall Intensity (100 Year Event) I₁₀₀= 178.56 mm/hr

$$100 \text{ year Intensity} = 1735.688 / (\text{Time in min} + 6.014)^{0.820}$$

$$5 \text{ year Intensity} = 998.071 / (\text{Time in min} + 6.053)^{0.814}$$

PROJECT #: 117074

PROJECT NAME: 2175 Prince of Wales Dr
 LOCATION: Ottawa, ON
 DATE PREPARED: March 2015
 DATE REVISED: July 2024



TABLE C2: Free Flow Post-Development

Post-Development Runoff Coefficient "C"

| Area | Surface | Ha | 1:5 Year Event | | 1:100 Year Event | |
|-------|---------|-------|----------------|----------------|------------------|------------------|
| | | | C | C ₅ | C | C ₁₀₀ |
| Total | Hard | 1.804 | 0.90 | 0.85 | 1.00 | 0.95 |
| 1.940 | Soft | 0.136 | 0.20 | | 0.25 | |

Post-Development Free Flows

| Outlet Options | Area (ha) | Q _{5 Year} (L/s) | Q _{100 Year} (L/s) |
|--------------------|-----------|---------------------------|-----------------------------|
| Overland Free flow | 1.940 | 477.6 | 914.9 |

Runoff Coefficient Equation

$$C_5 = (A_{\text{hard}} \times 0.9 + A_{\text{soft}} \times 0.2) / A_{\text{Tot}}$$

$$C_{100} = (A_{\text{hard}} \times 1.0 + A_{\text{soft}} \times 0.25) / A_{\text{Tot}}$$

* Runoff Coefficient increases by 25% up to a maximum value of 1.00 for the 100-Year event

Time of Concentration T_c= 10 min
 Rainfall Intensity (5 Year Event) I₅= 104.19 mm/hr
 Rainfall Intensity (100 Year Event) I₁₀₀= 178.56 mm/hr

100 year Intensity = $1735.688 / (\text{Time in min} + 6.014)^{0.820}$
 5 year Intensity = $998.071 / (\text{Time in min} + 6.053)^{0.814}$